GENERAL NOTES

- THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH CONTRACT SPECIFICATIONS.
- GEOMETRY, REINFORCEMENT AND LAYOUT OF THE EXISTING STRUCTURE ARE BASED ON EXISTING DESIGN INFORMATION AND LIMITED FIELD SURVEY DATA. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO FIELD VERIFY ALL NECESSARY DIMENSIONS SUCH THAT WORK CAN BE CONSTRUCTED AS SHOWN ON THESE DRAWINGS. THE CONTRACTOR SHALL REPORT ANY DISCREPANCIES TO THE CONTRACT ADMINISTRATOR PRIOR TO CONSTRUCTION.
- CONTRACTOR TO REFER TO REFERENCE DRAWINGS FOR DETAILS OF EXISTING CONSTRUCTION.
- WHOLE DIMENSIONS SHOWN ON THESE DRAWINGS ARE IN MILLIMETERS. DECIMAL DIMENSIONS ARE IN METRES. THE ORIGINAL BRIDGE STRUCTURE WAS CONSTRUCTED WITH IMPERIAL UNITS OF MEASURE (HARD UNIT CONVERSION WHERE APPLICABLE).
- THE SCALES SHOWN ON THESE DRAWINGS ARE CORRECT FOR A1 SIZED DRAWING SHEETS. DO NOT DETERMINE DIMENSIONS BY SCALING OFF DRAWINGS.
- THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING THE EXACT LOCATIONS OF ALL EXISTING ABOVE GROUND AND BELOW GROUND UTILITIES AND REPORTING ANY DISCREPANCIES OR CONFLICTS TO THE CONTRACT ADMINISTRATOR PRIOR TO CONSTRUCTION.
- EXCEPT WHERE INDICATED OTHERWISE THESE DRAWINGS SHOW DETAILS FOR THE COMPLETED STRUCTURE. THE CONTRACTOR IS RESPONSIBLE FOR THE SAFETY OF WORKERS AND THE DESIGN AND STABILITY OF ANY TEMPORARY WORKS DURING CONSTRUCTION. CONSTRUCTION METHODS REQUIRING THE TEMPORARY INSTALLATION OF SHORING, SCAFFOLDING, BRACING, ETC. SHALL BE SUBMITTED TO THE CONTRACT ADMINISTRATOR FOR REVIEW AND ACCEPTANCE PRIOR TO PROCEEDING WITH THE WORK. THE CONTRACTOR SHALL RETAIN A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF MANITOBA TO PERFORM AND TAKE RESPONSIBILITY FOR ANY SUCH DESIGNS NECESSARY TO COMPLETE THE CONSTRUCTION AND AS REQUIRED BY THE CONTRACT DOCUMENTS.

DESIGN DATA NOTES

STRUCTURAL DESIGN DATA

- CULVERT CONFIGURATION: NOMINAL 2100 mm INSIDE DIAMETER x 36.8 m LONG DOUBLE-WIDE SEGMENTAL PRECAST CONCRETE PIPE CULVERT COMPLETE WITH CAST-IN-PLACE HEADWALLS, INSTALLED BY THE OPEN TRENCH METHOD.
- DESIGN SPECIFICATION: CAN/CSA-S6-14 "CANADIAN HIGHWAY BRIDGE DESIGN CODE" • LIVE LOAD:
- CL-625 TRUCK AND CL-625 LANE LOAD

TRANSPORTATION DESIGN DATA DESIGN SPECIFICATIONS:

- CITY OF WINNIPEG TRANSPORTATION STANDARDS (2012 UPDATE)
- •• TRANSPORTATION ASSOCIATION OF CANADA GEOMETRIC DESIGN GUIDE FOR CANADIAN ROADS
- ROADWAY DESIGN CRITERIA:
- ROADWAY CLASSIFICATION: RESIDENTIAL LOCAL (RURAL, NON-REGIONAL)
- POSTED SPEED: 50 km/h; DESIGN SPEED: 60 km/h

GEOTECHNICAL DESIGN DATA

- A GEOTECHNICAL REPORT HAS BEEN PREPARED BY TREK GEOTECHNICAL TITLED "PRELIMINARY DESIGN - RUE DES TRAPPISTES CULVERT REPLACEMENT GEOTECHNICAL REPORT (1ST REVISION)" DATED DECEMBER 6, 2016. REFER TO GEOTECHNICAL REPORT FOR DESIGN DATA AND RECOMMENDATIONS.
- EXCAVATION SLOPES SHOWN BASED ON PRELIMINARY ENGINEERING FOR THE PURPOSE OF SCOPING WORK AND DEVELOPING QUANTITIES.
- THE CONTRACTOR SHALL SUBMIT AN EXCAVATION AND DEMOLITION PLAN WHICH INCLUDES A DESCRIPTION OF THE EXCAVATION METHODOLOGY AND EQUIPMENT, STOCKPILING LOCATIONS, AND THE PROCESS AND RATE OF REMOVALS OF EXCAVATED AND DEMOLISHED MATERIAL. THE SUBMITTAL SHALL INCLUDE AN ASSESSMENT OF THE IMPACT OF SURCHARGE LOADS INTRODUCED BY CONSTRUCTION ACTIVITIES ON THE STABILITY OF THE EXCAVATION, AND SHALL INCLUDE SLOPE STABILITY ANALYSIS SIGNED AND SEALED BY A GEOTECHNICAL ENGINEER LICENSED TO PRACTICE IN THE PROVINCE OF MANITOBA
- SELECT GEOTECHNICAL DESIGN DATA:
- •• MINIMUM FACTOR OF SAFETY AGAINST SLOPE FAILURE IN FINAL CONSTRUCTED CONDITION = 1.28
- •• BEARING RESISTANCE OF UNDISTURBED FIRM TO STIFF CLAY IMMEDIATELY BELOW PROPOSED CULVERT
- σ_{ULS} = 100 kPa, INCLUDING A RESISTANCE FACTOR OF Φ = 0.50 ... σ_{SLS} = 60 kPa, BASED ON A MAXIMUM ALLOWABLE SETTLEMENT OF 25 mm •••
- •• UNIT WEIGHT OF GRANULAR BACKFILL, : 21 KN/m³
- •• LATERAL EARTH PRESSURE COEFFICIENTS FOR HEADWALL DESIGN:
- ••• AT REST, $K_0 = 0.4$ PASSIVE, $K_P = 3.0$...

HYDRAULIC DESIGN DATA

- A HYDRAULIC REPORT HAS BEEN PREPARED BY BRUCE HARDING CONSULTING LTD. TITLED "RUE DES TRAPPISTES CROSSING REPLACEMENT HYDROLOGIC AND HYDRAULIC ASSESSMENT (REV 2)" DATED MARCH 2017.
- HYDRAULIC DESIGN DATA:
- DESIGN DISCHARGE (Q1%) FOR THE DOUBLE WIDE SEGMENTAL PRECAST CULVERT PIPE IS THE 1% FLOOD, WHICH HAS A 1% PROBABILITY OF EXCEEDANCE IN A GIVEN YEAR.
- HIGH WATER LEVEL FOR THE 1% FLOOD AND 50% FLOOD AT THE CULVERT ARE SHOWN ON DETAIL 1, SHEET 7.
- •• DESIGN DISCHARGE AT THE 1% FLOOD $Q_{1\%}$ = 10.5 m³ @ EL. 227.79 m
- VELOCITY AT DESIGN DISCHARGE V_{1%} = 2.05 m/s
- •• HEADLOSS DURING DESIGN DISCHARGE = 0.22 m
- •• MAXIMUM VELOCITY TO ACCOMMODATE FISH PASSAGE IS NOT A DESIGN CRITERIA AT THIS SITE
- •• MINIMUM SOFFIT ELEVATION NO SUBMERGENCE DURING PASSAGE OF DESIGN DISCHARGE (WHEN LASALLE RIVER IS LOW). WHEN LASALLE RIVER IS IN FLOOD, CULVERT MAY BE COMPLETELY SUBMERGED.

ENVIRONMENTAL PROTECTION

- NO IN-STREAM WORK IS PERMITTED BETWEEN APRIL 1 AND JUNE 15.
- IMPLEMENT ENVIRONMENTAL PROTECTION MEASURES AS DESCRIBED BY THE CONTRACT SPECIFICATIONS.

EXISTING UTILITY PROTECTION

- SEVERAL UTILITIES ARE BURIED BELOW THE WORK ZONE, INCLUDING BUT POTENTIALLY NOT LIMITED TO: 50 GAS, 250 WWS, 200 WM, AND MTS CONDUIT. OVERHEAD UTILITIES INCLUDE A POWERLINE AND MTS FIBRE-OPTICS.
- THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING THE EXACT LOCATIONS OF ALL EXISTING ABOVE GROUND AND BELOW GROUND UTILITIES AND REPORTING ANY DISCREPANCIES OR CONFLICTS TO THE CONSULTANT PRIOR TO CONSTRUCTION.
- THE 250 WWS IS EXTREMELY SHALLOW RELATIVE TO THE BASE OF THE EXCAVATION. THE WWS CANNOT BE TAKEN OUT OF SERVICE TO FACILITATE CONSTRUCTION, AND INADVERTENT DAMAGE CAUSED TO THE PIPE MAY HAVE SEVERE CONSEQUENCES.

- THE 200 WM IS SHALLOW RELATIVE TO THE BASE OF THE EXCAVATION. THE WM CANNOT BE TAKEN OUT OF SERVICE TO FACILITATE CONSTRUCTION, AND INADVERTENT DAMAGE CAUSED TO THE PIPE MAY HAVE SEVERE CONSEQUENCES.
- THE CONTRACTOR SHALL SUBMIT A CONSTRUCTION METHOD STATEMENT DEMONSTRATING ADHERENCE TO THE OPERATING CONSTRAINTS FOR WORK IN CLOSE PROXIMITY TO ALL BURIED AND OVERHEAD UTILITIES.

PROPERTY AND CONSTRUCTION EASEMENTS

- CITY OF WINNIPEG OWNED PROPERTIES PROXIMATE TO CULVERT WORKS: PLAN 13,968 LOT 2, PLAN 22,368 LOT 14.
- CONSTRUCTION EASEMENTS HAVE BEEN OBTAINED FOR TEMPORARY WORKS AND SLOPE SHAPING / REGRADING FOR THE FOLLOWING PROPERTIES: PLAN 6857 PT PCL A, AND A SMALL TRIANGULAR AREA OF PLAN 10.669 BLOCK 3 LOT 1 AT THE CREEK TIE-IN.

MATERIAL NOTES

CONCRETE:

DELEGATED DESIGN OF PRECAST CONCRETE PIPE

THE DESIGN OF THE PRECAST REINFORCED CONCRETE CULVERT PIPE IS DELEGATED TO

| ITEM | CLA EXP | SS OF OSURE | CEMENT TYPE | MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS (MPa) | NOMINAL MAX SIZE OF AGGREGATE (mm) | AIR CONTENT (%) | | | |
|--------------------------------------------------------------|------------------|----------------|-----------------------|--------------------------------------------------------|---------------------------------------------|--------------------|--|--|--|
| PRECAST CONCRETE CULVERT PIPE | F-1 | & S-1 | HS, HSb | 35 | 20 | 5-8 | | | |
| CAST-IN-PLACE HEADWALLS | F-1 & S-1 | | S-1 HS, HSb 35 | | 20 | 5-8 | | | |
| CLEAR COVER REINFORCING ST (mm) UNLESS NO OTHERWISE | TO EEL TED | LOCATION | | | | | | | |
| 50 | | | PRECAST CONCRETE PIPE | | | | | | |
| | | | | | | | | | |

- 75 EARTH SIDE AND CREEK SIDE OF CAST-IN-PLACE HEADWALLS
- CONCRETE FINISHES REFER TO SPECIFICATIONS
- ALL VISUALLY EXPOSED CONCRETE CORNERS SHALL HAVE A 20 mm CHAMFER UNLESS NOTED OTHERWISE.

REINFORCEMENT FOR REINFORCED PRECAST CONCRETE CULVERT PIPE: DESIGN OF REINFORCEMENT FOR REINFORCED PRECAST CONCRETE CULVERT PIPE IS

- DELEGATED TO THE PIPE SUPPLIER.
- STEEL REINFORCEMENT SHALL CONSIST OF CARBON STEEL BARS, COLD-DRAWN STEEL WIRE, DEFORMED STEEL WIRE, WELDED STEEL WIRE FABRIC, OR WELDED DEFORMED STEEL WIRE FABRIC AND SHALL COMPLY WITH THE SPECIFICATIONS.

GASKETS FOR PRECAST CONCRETE CULVERT PIPE:

 GASKETS SHALL MEET THE REQUIREMENTS OF ASTM C443 AND CAN/CSA A257.4 CONCRETE PIPE GASKETS, FLEXIBLE RUBBER.

REINFORCING STEEL

 PLAIN REINFORCING STEEL TO CAN/CSA-G30.18-09 GRADE 400W. • REINFORCING STEEL SPLICES TO CAN/CSA S6-14 CLASS B.

| | LAP SPLICE TABLE | |
|----------|------------------|--|
| BAR SIZE | HORIZONTAL LAP | |
| 15M | 700 | |
| 20M | 850 | |
| | | |

STAINLESS STEEL STRAPS FOR PRECAST CONCRETE CULVERT PIPE • STRAPS SHALL BE STAINLESS STEEL IN ACCORDANCE WITH ASTM A320, ANSI TYPE 36 MARKED AS SUCH WITH RAISED OR INDENTED NUMERALS.

POST-INSTALLED ANCHOR RODS

- POST-INSTALLED ANCHOR ROD SHALL BE STAINLESS STEEL THREADED ROD ANCHORS MEETING THE REQUIREMENTS OF AISI 316 ASTM F593 CW1, WITH MINIMUM YIELD STRENGTH OF 448 MPA. HILTI HAS-R 316 RODS OR ACCEPTED EQUIVALENT.
- NUTS FOR POST-INSTALLED ANCHOR RODS SHALL MEET ASTM F594.
- WASHERS FOR POST-INSTALLED ANCHOR RODS SHALL MEET SS304 OR SS316.

MISCELLANEOUS METAL

• STRUCTURAL SHAPES AND PLATES, MATERIAL REQUIREMENTS TO CSA G40.20-04/G40.21-04 (r2009) GRADE 300W.

WELDING SHALL CONFORM TO CURRENT AWS SPECIFICATION D1.5.

• ASPHALTIC CONCRETE PAVEMENT TYPE 1A PER CW 3310.

ASPHALTIC CONCRETE PAVEMENT

BASE & SUB-BASE

- BASE AND SUB-BASE SHALL BE SUPPLIED, PLACED, AND COMPACTED IN AN UNFROZEN CONDITION.
- BASE AND SUB-BASE COURSE SHALL COMPLY WITH CW 3110.





THE PRECAST CONCRETE SUPPLIER. REFER TO THE SPECIFICATIONS FOR REQUIREMENTS.

VERTICAL LAP 550

650

STRUCTURAL BACKFILL AND GRANULAR BEDDING

BACKFILL SHALL BE SUPPLIED, PLACED, AND COMPACTED IN AN UNFROZEN CONDITION.

- BACKFILL IN OVERFILL ZONE OF EXCAVATION
- BACKFILL OVERFILL ZONE WITH TYPE 1 MODIFIED GRANULAR BACKFILL PER CW ••• 2030 WITH REQUIREMENTS AS MODIFIED IN THE SPECIFICATIONS. COMPACT TO MINIMUM 95% SPMDD.

•• BACKFILL AT PRECAST CONCRETE PIPE

- BACKFILL WITH TYPE 2 MODIFIED GRANULAR BACKFILL PER CW 2030, WITH REQUIREMENTS AS MODIFIED IN THE SPECIFICATIONS.
- MAXIMUM LIFT HEIGHT 300 mm. ... MAXIMUM IMBALANCE IN LIFT HEIGHT ACROSS PIPE WIDTH DURING BACKFILLING ...
- 200 mm
- COMPACTION OF NATIVE CLAY SURFACE, GRANULAR BEDDING AND GRANULAR BACKFILL PROXIMATE TO THE REINFORCED PRECAST CONCRETE CULVERT PIPE INCLUDING THE HAUNCH ZONE, MIDDLE BEDDING ZONE, OUTER BEDDING ZONE, SHALL BE ACCORDING TO DETAIL 3, SHEET 08.
- BACKFILL AT CAST-IN-PLACE RETAINING WALLS
- BACKFILL WITH TYPE 2 MODIFIED GRANULAR BACKFILL PER CW 2030, WITH REQUIREMENTS AS MODIFIED IN THE SPECIFICATIONS.
- WITHIN 1.5 m OF RETAINING WALLS, LIGHTLY COMPACT GRANULAR BACKFILL TO ... MAXIMUM 92% SPMDD USING LIGHT HAND-OPERATED VIBRATING PLATE COMPACTOR.
- •• GRANULAR BACKFILL BELOW CAST-IN-PLACE APRON SLABS
- BACKFILL WITH TYPE 2 MODIFIED GRANULAR BACKFILL PER CW 2030, WITH
- REQUIREMENTS AS MODIFIED IN THE SPECIFICATIONS. MAXIMUM LIFT HEIGHT 150 mm. •••
- COMPACT TO MINIMUM 100% SPMDD. •••

SUBDRAINS

 SUBDRAINS SHALL BE PERFORATED SCHEDULE 40 PVC DRAIN PIPE IN CLEAN-DRAINING GRAVEL WRAPPED IN GEOTEXTILE.

COMMON FILL FOR CREEKWORKS

- COMMON FILL FOR CREEKWORKS SHALL CONSIST OF SUITABLE SITE BACKFILL MATERIAL CLASS 4 BACKFILL: COMPACTED, EXCAVATED MATERIAL PER CW 2030 AND SD-002.
- COMMON FILL FOR CREEKWORKS SHALL BE PLACED AND COMPACTED IN AN UNFROZEN CONDITION. REMOVE AND DISPOSE OF UNSUITABLE FROZEN EXCAVATED MATERIAL.
- COMMON FILL FOR CREEKWORKS SHALL BE COMPACTED IN LIFTS NO GREATER THAN 500 MM TO 90% SPMDD.

CHAIN LINK FENCE

- CHAIN LINK FENCE SHALL CONFORM TO CW 3550-R3. POSITION POSTS AS SHOWN ON THE DRAWINGS.
- FIT MESH ON THE SIDE OF THE CULVERT HEADWALLS WHERE EARTH IS RETAINED, OR ON THE SIDE OF THE FENCE FACING THE SIDEWALK.
- FIT CHAIN LINK FENCE WITH TOP RAILS AND BOTTOM RAILS. CONNECT CHAIN MESH FABRIC TO TOP AND BOTTOM RAILS BY MEANS OF CONTINUOUS SPIRAL GALVANIZED TIE-WIRE, WITH AT LEAST ONE FULL WRAP AROUND THE SUPPORTING RAIL EVERY 150 mm.
- FIELD-APPLIED GALVANIZING, TOUCH-UP, AND WELD GALVANIZING SHALL BE DONE WITH SELF-FLUXING, LOW TEMPERATURE, ZINC-BASED ALLOY RODS IN ACCORDANCE WITH ASTM A780-01. GALVALLOY OR WELCO GAL-VIZ GALVANIZING ALLOY, OR ACCEPTED EQUIVALENT IN ACCORDANCE WITH B7.

RIGID BOARD INSULATION

- EXTRUDED POLYSTYRENE RIGID INSULATION IN ACCORDANCE WITH NATIONAL STANDARD OF CANADA CAN/ULC-S701 (TYPE 4).
- MINIMUM COMPRESSIVE STRENGTH SHALL BE 275 KPA (40 PSI) IN ACCORDANCE WITH ASTM D1621.

EROSION CONTROL BLANKET

 EROSION CONTROL BLANKET SHALL BE MACHINE PRODUCED 100% COCONUT FIBRE MATRIX MEETING THE REQUIREMENTS OF THE SPECIFICATIONS.

STRAW WATTLES

STRAW WATTLES SHALL MEET THE REQUIREMENTS OF THE SPECIFICATIONS.

GROUTED STONE RIP-RAP

 GROUTED STONE RIP-RAP AT DITCH INLET GRATE SHALL MEET THE REQUIREMENTS OF CW 3615.

ROCKFILL SHEAR KEY EXCAVATION NORTH OF RUE DES TRAPPISTES

- INSTALL ROCKFILL SHEAR KEY FOLLOWING EXCAVATION OF CULVERT TRENCH. BACKFILL NARROW ROCK KEY EXCAVATION TRENCH IMMEDIATELY FOLLOWING PERFORMANCE OF EXCAVATION. DO NOT LEAVE EXPOSED.
- BACKFILL ROCKFILL SHEAR KEY WITH 150mm CRUSHED LIMESTONE SUB-BASE MATERIAL IN ACCORDANCE WITH CW 3110.
- COMPACTION AS DESCRIBED IN THE SPECIFICATIONS.

THICKENED RIP-RAP ROCK KEY EXCAVATION SOUTH OF RUE DES TRAPPISTES

INSTALL ROCK KEY FOLLOWING EXCAVATION OF CULVERT TRENCH.

CLASS 450 RIP-RAP

- CLASS 450 RIP-RAP SHALL BE AS DESCRIBED IN THE SPECIFICATIONS.
- INSTALL ON NON-WOVEN GEOTEXTILE KEYED MINIMUM 300 mm VERTICALLY INTO THE TOP OF SLOPE.

GEOTEXTILE

 GEOTEXTILE FOR RIP-RAP AND SUBDRAINS SHALL BE NON-WOVEN GEOTEXTILE IN ACCORDANCE WITH CW 3120 AND CW 3130.

DRAINAGE BOARD

• DRAINAGE BOARD SHALL BE NU-DRAIN DN50-1 OR ACCEPTED EQUAL IN ACCORDANCE WITH

| | LOCATION APPROVED UNDERGROUND STRUCTURES | | BM ELEV | | | | | | PROFESSIONAL'S SEAL | |
|--------|-------------------------------------------------------------------------------------------------------------------|----------|--------------------------------|----------------------|-----------|----------------|--------|-----------------|---------------------|---------------------------|
| 1 | SUPR. U/G STRUCTURES DATE COMMITTEE | | | | | | MORRIS | ON HER | SHFIELD | PROVINCE OF MAR |
| | NOTE: LOCATION OF UNDERGROUND STRUCTURES | | | | | DESIGNED BY | DAN | CHECKED BY | BE | ORIGINAL SIGI 17/09/15 |
| | INFORMATION AVAILABLE BUT NO GUARANTEE IS GIVEN THAT ALL EXISTING | | | | | DRAWN BY | MS | APPROVED BY | SAL | A THE ARCE |
| | LOCATIONS ARE EXACT. CONFIRMATION OF EXISTENCE AND EXACT LOCATION OF ALL SERVICES MUST BE OBTAINED EDOM THE | | | | | | NTS | RELEASED FOR CO | NSTRUCTION | CONSULTANT FILE |
| S S | INDIVIDUAL UTILITIES BEFORE PROCEEDING WITH CONSTRUCTION. | 0 No. | ISSUED FOR TENDER REVISIONS | 17/09/15 YY/MM/DD | DAN BY | DATE | | DATE | | 5160966-GN-02 |

| LIST OF ACRONYMS & SYMBOLS* | | | | | |
|-----------------------------|-----------------------------------------------------------------------------|--|--|--|--|
| N,S,E,W | COMPASS DIRECTIONS | | | | |
| ALT | ALTERNATE | | | | |
| ASTM | AMERICAN SOCIETY FOR TESTING AND MATERIALS | | | | |
| AWS | AMERICAN WELDING SOCIETY | | | | |
| BLL | BOTTOM LOWER LAYER | | | | |
| BRG | BEARING | | | | |
| BUL | BOTTOM UPPER LAYER | | | | |
| СВ | CATCH BASIN | | | | |
| CL | CENTRELINE | | | | |
| CPR | CANADIAN PACIFIC RAILWAY | | | | |
| CSA | CANADIAN STANDARDS ASSOCIATION | | | | |
| CW | CITY OF WINNIPEG STANDARD CONSTRUCTION SPECIFICATION | | | | |
| C/W | COMPLETE WITH | | | | |
| EB | EASTBOUND | | | | |
| EL | ELEVATION | | | | |
| EX | EXISTING | | | | |
| FM | FEEDERMAIN | | | | |
| HWL | HIGH WATER LEVEL | | | | |
| MIN | MINIMUM | | | | |
| O/C | ON CENTRE | | | | |
| O/H | OVERHEAD | | | | |
| OHWL | ORDINARY HIGH WATER LEVEL | | | | |
| PL | PROPERTY LINE | | | | |
| RSIC | REINFORCING STEEL INSTITUTE OF CANADA | | | | |
| SD | STANDARD DRAWING (CITY OF WINNIPEG STANDARD CONSTRUCTION SPECIFICATIONS) | | | | |
| SHLD | SHOULDER | | | | |
| SPMDD | STANDARD PROCTOR MODIFIED DRY DENSITY | | | | |
| TLL | TOP LOWER LAYER | | | | |
| TUL | TOP UPPER LAYER | | | | |
| ТҮР | TYPICAL | | | | |
| UG | UNDERGROUND | | | | |
| UNS | UNIFIED CLASSIFICATION SYSTEM | | | | |
| W/ | WITH | | | | |
| WB | WESTBOUND | | | | |
| WL | WATER LEVEL | | | | |
| WM | WATER MAIN | | | | |
| @ | AT | | | | |
| Ø | DIAMETER | | | | |

*REFER ALSO TO TITLE BLOCK ON SELECT DRAWINGS

SECTION & DETAIL SYMBOLS

-SECTION LETTER / DETAIL NUMBER



SHEET WHERE SECTION / DETAIL IS DRAWN -SHEET WHERE SECTION / DETAIL IS TAKEN



BID OPPORTUNITY No. 698-2017