

FOUNDATION

- FOUNDATION DESIGN BASED ON THE GEOTECHNICAL REPORT PREPARED BY DYREGROV CONSULTANTS DATED AUGUST 17, 2005.
- CENTER PILES ON GRADE BEAM UNLESS OTHERWISE NOTED.
- CAST-IN-PLACE PILES TO BE 32 MPa CONCRETE (CLASS S-2 EXPOSURE)
- A GEOTECHNICAL ENGINEER IN EMPLOY OF THE OWNER SHALL INSPECT THE PILE INSTALLATIONS.
- DESIGN SKIN FRICTION VALUE: 16 kPa.

CONCRETE

- ALL CONCRETE CONSTRUCTION, COLD WEATHER CONSTRUCTION & CONCRETE TESTING TO BE IN ACCORDANCE WITH THE LATEST EDITION OF CSA STANDARDS A23.1 AND A23.2.
- ALL CONCRETE TO BE NORMAL WEIGHT HARD ROCK CONCRETE WITH A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 20 MPa WITH A CLASS C-1 EXPOSURE UNLESS SPECIFIED OTHER. CONCRETE IN CONTACT WITH NATIVE SOIL SHALL HAVE A CLASS S-2 EXPOSURE. CONCRETE FOR ALL INTERIOR SLABS AND TOPPING TO BE 25 MPa WITH A CLASS N EXPOSURE.
- ALL EXTERIOR CONCRETE SLABS, CURBS, TOPPING & PADS TO BE 32 MPa WITH A CLASS C-1 EXPOSURE.
- CONCRETE SLUMP TO BE COORDINATED BETWEEN CONTRACTOR AND CONCRETE SUPPLIER CONSIDERING THE PERFORMANCE CRITERIA AND THE CONTRACTOR'S CRITERIA FOR CONSTRUCTION AND PLACEMENT.
- MISCELLANEOUS CONCRETE ELEMENTS (PITS, TRENCHES, ETC.) TO BE MINIMUM 150mm THICK REINFORCED WITH 10M @ 12" O/C EACH WAY UNLESS NOTED OTHERWISE.

REINFORCING

- REINFORCING STEEL SHALL BE GRADE 400 DEFORMED NEW BILLET STOCK CONFORMING TO LATEST CSA SPECIFICATION C30.18-M92. WELDED WIRE MESH SHALL CONFORM TO CSA G30.5-M1983, (R1991). GRADE 300 STEEL MAY BE USED FOR ALL STIRRUPS AND TEMPERATURE STEEL.
- CONCRETE COVER TO BE AS FOLLOWS:
 - CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 75mm (3").
 - EXPOSED TO EARTH OR WEATHER 50mm (2").
 - NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
 - SLABS 20mm (3/4")
 - BEAMS 40mm (1 1/2")
- TOP STEEL IN GRADE BEAMS TO BE SPLICED AT CENTER SPAN AND BOTTOM STEEL TO BE SPLICED OVER SUPPORTS. SPLICE LENGTHS:
 - TENSION ZONE SPLICE TO BE AVOIDED WHEREVER POSSIBLE, BUT IF REQUIRED, LENGTH SHOULD BE SPECIFIED BY THE DESIGN ENGINEER.
 - COMPRESSION ZONE SPLICE SHOULD NOT BE LESS THAN 30 BAR DIAMETERS.

MASONRY

- CONCRETE BLOCKS TO CONFORM TO CSA A165.1-M94 TO SPECIFICATIONS FOR, BLOCK TYPE, WATERPROOFING ADMIXTURES, ETC.
- MASONRY WALLS TO BE BUILT WITH TYPE "S" MORTAR HAVING A MINIMUM STRENGTH OF 13 MPa 28 DAYS. MORTAR TO BE IN ACCORDANCE WITH CAN-S304-M84. ALL MORTAR JOINTS SHALL BE FLUSH, FULL BED JOINTS.
- USE CURB-WALL (OR EQUAL) SPACED VERTICALLY AT 400mm O/C.
- COLD WEATHER CONSTRUCTION OF MASONRY SHALL CONFORM TO THE NATIONAL BUILDING CODE, WITH ADEQUATE PREHEATING OF MATERIALS, HOARDING AND HEATING DURING CONSTRUCTION AND THEREAFTER AS SPECIFIED. THE "TORCHING TECHNIQUE" WILL NOT BE PERMITTED UNDER ANY CIRCUMSTANCES.
- MASONRY CONTRACTOR SHALL BE RESPONSIBLE FOR TEMPORARY BRACING OF ALL MASONRY COMPONENTS UNTIL ALL RELATED STRUCTURAL FRAMING HAS BEEN ERRECTED AND COMPLETELY INSTALLED.
- PROVIDE EXPANSION JOINTS @ MAXIMUM OF 6.5m O/C U.N.O.
- PROVIDE CONTINUOUS BOND BEAMS WITH 2-15M BARS BOTTOM IN CONCRETE FILL AT TOP OF ALL EXTERIOR WALLS, BEARING WALLS OR AS INDICATED ON DRAWINGS. PROVIDE 2-15M VERTICAL BARS AT ALL OPENINGS EXCEEDING 1200mm IN WIDTH AND AT END OF WALLS. FILL WITH CONCRETE.
- INSPECTION HOLES SHALL BE LEFT AT THE BASE OF CONCRETE FILLED WALLS.
- MASONRY CORES SHALL BE FILLED IN LIFTS NOT EXCEEDING 3m.
- CONCRETE BLOCKS TO HAVE COMPRESSIVE STRENGTH OF 15 MPa OF BETTER.
- ENSURE MASONRY CORES FILLED WITH CONCRETE AT EXPANSION ANCHOR LOCATIONS. MINIMUM 4" CONCRETE ON ALL SIDES.
- TYPICAL MASONRY LINTELS UNLESS NOTED ON DRAWINGS:

SPANS UP TO 1200mm	-200 U-BLOCK	2-15M CONT. BOTTOM
SPANS UP TO 2000mm	-400 U-BLOCK	2-15M CONT. BOTTOM

 PROVIDE MINIMUM 200mm BEARING AT EACH END.
- BRICK TIES TO BE "FERRO" BLOCK SHEAR CONNECTORS SPACED AS FOLLOWS:

HORIZONTAL:	450mm O/C
VERTICAL:	1ST ROW @ 200mm FROM TOP AND BOTTOM.
	2ND ROW @ 400mm FROM TOP AND BOTTOM.

 BALANCE @ 600mm O/C.
- INTERIOR 6" WIDE MASONRY BLOCK TO BE 15MPa UNITS, TYPE N MORTAR. INSTALL BOND BEAM AT TOP OF WALL REINFORCED WITH 1-15M BAR. INSTALL 1-15M VERTICAL BAR AT ALL CORNERS AND DOORWAYS, FILL CORES WITH CONCRETE. PROVIDE 10M DOWELS FROM CONCRETE CURB TO MASONRY WALL EVERY 4th CORE, FILL BOTTOM 2 CORES WITH CONCRETE.

STRUCTURAL STEEL

- ALL STRUCTURAL STEEL ROLLED SECTIONS AND STRUCTURAL PLATES SHALL CONFORM TO THE LATEST EDITION OF CSA STANDARDS G40.21-M 350W. ALL HOLLOW STRUCTURAL SECTION SHALL CONFORM TO THE LATEST EDITION OF CSA STANDARD G40.21-M 350W.
- ALL ANCHOR BOLTS SHALL CONFORM TO THE LATEST EDITION OF ASTM A307 UNLESS OTHERWISE NOTED. BOLTED CONNECTION SHALL BE TORQUE-TESTED IN ACCORDANCE WITH THE LATEST EDITION OF CSA S16.1. ANCHOR BOLTS TO BE 3/4" x 18" C/W 3" HOOK.
- ALL WELDERS AND WELDING PROCEDURES TO BE CERTIFIED BY CANADIAN WELDING BUREAU.
- STEEL FABRICATOR TO DESIGN AND SUPPLY ANGLES AS INDICATED FOR SUPPORT AND SUSPENSION OF MECHANICAL EQUIPMENT.

HOLLOWCORE

- THE DESIGN OF ALL PRECAST FLOOR SLABS SHALL BE BY THE SUPPLIERS ENGINEER, AS PER SPECIFICATION, TO SUPPORT LOADS INDICATED ON DRAWING.
- DESIGNS SHALL BE IN ACCORDANCE WITH THE LATEST EDITION CSA 135 AND TOLERANCES SHALL BE IN ACCORDANCE WITH CSA A25.1.
- MINIMUM STRENGTH AT 28 DAYS SHALL BE 35 MPa.
- THE SUPPLIER SHALL CHECK WITH ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR OPENINGS LARGER THAN 150mm AND FORM THEM IN SHOP, ALL FRAMING AND REINFORCING FOR OPENINGS TO BE DESIGNED AND SUPPLIED BY PRECAST SUPPLIER.
- THE SUPPLIER SHALL PROVIDE THE CONTRACTOR WITH SETTING DRAWINGS, SHOWING THE LOCATIONS OF ALL EMBEDDED PARTS REQUIRED.
- HOLLOWCORE SLABS MUST BE LEVELED AND ALIGNED BEFORE GROUTING THE KEYS AND JOINTS.

ITEMS EMBEDDED IN CONCRETE

SEE ALSO CSA-A23.1 - CLAUSE 12.5
EXCEPT WHEN APPROVED BY THE STRUCTURAL ENGINEER, PIPES, CONDUITS, AND SLEEVES EMBEDDED IN CONCRETE SHALL BE INSTALLED IN ACCORDANCE WITH THE FOLLOWING GUIDELINES:

- GENERAL
 - NOT WITHSTANDING THE SATISFYING OF THESE GUIDELINES, THE CONDUIT, SLEEVES, PIPES ETC. SHALL NOT IMPAIR THE STRUCTURAL STRENGTH AND SHALL BE MOVED IF SO DIRECTED BY THE STRUCTURAL ENGINEER.
 - CENTERLINE SPACING TO BE NOT LESS THAN 3 DIAMETERS.
 - CENTERLINE SPACING BETWEEN PARALLEL CONDUIT AND REINFORCING BARS TO BE 3 DIAMETERS.
 - ADD REINFORCING AT POINTS OF CONGESTION AS DIRECTED BY THE STRUCTURAL ENGINEER.
- FOR SLABS - CONDUITS IN THE PLANE OF THE SLAB:
 - LOCATE BETWEEN TOP AND BOTTOM REINFORCING. (WHERE APPLICABLE)
 - MAXIMUM SIZE IN ONE LAYER TO BE NOT MORE THAN 1/4 OF CONCRETE THICKNESS.
 - THREE LAYERS OR MORE CROSSING WILL NOT BE PERMITTED.
- FOR WALLS - CONDUIT/ PIPES NOT ALLOWED WITHOUT THE WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER.

NON-STRUCTURAL ELEMENTS

- "NON-STRUCTURAL" OR "SECONDARY STRUCTURAL" ELEMENTS ARE NOT THE RESPONSIBILITY OF TOWER ENGINEERING GROUP. THEY ARE DESIGNED, DETAILED AND REVIEWED IN THE FIELD BY OTHERS. THEY APPEAR ON DRAWINGS OTHER THAN THOSE OF TOWER ENGINEERING GROUP WHERE STRUCTURAL ENGINEERING RESPONSIBILITY IS REQUIRED FOR THESE ELEMENTS, THIS SHALL BE PROVIDED BY SPECIALTY STRUCTURAL ENGINEERS, WHO SHALL ALSO PROVIDE ANY LETTERS REQUIRED BY BUILDING PERMIT AUTHORITIES.
- EXAMPLES OF NON-STRUCTURAL ELEMENTS INCLUDE, BUT ARE NOT LIMITED TO:
 - ARCHITECTURAL COMPONENTS SUCH AS GUARDRAILS, HANDRAILS, CEILINGS, MILLWORK ETC.
 - LANDSCAPE ELEMENTS SUCH AS BENCHES, LIGHT POSTS, PLANTERS, ETC.
 - CLADDING, GLAZING, WINDOW MULLIONS, INTERIOR STUD WALLS AND EXTERIOR STUD WALLS.
 - ARCHITECTURAL PRECAST, PRECAST CLADDING.
 - MECHANICAL AND ELECTRICAL EQUIPMENT, COMPONENTS, AND THEIR ATTACHMENT DETAILS.
 - ELEVATORS AND CONVEYING SYSTEMS.
 - BRICK OR BLOCK VENEERS AND THEIR ATTACHMENTS.
 - NON-LOAD BEARING MASONRY.
 - NON-STRUCTURAL CONCRETE TOPPING
 - ALUMINUM SKYLIGHTS.
- SHOP DRAWINGS FOR NON-STRUCTURAL ELEMENTS WHICH MAY AFFECT THE PRIMARY STRUCTURAL SYSTEM SHALL BE SUBMITTED TO TOWER ENGINEERING GROUP THESE DRAWINGS WILL BE REVIEWED ONLY FOR THE EFFECT ON THE PRIMARY STRUCTURAL SYSTEM.

STRUCTURAL MOVEMENTS/ TOLERANCES

THIS STRUCTURE WILL UNDERGO NORMAL TYPES OF MOVEMENT AND DEFLECTION AND THE NON-STRUCTURAL COMPONENTS MUST BE DETAILED TO ACCOMMODATE THIS. DRYWALL PARTITIONS, MECHANICAL EQUIPMENT, ELECTRICAL EQUIPMENT AND BUILDING FIXTURES MUST BE DETAILED AND INSTALLED TO ACCOMMODATE SLAB MOVEMENT. ALL STRUCTURES ARE SUBJECT TO CONSTRUCTION TOLERANCES. THIS SHOULD BE ALLOWED FOR IN DETAILING NON-STRUCTURAL COMPONENTS.

LUMBER

- FRAMING LUMBER SHALL CONFORM TO THE LATEST EDITION OF CSA 0141 AND SHALL BE OF THE FOLLOWING MINIMUM GRADES:

LINTELS, JOISTS, AND BEAMS: S-P-F NO. 2
STUD WALLS: S-P-F NO. 2
- ALL SHEATHING MATERIAL TO BE 1/2" STD. SPRUCE PLYWOOD IN ACCORDANCE WITH CSA 0325 UNLESS NOTED OTHERWISE. ALL SHEETS TO BE STAGGERED. FASTEN SHEETS WITH 3" COMMON NAILS AT 12" O/C ALONG ALL STUDS AND AT 6" O/C ALONG EDGES OF SHEET, UNLESS NOTED OTHERWISE.
- ALL FLOOR AND ROOF JOISTS TO HAVE CONTINUOUS CROSS BRIDGING AT 6'-0" MAX. SPACING UNLESS NOTED OTHERWISE.

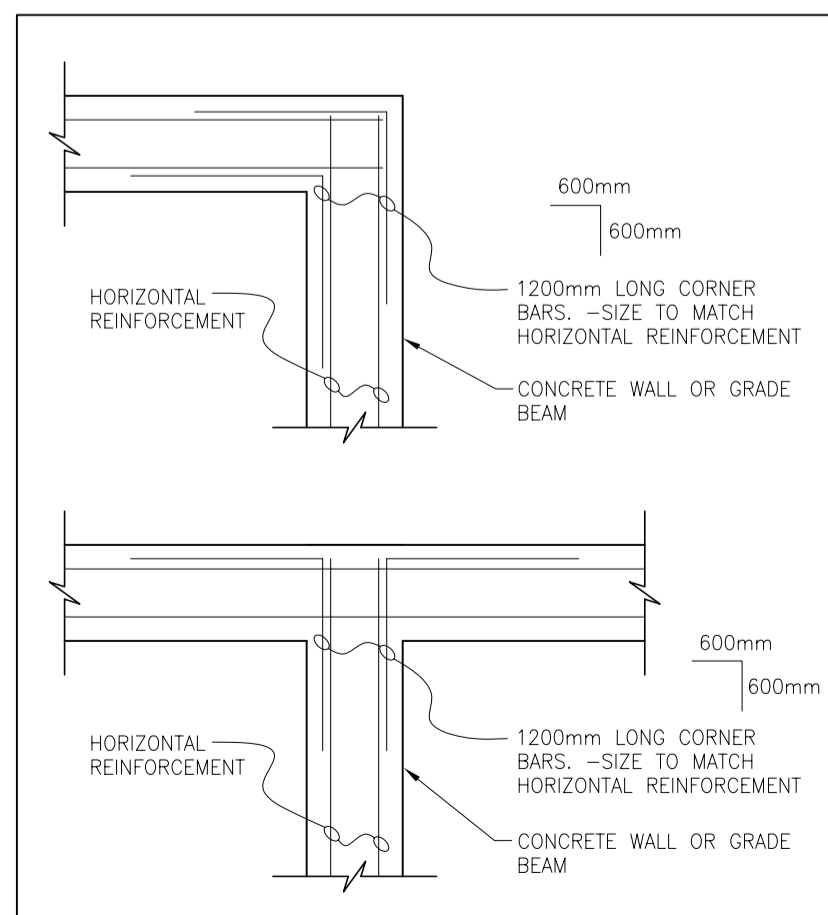
WOOD TRUSSES

- DESIGN ROOF TRUSSES, BRACING, BRIDGING, AND CONNECTORS TO THE REQUIREMENTS OF CSA 086.1-M89, AND OTHER APPLICABLE STANDARDS, TO SAFELY CARRY LOADS AS INDICATED ON THE DRAWINGS.
- SUBMIT SHOP DRAWINGS BEARING STAMP OF QUALIFIED PROFESSIONAL ENGINEER RESPONSIBLE FOR DESIGN.
 - INDICATE SPECIES, SIZES, AND STRESS GRADES OF LUMBER USED AS TRUSS MEMBERS. SHOW PITCH, SPAN, CAMBER CONFIGURATION, AND SPACING OF TRUSSES. INDICATE CONNECTOR TYPES, THICKNESS SIZES, LOCATIONS AND DESIGN VALUE. SHOW BEARING DETAILS.
 - SUBMIT DIAGRAM INDICATING DESIGN LOAD ON EACH TRUSS MEMBER, SPECIAL LOADS, ALLOWABLE STRESS INCREASE AND DEFLECTION LIMITS.
- TRUSS SUPPLIER SHALL BE RESPONSIBLE FOR FINAL INSPECTION AND CERTIFICATION THAT TRUSSES ARE CONSTRUCTED AND ERRECTED AS PER TRUSS SUPPLIERS DESIGN ASSUMPTIONS.

DESIGN LOADS

SNOW LOAD	Ss = 1.9 kPa (1/50)	Sr = 0.2 kPa
WIND LOAD	0.45 kPa (1/50)	0.35 kPa (1/10)
SEISMIC LOADS	AS PER CODE	Sa(0.2) = 0.12 Sa(0.5) = 0.056 Sa(1.0) = 0.023 Sa(2.0) = 0.006
	PGA = 0.059	

FLOOR LOADS REFER TO PLAN
CONTRACTOR TO ENSURE THAT CONSTRUCTION LOADS DO NOT EXCEED DESIGN LOADS.



1 TYPICAL CORNER BARS
SCALE: 1:25

ABBREVIATIONS

ALT.-----	ALTERNATE	N.T.S.-----	NOT TO SCALE
BOT.-----	BOTTOM	O/C-----	ON CENTER
B.W.-----	BOTH WAYS	R/W-----	REINFORCED WITH
C/W-----	COMPLETE WITH	S.D.L.-----	SUPERIMPOSED DEAD LOAD
D.L.-----	DEAD LOAD	SIM.-----	SIMILAR
E.E.-----	EACH END	S.O.G.-----	SLAB ON GRADE
E.F.-----	EACH FACE	STAG.-----	STAGGER
E.S.-----	EACH SIDE	S.J.-----	STRUT JOIST
E.W.-----	EACH WAY	TYP.-----	TYPICAL
H.1E-----	HOOK ONE END	T/O-----	TOP OF
H.2E-----	HOOK TWO ENDS	TIE-----	TIE ONE END
H & V-----	HORIZONTAL AND VERTICAL	T & B-----	TOP AND BOTTOM
HORIZ.-----	HORIZONTAL	T & C-----	TENSION AND COMPRESSION
LL-----	LIVE LOAD	U.N.O.-----	UNLESS NOTED OTHERWISE
MAX.-----	MAXIMUM	VERT.-----	VERTICAL
MIN.-----	MINIMUM		

FIELD REVIEW BY TOWER ENGINEERING GROUP

TOWER ENGINEERING GROUP PROVIDES FIELD REVIEW ONLY FOR THE WORK SHOWN ON THESE STRUCTURAL DRAWINGS. THIS REVIEW IS NOT A "FULL TIME" REVIEW BUT IS A PERIODIC REVIEW AT THE SOLE DISCRETION OF TOWER ENGINEERING GROUP'S ENGINEERS IN ORDER TO ASCERTAIN THAT THE WORK IS IN GENERAL CONFORMANCE WITH THE PLANS AND SUPPORTING DOCUMENTS PREPARED BY TOWER ENGINEERING GROUP. FIELD REVIEW BY TOWER ENGINEERING GROUP IS NOT CARRIED OUT FOR THE CONTRACTOR'S BENEFIT, NOR DOES IT MAKE TOWER ENGINEERING GROUP GUARANTORS OF THE CONTRACTOR'S WORK. IT REMAINS THE CONTRACTOR'S RESPONSIBILITY TO BUILD THE WORK IN CONFORMANCE WITH THE CONTRACT DOCUMENTS. TOWER ENGINEERING GROUP SHALL NOT BE RESPONSIBLE FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR, SUBCONTRACTOR, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK OF FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

TOWER ENGINEERING GROUP WILL REVIEW SHOP DRAWINGS PERTAINING TO WORK SHOWN ON TOWER ENGINEERING GROUP'S DRAWINGS. THE EXTENT OF THIS REVIEW IS AT THE SOLE DISCRETION OF TOWER ENGINEERING GROUP'S ENGINEER AND IS FOR THE SOLE PURPOSE OF ASCERTAINING GENERAL CONFORMANCE WITH THE STRUCTURAL DESIGN CONCEPT. THE REVIEW IS NOT AN APPROVAL OF THE DESIGN, DETAILS AND DIMENSIONS INHERENT IN THE SHOP DRAWINGS. RESPONSIBILITY FOR WHICH SHALL REMAIN WITH THE CONTRACTOR SUBMITTING THEM. SUCH REVIEW SHALL NOT RELIEVE THE CONTRACTOR OF HIS RESPONSIBILITY FOR ERRORS AND OMISSIONS IN THE SHOP DRAWINGS OF FOR MEETING ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS.

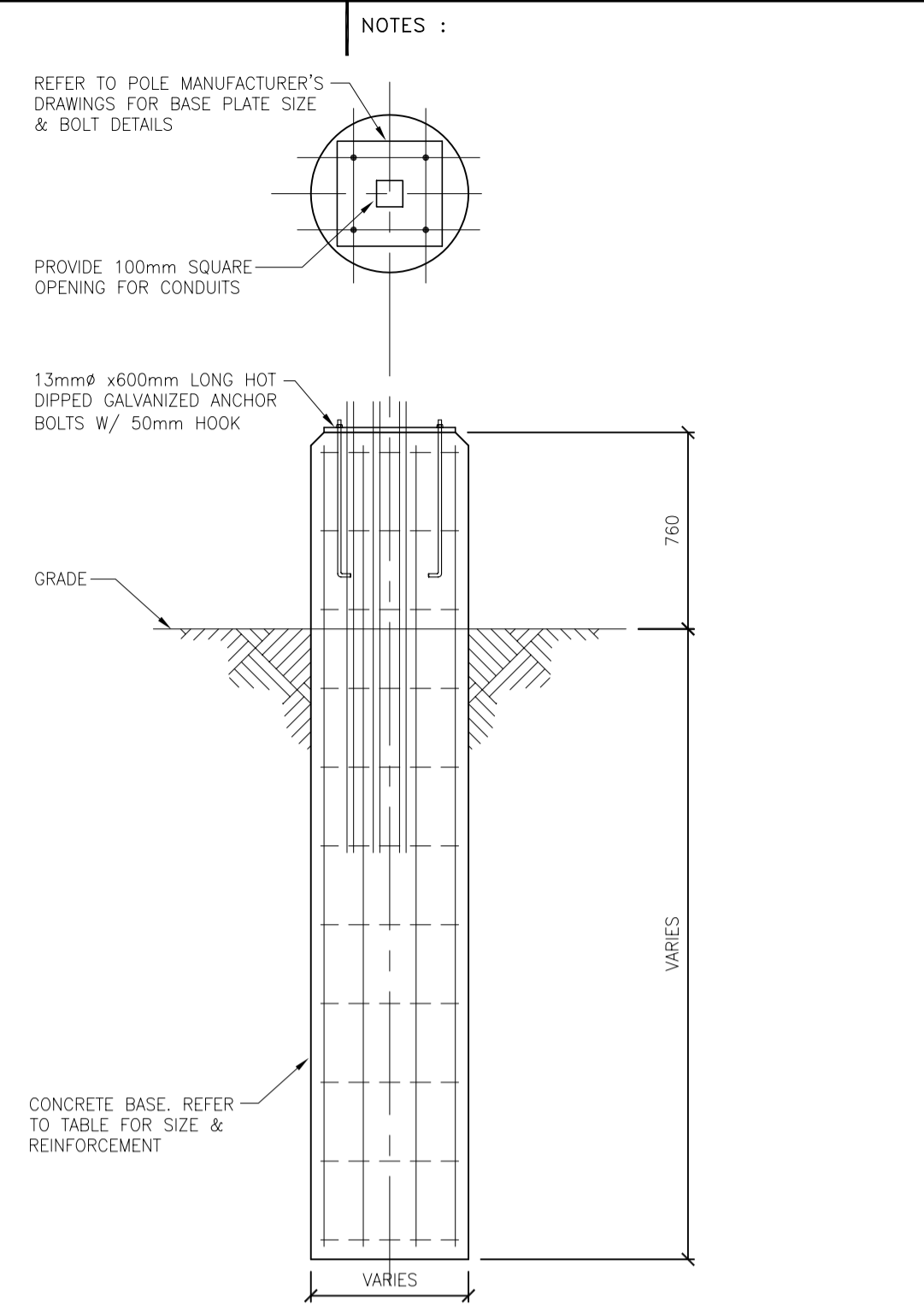
GENERAL NOTES

- THIS SET OF DRAWINGS SHOWS THE COMPLETED PROJECT. THEY DO NOT INCLUDE COMPONENTS THAT MAY BE NECESSARY FOR CONSTRUCTION SAFETY. THE CONTRACTOR IS RESPONSIBLE FOR SAFETY IN AND ABOUT THE JOB SITE DURING CONSTRUCTION, AND THE DESIGN AND ERECTION OF ALL TEMPORARY STRUCTURES, FORM WORK, FALSE WORK, SHORING, ETC. REQUIRED TO COMPLETE THE WORK.
- THE USE OF THESE DRAWINGS IS LIMITED TO THAT IDENTIFIED IN THE REVISIONS COLUMN. DO NOT CONSTRUCT FROM THESE DRAWINGS UNLESS MARKED "ISSUED FOR CONSTRUCTION" OR "ISSUED FOR TENDER" IN THE REVISION'S COLUMN BY TOWER ENGINEERING GROUP.
- THE INFORMATION ON THIS DRAWING SHALL NOT BE USED FOR ANY OTHER THAN THE SPECIFIED WORKS OR PART OF THE WORKS FOR WHICH IT HAS BEEN AUTHORIZED BY TOWER ENGINEERING GROUP.
- SECTION MARKER SHOWN THUS MEANS SECTION # SHOWN ON DRAWING SHEET S#.
- SEE ARCHITECTURAL DRAWINGS FOR FLOOR AND ROOF ELEVATIONS, RECESSED, DRAINAGE SLOPES, DETAILED DIMENSIONS FOR DOORS, WINDOWS AND OTHER OPENINGS ETC.
- SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR SLEEVES, NAILERS, INSERTS, ETC. TO BE ENCASED IN CONCRETE.
- THE GENERAL CONTRACTOR SHALL REVIEW ALL THE DRAWINGS AND CHECK DIMENSIONS BEFORE CONSTRUCTION. REPORT DISCREPANCIES BETWEEN STRUCTURAL AND OTHER DISCIPLINES DRAWINGS FOR CLARIFICATION.
- DO NOT CUT OR DRILL ANY OPENINGS IN STRUCTURAL MEMBERS WITHOUT THE WRITTEN PERMISSION OF TOWER ENGINEERING GROUP. CONTRACTOR TO PROVIDE APPROPRIATE ATTACHMENTS AND CONNECTIONS FOR MECHANICAL, ELECTRICAL, AND OTHER SERVICES WITHOUT CUTTING OR DRILLING.
- REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND LANDSCAPE DRAWINGS FOR LOCATIONS, CONFIGURATIONS, EXTENT, AND SIZES OF ALL CURBS, UPSTANDS, DOWNTURNS; AND FOR OPENINGS THROUGH FLOORS AND WALLS FOR DUCTS.
- FIRE RESISTANCE RATINGS:

SEE ARCHITECTURAL DRAWINGS AND SPECIFICATION FOR PRECISE LOCATION OF REQUIRED FIRE RESISTANCE RATINGS.
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- THE GENERAL CONTRACTOR SHALL PROVIDE REASONABLE NOTICE TO TOWER ENGINEERING GROUP PRIOR TO POURING CONCRETE OR CONCEALING ANY STRUCTURAL COMPONENTS. THE PURPOSE OF THIS NOTICE IS TO ENABLE TOWER ENGINEERING GROUP TO CONDUCT ANY REQUIRED FIELD REVIEWS.
- THE GENERAL CONTRACTOR SHALL FAMILIARIZE HIMSELF WITH THE CONTENT AND RECOMMENDATIONS OF THE GEOTECHNICAL REPORTS.
- THE GENERAL CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR ALL STRUCTURAL COMPONENTS TO TOWER ENGINEERING GROUP FOR REVIEW AND APPROVAL PRIOR TO FABRICATION. SHOP DRAWINGS TO INCLUDE SEAL AND SIGNATURE OF A PROFESSIONAL ENGINEER FOR DESIGN OF COMPONENTS AND/ OR CONNECTIONS AS REQUIRED.

LIST OF STRUCTURAL DRAWINGS

S1.0	GENERAL NOTES
S2.0	FOUNDATION PLAN/ BASEMENT SLAB PLAN
S3.0	MAIN FLOOR/ MEZZANINE FLOOR FRAMING PLAN
S4.0	ROOF FRAMING PLAN
S5.0	FOUNDATION/ MAIN FLOOR SECTIONS
S5.1	MAIN FLOOR SECTIONS
S6.0	MEZZANINE/ ROOF SECTIONS



2 TYPICAL FLAG POLE BASE
SCALE: 1:25

FLAG POLE BASE

POLE HT.	DIAMETER	DEPTH BELOW FINISH GRADE	REINFORCEMENT
TO 7.500 M	600mm#	1.800 M	6-20M VERT. 10M TIES @300mm
TO 9.500 M	750mm#	2.400 M	10-20M VERT. 10M TIES @300mm
TO 12.000 M	750mm#	3.000 M	10-20M VERT. 10M TIES @300mm

ORIGINAL STAMPED BY: N. MARTINEZ, P. ENG.
DATE: 2006.07.17

NO.	REVISION/DESCRIPTION	BY	DATE
2.	ISSUED FOR TENDER	NM	JULY 17/06
1.	ISSUED FOR CLIENT REVIEW	NM	JUNE 26/06

CONSULTANT

TOWER PROJECT NO. : 6079

TOWER ENGINEERING GROUP

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SEALS

APEGM

Certificate of Authorization

Tower Engineering Group Inc.

No. 1918 Expiry: April 30, 2007

DRAWN BY	SM	CHECKED BY	APPROVED
DATE	2006.07.17	USER APPROVAL	

WINNIPEG

CITY OF WINNIPEG
PLANNING, PROPERTY &
DEVELOPMENT DEPARTMENT
CIVIC ACCOMMODATIONS DIVISION
300 - 65 GARRY ST. R3C 4K4

PROJECT
WEST TRANSCONA
FIRE PARAMEDIC STATION 21

1446 REGENT AVENUE W

SHEET TITLE

GENERAL NOTES

METRIC
WHOLE NUMBERS ARE IN MILLIMETRES.
DECIMALIZED NUMBERS ARE IN METRES

SCALE	PROJECT NO.	SHEET NO.
AS SHOWN	2004-058	S1.0

DRAWING SHEET SIZE: A1 (841mm x 594mm) PLOT 1:1