

PART E
SPECIFICATIONS

PART E - SPECIFICATIONS

GENERAL

E1. APPLICABLE SPECIFICATIONS, STANDARD DETAILS AND DRAWINGS

- E1.1 *The City of Winnipeg Standard Construction Specifications* in its entirety, whether or not specifically listed on Form B: Prices, shall apply to the Work.
- E1.1.1 *The City of Winnipeg Standard Construction Specifications* is available in Adobe Acrobat (.pdf) format on the Information Connection page at The City of Winnipeg, Corporate Finance, Materials Management Division internet site at <http://www.winnipeg.ca/matmgt>.
- E1.1.2 The version in effect three (3) Business Days before the Submission Deadline shall apply.
- E1.1.3 Further to GC:2.4(d), Specifications included in the Bid Opportunity shall govern over *The City of Winnipeg Standard Construction Specifications*.
- E1.2 The following Drawings are applicable to the Work:

| <u>Drawing No.</u> | <u>Drawing</u> |
|--------------------|--|
| 05399 | Birchdale Ave. – 1 st MH N. of Coniston St. to MH at Highfield St. (CL) |
| 05400 | Aikins St. – MH at Bannerman Ave. to MH at Cathedral Ave. |
| 05401 | Wall St. – 3 rd MH S. of Notre Dame Ave. to 2 nd MH S. of Notre Dame Ave. |
| 05402 | Luxton Ave. (ROW) – 4 th MH E. of Main St. to MH at St. Cross St. |
| 05403 | Birchdale Ave. – MH at Lyndale Dr. to 1 st MH N. of Lyndale Dr. |
| 05404 | Montague Ave. – 1 st MH W. of Nassau St. S. to MH at Churchill Dr. |
| 05405 | Birchdale Ave. – 2 nd MH N. of Lyndale Dr. to MH at Coniston St. (CL) |
| 05406 | St. James St. – 2 nd MH N. of Wellington Ave. to 1 st MH N. of Wellington Ave. |
| 05407 | Lyndale Dr. (CL) – MH at Lane Between Balsam and Beechwood to MH at Highfield (CL) |
| 05408 | Tache Ave. – MH at Kitson St. (S. of CL) to MH at Hanbury St. |
| 05409 | Magnus Ave. – MH at McNichol St. to 1 st MH E. of McNichol St. |
| 05410 | Arlington St. – 1 st MH N. of College Ave. to MH at Mountain Ave. (S. PL) |
| 05411 | Dubuc St. – MH at Hill St. to MH at Braemar St. (E. of CL) |
| 05412 | Lyndale Dr. – 1 st MH S. of Marion Pl. to MH at Marion Pl. |
| 05413 | Marion St. – MH at Des Meurons St. to Node at Enfield Cres. |

E2. CURED-IN-PLACE-PIPE (CIPP)

- E2.1 DESCRIPTION
- E2.1.1 This specification covers the supply and installation of full segment, partial full segment (blind shot) and internal point repairs using cured-in-place pipe (CIPP).
- E2.2 DEFINITIONS
- E2.2.1 Cured-in-place-pipe (CIPP) means trenchless sewer rehabilitation by installing a resin-felt composite structure which when cured will form a continuous-close fit liner within an existing sewer.
- E2.2.2 Approved CIPP Suppliers and Installers means suppliers and installers pre-approved under City of Winnipeg "Request for Qualifications for the Supply and Installation of Cured in Pipe (CIPP)". A list of pre-approved CIPP suppliers and installers for 2004 is included in the Specifications.
- E2.2.3 Full segment CIPP means CIPP extending from manhole to manhole or manhole to node (wee or tee connection to another sewer).

- E2.2.4 Partial full segment CIPP means CIPP extending from a manhole to an intermediate point within the sewer and shall generally be longer than ten metres in length.
- E2.2.5 Internal point repair CIPP means CIPP a short length or multiples of short length CIPP to repair localized defects anywhere within a sewer or sewer service. Internal point repairs are generally be one to ten metres in length.
- E2.2.6 Minimum material requirements for CIPP shall conform to ASTM D5813-95 “Standard Specification for Cured-In-Place Thermosetting Resin Sewer Pipe” and the supplemental requirements noted herein.

E2.3 MATERIALS

E2.3.1 Pre-Approved CIPP Suppliers and Installers and Materials

- (a) The following is a list of sewer lining systems – suppliers and installers and materials that have been pre-approved under the City of Winnipeg “Request for Qualifications for the Supply and Installation of Cured in Place Pipe (CIPP), Bid Opportunity No. 82-2004” for 2004 City of Winnipeg sewer rehabilitation projects.

Table E2.3.1a): Pre-Approved CIPP Suppliers and Installers

| <i>Applicant</i> | <i>Insituform Technologies Limited</i> | <i>Capital Commercial Pipe Services</i> | <i>Nelson River Construction Inc.</i> |
|-------------------------|---|--|--|
| Contact | Ken Foster 780-413-0200 | Brian Ratchford 905-522-0522 | Gordon Lee 204-949-8700 |
| Supplier | Insituform Technologies Inc. | Capital Commercial Pipe Services | C.I.P.P. Corporation |
| Installer | Insituform Technologies Limited | Capital Commercial Pipe Services | Municipal Pipe Tool Inc. |
| Liner Name | Standard Insituform® CIPP | Capital Lining System | C.I.P.P. Corp Liner |

E2.3.2 CIPP Design Objectives

- (a) Design objectives for CIPP include.
 - (i) Maximizing the structural enhancement of the sewer by installing a close-fit CIPP.
 - (ii) Providing no impact or increasing the hydraulic capacity of the rehabilitated sewer.
 - (iii) Reducing infiltration and exfiltration.
 - (iv) Preventing root intrusion.
 - (v) Providing sufficient chemical resistance to prevent further sewer pipe degradation related to the conveyance of sewage.
 - (vi) Minimizing sewer service disruption during rehabilitation.
 - (vii) Minimizing the time required to complete the sewer rehabilitation.
 - (viii) Minimizing disturbance to pavements and boulevards.
 - (ix) Minimizing disruption to vehicular and pedestrian traffic.
 - (x) Minimizing the impact of construction on commercial, industrial, and institutional facilities.
- (b) Additional design objectives for internal point repair CIPP include.

- (i) Providing a smooth transition between the internal point repair CIPP and the host pipe to prevent the build-up of solids and minimize wear on the repair due to routine sewer cleaning and other maintenance activities.
 - (ii) Filling any existing voids outside the sewer at the point of repair.
- (c) Select CIPP and plan approach to rehabilitation toward maximizing the achievement of these design objectives.

E2.3.3 CIPP Design – General

- (a) Design full segment and partial full segment CIPP in accordance with Appendix X1 of ASTM F1216 and these specifications as a gravity pipe in a partially or fully deteriorated pipe condition in accordance with design conditions noted in the Drawings and Specifications.
- (b) Design internal point repair CIPP in accordance with Appendix X1 of ASTM F1216 as a gravity pipe in a fully deteriorated pipe condition and the depth of cover calculated based on the specific location of the repair in the sewer or sewer service.
- (c) Size CIPP in accordance with the design objectives to provide a close-fit to the host pipe with no annulus except for the maximum allowable diametric shrinkage due to curing permitted in ASTM D5813.
- (d) Perform a design check to confirm the full flow hydraulic capacity of the CIPP will be equal to or greater than the existing sewer. Use “Manning's” formula with assumed ‘n’ value of 0.012 for the CIPP and an “n” value for the existing section estimated on the observed condition of the pipeline from the Sewer Maintenance Inspection.
- (e) Design features of internal point repair CIPP are to also include.
 - (i) Tapered end sections to promote a smooth transition from the repair to the host pipe.
 - (ii) A means to facilitate flow through by-pass the existing dry weather flow during the course of the repair.

E2.3.4 CIPP Design - Partially Deteriorated Condition

- (a) Design CIPP for partially deteriorated pipe condition in accordance with Appendix X1 of ASTM F1216 and the following minimum design checks.
 - (i) Determine wall thickness by restrained buckling analysis.
 - (ii) Determine whether wall thickness will be governed by long-term flexural stress.
 - (iii) Determine whether any localized thickening is required for missing segments or holes in the host pipe.
- (b) Perform supplemental design checks where the host pipe has invert “flats” to determine whether wall thickness will be governed by one of the following:
 - (i) Buckling by assuming the flat functions as a pin-ended strut.
 - (ii) Stress, by assuming the flat functions as a pinned member, subjected to axial and transverse loads.
 - (iii) Deflection by assuming that allowable deflection is limited to 3% of the length of the flat.
- (c) Use the following minimum design assumptions.
 - (i) Groundwater table is 2.0 m below the existing ground surface.
 - (ii) An enhancement factor (K) of 7.
 - (iii) Long-term values for flexural modulus of elasticity and flexural strength will be considered to be the projected value at 50 years of a continuous application of the design load based on the specific resin and felt composite approved for use in the pre-qualification process.

- (iv) Minimum value for ovality of the existing sewer will be 3% unless a greater value is indicated in the contract specifications or as determined from observation of the maintenance inspection.
- (v) Minimum factor of safety (N) of 2 for restrained buckling analysis.

E2.3.5 CIPP Design – Fully Deteriorated Condition

- (a) Design CIPP for fully deteriorated pipe condition in accordance with Appendix X1 of ASTM F1216 and the following minimum design assumptions.
 - (i) Include an allowance for an AASHTO HSS25 concentrated live load in the total external pressure on the pipe. Calculate minimum live load surcharge based on Cooper E80 distributed load for portions of CIPP installed under railway lines.
 - (ii) Calculate dead load based on soil density of 1920 kg/m^3 .
 - (iii) Groundwater table is 2.0 m below the existing ground surface.
 - (iv) Minimum value for ovality of the existing sewer will be 2% unless a greater value is indicated in the contract specifications or as determined from observation of the maintenance inspection.
 - (v) Long-term value for flexural modulus of elasticity will be considered to be the projected value at 50 years of a continuous application of the design load based on the specific resin and felt composite as established by ASTM D2990 and approved for use in the pre-qualification process.
 - (vi) Modulus of soil reaction ($E's$) will be assumed to be 6900 kPa unless a higher or lower value is indicated in the contract specifications.
 - (vii) Minimum factor of safety (N) of 2.

E2.3.6 Existing Sewer Design Conditions

- (a) The assessment of liner system design conditions and site-specific repairs required to accommodate lining were based on the conditions observed from sewer inspections that were performed in 2000 – 2003 as part of the City of Winnipeg's Sewer Cleaning and Inspection Programs. Copies of these video inspections are available to the Contractor in digital format on CDs.
- (b) The Contractor shall be aware the video inspections provided were completed immediately after sewer cleaning and the amount of sediment and debris present at the time of this Bid Opportunity may not be the same. The Contractor shall be responsible to determine the actual amount of sediment and debris in the sewers included in this Work.
- (c) The following specific design conditions and site specific repair requirements apply to the Work.

Table E2.3.6: Specific Design Conditions and Site Specific Repairs

| Birchdale Avenue (Sewer ID 20029) | 1st MH N. of Coniston St. (MH ID 20028) to MH at Highfield St. (MH ID 20030) |
|---|--|
| Size/Shape | 450mm Circular |
| Material | Concrete |
| Total Length | 86.3m |
| Sewer Depth to Invert – maximum | 4.61m |
| Deformation/Ovality | 5.0% Partially Deteriorated, 3.0% Fully Deteriorated |
| Design Condition MH 20030 to MH 20028 Chainage – from 0.0m to 17.8m Chainage – from 17.8m to 33.3m | Partially Deteriorated Fully Deteriorated |
| Site Specific Repairs | |
| Sewer Defect Location/Type | Required Action |
| MH 20028 to MH 20030 53.9m – ELJ (03 to 04) | Remove encrustation |
| MH 20030 to MH 20028 2.7m – EL (03 to 05) | Remove encrustation |
| 2.9m – EL (07 to 10) | Remove encrustation |
| 4.2m – ELJ (02 to 04) | Remove encrustation |
| 5.2m – ELJ (07 to 10) | Remove encrustation |
| 7.0m to 8.1m – EL (07 to 05) | Remove encrustation |
| 9.9m – ELJ (07 to 09) | Remove encrustation |
| 10.8m – ELJ (07 to 09) | Remove encrustation |
| 12.6m – ELJ (07 to 09) | Remove encrustation |
| 13.6m – EMJ (07 to 09) | Remove encrustation |
| 14.5m – EMJ (03 to 05 & 07 to 09) | Remove encrustation |
| 15.5m – EMJ (02 to 04 & 07 to 09) | Remove encrustation |
| 16.0m – EL (07 to 09) | Remove encrustation |
| 16.4m – EM (02 to 04 & 07 to 09) | Remove encrustation |
| 17.4m – EMJ (03 to 05 & 07 to 09) | Remove encrustation |
| 18.4m – EMJ (04 to 05) | Remove encrustation |
| 20.4m – CNI (02) | Remove intruding connection |
| 20.5m – CNI (10) | Remove intruding connection |
| 22.2m – OB (03) | Remove rag |
| 22.4m – OB (06) | Remove stick |
| 23.6m – EL (08) | Remove encrustation |
| 24.8m – ELJ (07 to 09) | Remove encrustation |
| 25.7m to 26.3m – EM (07 to 09) | Remove encrustation |

| Aikins Street (Sewer ID 18184) | MH at Bannerman Ave. (MH ID 18170) to MH at Cathedral Ave. (MH ID 18185) |
|---|---|
| Size/Shape | 600mm Circular |
| Material | Concrete |
| Total Length | 101.2m |
| Sewer depth to invert – maximum | 4.85m |
| Deformation/Ovality | 7.0% Partially Deteriorated, 6.0% Fully Deteriorated |
| Design Condition MH 18170 to MH 18185 Chainage – from 0.0m to 73.0m Chainage – from 73.0m to 80.0m Chainage – from 80.0m to 101.2m | Partially Deteriorated Fully Deteriorated Partially Deteriorated |
| Site Specific Repairs | |
| Sewer Defect Location/Type | Required Action |
| MH 18170 to MH 18185 3.0m – CNI (01) 31.7m to 32.8m – EL (07 to 10) 45.6m – EM (02 to 05) 46.8m to 47.5m – EM (07 to 10) 47.6m – EM (02 to 04) 47.8m to 48.5m – EL (07 to 11) 52.2m – CNI (10) 64.1m – EL (09) 73.4m to 75.4m – H (07 to 11) 78.2m – OB (08) 80.4m – EL (08 to 09) | Remove intruding connection Remove encrustation Remove encrustation Remove encrustation Remove encrustation Remove encrustation Remove encrustation Remove intruding connection Remove encrustation Repair concrete wall prior to lining Remove steel that has peeled away Remove encrustation |

| Wall Street (Sewer ID 9345) | 3rd MH S. of Notre Dame Ave. (MH ID 9346) to 2nd MH S. of Notre Dame Ave. (MH ID 9347) |
|--|--|
| Size/Shape | 450mm Circular |
| Material | Concrete |
| Total Length | 97.5m |
| Sewer depth to invert - maximum | 3.86m |
| Deformation/Ovality | 3.0% Partially Deteriorated, 2.0% Fully Deteriorated |
| Design Condition MH 9346 to MH 9347 Chainage – from 0.0m to 33.1m Chainage - from 33.1m to 38.0m Chainage – from 38.0m to 64.1m Chainage – from 64.1m to 64.7m Chainage – from 64.7m to 97.5m | Partially Deteriorated Fully Deteriorated Partially Deteriorated Fully Deteriorated Partially Deteriorated |
| Site Specific Repairs | |
| Sewer Defect Location/Type | Required Action |
| MH 9346 to MH 9347 0.0m to 1.2m – DE 6.2m – JN (10) 8.1m to 10.3m – DE 22.4m to 32.2m – DE 35.5m to 38.0m – DE 44.4m to 48.1m - DE | Remove debris Grout void under service Remove debris Remove debris Remove debris Remove debris |
| CB Lead Defect Location/Type | Required Action |
| 2 nd CB S. of 2 nd MH S. of Notre Dame Ave. (E. of CL) to Node at Wall St. (CL) Chainage – 0.0m 1 st CB S. of 2 nd MH S. of Notre Dame Ave. (E. of CL) to Node at Wall St. (CL) Chainage – 0.0m | Grout CB lead at CB Grout CB lead at CB |

| Luxton Avenue (ROW) (Sewer ID 17256) | 4th MH E. of Main St. (MH ID 17255) to MH at St. Cross St. (CL) (MH ID 17257) |
|--|--|
| Size/Shape | 375mm Circular |
| Material | Vitrified Clay |
| Total Length | 111.3m |
| Sewer depth to invert - maximum | 4.46m |
| Deformation/Ovality | 7.0% Partially Deteriorated, 11.0% Fully Deteriorated |
| Design Condition MH 17255 to MH 17257 Chainage - from 0.0m to 0.6m Chainage – from 0.6m to 6.0m MH 17257 to MH 17255 Chainage – from 0.0m to 39.5m Chainage – from 39.5m to 41.5m Chainage – from 41.5m to 105.3m | Fully Deteriorated Partially Deteriorated Partially Deteriorated Fully Deteriorated Partially Deteriorated |
| Site Specific Repairs | |
| Sewer Defect Location/Type | Required Action |
| MH 17257 to MH 17255 65.9m – CXI (03) 67.1m – CNI (02) 99.0m – OB (05) | Remove intruding connection Remove intruding connection Remove piece of pipe |

| Birchdale Avenue (Sewer ID 20019) | MH at Lyndale Dr. (MH ID 20020) to 1st MH N. of Lyndale Dr. (MH ID 20021) |
|---|---|
| Size/Shape | 300mm Circular |
| Material | Concrete |
| Total Length | 96.3m |
| Sewer depth to invert - maximum | 3.50m |
| Deformation/Ovality | 10.0% Partially Deteriorated |
| Design Condition MH 20020 to MH 20021 Chainage – from 0.0m to 29.3m | Partially Deteriorated |
| Site Specific Repairs | |
| Sewer Defect Location/Type | Required Action |
| None | None |

| | |
|-----------------------------------|-----------------------------|
| 26.9m – EL (07 to 03) | Remove encrustation |
| 32.3m to 32.6m – EL (01 to 08) | Remove encrustation |
| 39.9m – EL (03 to 05) | Remove encrustation |
| 40.9m – ELJ (02 to 05) | Remove encrustation |
| 41.8m – ELJ (03 to 05) | Remove encrustation |
| 52.9m – ELJ (05 & 08) | Remove encrustation |
| 57.4m – RFJ (10) | Remove roots |
| 58.3m – ELJ (03 to 05) | Remove encrustation |
| 59.2m – RFJ (03) | Remove roots |
| 59.2m – ELJ (09) | Remove encrustation |
| 62.0m – RMJ (03) | Remove roots |
| 62.0m – ELJ (08) | Remove encrustation |
| 63.5m – CNI (02) | Remove intruding connection |
| 63.5m – RM (01 to 02) | Remove roots |
| 63.8m – EL (08) | Remove encrustation |
| 63.9m – RMJ (03) | Remove roots |
| 65.7m – RFJ (08) | Remove roots |
| 66.6m – RFJ (09) | Remove roots |
| 68.5m – RFJ (03 & 09 to 11) | Remove roots |
| 69.4m – RFJ (02 & 09) | Remove roots |
| 69.8m – EL (03 to 05) | Remove encrustation |
| 70.4m – ELJ (03 & 09) | Remove encrustation |
| 71.3m – RFJ (03) | Remove roots |
| 72.2m – RFJ (01 to 04 & 08 to 10) | Remove roots |
| 73.1m – RFJ (03 & 09) | Remove roots |
| 74.0m – RMJ (03 & 09) | Remove roots |
| 75.0m – RMJ (03) | Remove roots |
| 75.8m – RMJ (03 & 09) | Remove roots |
| 76.8m – RMJ (09) | Remove roots |
| 79.6m – RFJ (03) | Remove roots |
| 81.3m – RMJ (03 & 09 to 11) | Remove roots |
| 82.3m – RMJ (03) | Remove roots |
| 86.9m – RFJ (03) | Remove roots |
| 90.9m to 91.3m – EL (01 to 04) | Remove encrustation |
| 92.2m – EL (03 to 05) | Remove encrustation |

| St. James Street (Sewer ID 18951) | 2nd MH N. of Wellington Ave. (MH ID 18944) to 1st MH N. of Wellington Ave. (MH ID 18952) |
|--|---|
| Size/Shape | 375mm Circular |
| Material | Concrete |
| Total Length | 87.3m |
| Sewer depth to invert - maximum | 4.04m |
| Deformation/Ovality | 4.0% Partially Deteriorated, 2.0% Fully Deteriorated |
| Design Condition MH 18944 to MH 18952 Chainage – from 0.0m to 3.0m Chainage – from 50.1m to 51.1m Chainage – from 51.1m to 87.3m | Fully Deteriorated Fully Deteriorated Partially Deteriorated |
| Site Specific Repairs | |
| Sewer Defect Location/Type | Required Action |
| MH 18944 to MH 18952 0.8m – DE 2.7m – OB (03) 75.9m – ELJ (07 to 09) 78.7m – EL (07 to 10) | Remove piece of pipe at invert Trim off bolt Remove encrustation Remove encrustation |

| Lyndale Drive (CL) (Sewer ID 19990) | MH at Lane Between Balsam and Beechwood (MH ID 19989) to MH at Highfield (CL) (MH ID 19991) |
|--|--|
| Size/Shape | 300mm Circular |
| Material | Concrete |
| Total Length | 91.7m |
| Sewer depth to invert - maximum | 3.70m |
| Deformation/Ovality | 3.0% Partially Deteriorated, 7.0% Fully Deteriorated |
| Design Condition MH 19989 to MH 19991 Chainage – from 0.0m to 2.0m Chainage – from 21.5m to 24.5m | Partially Deteriorated Fully Deteriorated |
| Site Specific Repairs | |
| Sewer Defect Location/Type | Required Action |
| MH 19989 to MH 19991 23.7m – CNI (03) | Remove intruding connection |

| | |
|--|---|
| Tache Avenue (Sewer ID 19816) | MH at Kitson St. (S. of CL) (MH ID 19815) to MH at Hanbury St. (MH ID 19817) |
| Size/Shape | 375mm Circular |
| Material | Vitrified Clay |
| Total Length | 100.5m |
| Sewer depth to invert - maximum | 2.82m |
| Deformation/Ovality | 10.0% Fully Deteriorated |
| Design Condition MH 19815 to MH 19817 Chainage – from 0.0m to 1.0m | Fully Deteriorated |
| Site Specific Repairs | |
| Sewer Defect Location/Type | Required Action |
| None | None |

| | |
|--|---|
| Magnus Avenue (Sewer ID 14412) | MH at McNichol St. (MH ID 14411) to 1st MH E. of McNichol St. (MH ID 20172) |
| Size/Shape | 375mm Circular |
| Material | Vitrified Clay |
| Total Length | 92.3m |
| Sewer depth to invert - maximum | 4.93m |
| Deformation/Ovality | 2.0% Fully Deteriorated |
| Design Condition MH 14411 to MH 20172 Chainage – from 88.2m to 89.4m | Fully Deteriorated |
| Site Specific Repairs | |
| Sewer Defect Location/Type | Required Action |
| None | None |

| | |
|--|--|
| Arlington Street (Sewer ID 17773) | 1st MH N. of College Ave. (MH ID 17772) to MH at Mountain Ave. (S. PL) (MH ID 17774) |
| Size/Shape | 300mm Circular |
| Material | Vitrified Clay |
| Total Length | 47.3m |
| Sewer depth to invert - maximum | 5.58m |
| Deformation/Ovality | 2.0% Fully Deteriorated |
| Design Condition MH 17772 to MH 17774 Chainage – from 29.0m to 30.0m | Fully Deteriorated |
| Site Specific Repairs | |
| Sewer Defect Location/Type | Required Action |
| None | None |

| Dubuc Street (Sewer ID 19569) | MH at Hill St. (MH ID 8713) to MH at Braemar St. (E. of CL) (MH ID 19570) |
|---|--|
| Size/Shape | 450mm Circular |
| Material | Vitrified Clay |
| Total Length | 96.6m |
| Sewer depth to invert - maximum | 5.91m |
| Deformation/Ovality | 3.0% Partially Deteriorated |
| Design Condition MH 19570 to MH 8713 Chainage – from 13.5m to 14.4m | Partially Deteriorated |
| Site Specific Repairs | |
| Sewer Defect Location/Type | Required Action |
| None | None |

| Lyndale Drive (Sewer ID 20138) | 1st MH S. of Marion Pl. (MH ID 20137) to MH at Marion Pl. (MH ID 20139) |
|--|---|
| Size/Shape | 375mm Circular |
| Material | Vitrified Clay |
| Total Length | 82.4m |
| Sewer depth to invert - maximum | 7.04m |
| Deformation/Ovality | 3.0% Partially Deteriorated |
| Design Condition MH 20137 to MH 20139 Chainage – from 54.2m to 56.0m Chainage – from 80.0m to 82.3m | Partially Deteriorated Partially Deteriorated |
| Site Specific Repairs | |
| Sewer Defect Location/Type | Required Action |
| None | None |

| Marion Street (Sewer ID 19439) | MH at Des Meurons St. (MH ID 19440) to Node at Enfield Cres. (MH ID 19441) |
|--|---|
| Size/Shape | 300mm Circular |
| Material | Vitrified Clay |
| Total Length | 76.2m |
| Sewer depth to invert - maximum | 4.89m |
| Deformation/Ovality | 4.0% Partially Deteriorated |
| Design Condition MH 19440 to MH 19441 Chainage – from 0.0m to 2.7m | Partially Deteriorated |
| Site Specific Repairs | |
| Sewer Defect Location/Type | Required Action |
| None | None |

E2.3.7

Submittals Before Starting Work

- (a) Provide the required submittals to the Contract Administrator a minimum of 10 days before starting the lining.
- (b) Submit the CIPP design Shop Drawings in accordance with CW1110 and sealed and signed by a Professional Engineer licensed to practice in the Province of Manitoba. Include the following information.

- (i) CIPP thickness computations including all specified design checks. Identify design assumptions based on a review of the Sewer Maintenance Inspection that differ from the information provided in the Specifications for the existing sewer design conditions.
 - (ii) Calculations showing the hydraulic capacity of the CIPP sewer will be equal to or greater than the existing sewer.
 - (iii) Name and manufacturer of the resin and tube proposed for each CIPP.
 - (iv) CIPP curing schedule provided by the resin supplier indicating the temperature, staging, duration and pressure required to achieve a proper cure of the resin and fabric tube composite.
 - (v) Other information that may reasonably be required by the Contract Administrator to confirm the CIPP design proposed conforms to the specified requirements and design intent.
- (c) Resin Samples
- (i) Arrange for the manufacturer of the resin to forward a reference sample of each resin proposed for use on the Works to a test laboratory designated by the Contract Administrator to be used as a comparative reference sample for infrared spectrum testing.
 - (ii) Deliver a representative sample of each resin to be used on the project before adding the catalyst from the wet-out facility to a test laboratory designated by the Contract Administrator.
 - (iii) The Contract Administrator will arrange and pay for an infrared analysis of the samples.
- (d) Submit an operations protocol that provides information on the following.
- (i) Resin impregnation method.
 - (ii) Designated location of the wet out facility.
 - (iii) Documentation the resin to be used has not exceeded its shelf life as recommended by the manufacturer of the resin.
 - (iv) Volume of resin to be impregnated into each liner and repair section including the proposed excess allowance for polymerization and migration (typically 7%) into cracks and joints of the host pipe.
 - (v) Roller gap setting required to provide the final installed CIPP thickness based on the proposed volume of resin.
 - (vi) Details of the wet-out procedure for internal point repair CIPP.
- (e) Submit a construction protocol that provides information on the following.
- (i) Proposed main line and sewer service flow control arrangements.
 - (ii) Minimum pressure to hold the tube tight against the existing sewer and the maximum pressure to not damage the sewer or uncured liner.
 - (iii) Provide the maximum allowable axial and longitudinal tensile stress for the fabric tube and the arrangement for monitoring pull-in forces during installation if liner insertion is to be by pull-in methods.
 - (iv) Number and location of heat source monitor gauges.
 - (v) Minimum and maximum allowable temperature during each phase of the cure period as measured at the heat source return line.
 - (vi) Number of stages and anticipated time for each stage of the curing period based on resin supplier's recommendations.
 - (vii) Estimated length of time required to reinstate the main line sewer and sewer services.
- (f) Provide the following additional information for internal point repair CIPP.

- (i) Limiting capacity of the flow through by-pass piping.
- (ii) Details of the internal point repair CIPP installation method.
- (iii) Means of curing such as ambient, steam or hot water and quality assurance procedures in-place to determine curing requirements are achieved.
- (iv) Estimated length of time for installation of the internal point repair and to reinstate service services.

E2.4 CONSTRUCTION METHODS

E2.4.1 Verification of Existing Sewer Dimensions

- (a) Verify dimensional requirements of each sewer to be rehabilitated prior to manufacture of the CIPP tube as follows.
 - (i) Length of sewer from manhole to manhole for full segment and partial full segment CIPP.
 - (ii) Diameter and cross-section of the sewer at the upstream and downstream manholes and at a minimum distance of 500 millimetres inside the sewer from each manhole.
 - (iii) Use calibrated callipers or other suitable measuring device capable of measuring accurately to +/- 1 millimetre to confirm cross section geometry at clock positions of:
 - 12:00 to 6:00,
 - 2:00 to 8:00,
 - 3:00 to 9:00 and
 - 4:00 to 10:00.
 - (iv) Estimate the remainder of the sewer dimensional requirements based on dimensional checks and the Sewer Maintenance Inspections.
- (b) Obtain additional measurements for large diameter (larger than 600 millimetres) and for non-circular sewers sufficient to define the cross section to meet the design objective of manufacturing and installing a close-fit liner without annulus, including but not limited to.
 - (i) The length of the inside perimeter (circumference) of the sewer at the upstream and downstream end.
 - (ii) Continuous or discontinuous (every 5 metres) measurement of the height and width of the sewer along the entire length of the sewer. The actual measurements and distance of the measurements from the upstream manhole are to be visible and recorded on a Pre-Design Inspection and the Post Lining Inspection.

E2.4.2 Sewer Cleaning

- (a) Remove loose and solid debris and intruding connections in accordance with CW 2140 to adequately prepare the sewer for lining.

E2.4.3 Sewer Inspections

- (a) Perform the following sewer inspections in accordance with CW 2145 in the presence of the Contract Administrator.
 - (i) Pre-Sewer Repair Inspection, where required, before starting any excavation. No coding submission will be required.
 - (ii) Pre-Design Inspection, where required, prior to preparing the CIPP design. No coding of the submission will be required.

- (iii) Pre-Lining Inspection after sewer cleaning and preparation. No coding of the submission will be required.
 - (iv) Post-Lining Inspection subsequent to installing the CIPP and sewer service reinstatement. Full coding required.
 - (v) Warranty Inspection before expiration of the warranty period and acceptance. Full coding required.
- (b) Review the Pre-Sewer Repair Inspection video with the Contract Administrator before starting the repair work to confirm the extent and precise location of external sewer repairs.
- (c) Review the Pre-Design Inspection video to confirm the height and width of sewers larger than 600 millimetres in diameter and non-circular sewers.
- (i) Provide a copy of the video to the Contract Administrator.
 - (ii) Advise the Contract Administrator of any condition that is contrary to the design conditions or assumptions made that may affect either long or short term performance of the CIPP prior to liner design.
- (d) Review the Pre-Lining Inspection videotape with the Contract Administrator at least 24 hours before installing the CIPP and obtain approval to install the CIPP. The Pre-Lining Inspection shall confirm:
- (i) Necessary cleaning and pipe preparation work, including internal and external sewer repairs, have been satisfactorily completed.
 - (ii) Condition of the sewer pipe is consistent with the design conditions and the Specifications. Advise the Contract Administrator of any condition that is contrary to the design conditions or assumptions made that may affect either long or short term performance of the CIPP prior to commencing lining.
 - (iii) Location, condition and operational status of all sewer services.
 - (iv) The limit and precise location for each internal point repair.
 - (v) Review Sewer Service Reports while reviewing the Pre-Lining Inspection.
- (e) Post-Lining Inspection is to confirm the adequacy of sewer service reinstatements and the fit and finish of the CIPP including continuous or discontinuous (every 5 metres) measurement of the height and width of large diameter and non-circular sewers along the entire length of the sewer. The actual measurements and distance of the measurements from the upstream manhole are to be visible and recorded on the Post Lining Inspection.
- (f) Warranty Inspection to confirm the fit and finish of the CIPP, need for any remedial work and acceptance of any repair work performed during the warranty period. Sewer cleaning in accordance with CW 2140 is required to obtain a satisfactory inspection.

E2.4.4 Sewer Service Report

- (a) Confirm exact location of all sewer services connected to the sewer being lined by dye testing methods.
- (b) Submit a written Sewer Service Report for each CIPP location to the Contract Administrator providing the following information for each sewer service.
 - (i) Operational status of each sewer service (live or out of service). For live services indicate the property address of structure serviced.
 - (ii) Distance from the upstream manhole in metres and clock reference of the connection position to the sewer.
 - (iii) Diameter in millimetres.
 - (iv) Material type.
 - (v) Observed condition of the sewer service.

E2.4.5 Flow Control

- (a) Provide necessary flow control measures for the main line sewer and sewer services required to perform the Work. Diversion of wastewater flow directly or indirectly to the environment, Land Drainage Sewers, or Storm Relief sewers will not be allowed.
- (b) Maintain existing sewer flows from upstream sewers during construction around the sewers being lined.
- (c) Provide adequate temporary bypass pumping for live sewer services connected to the sewer being lined from when the service is blocked off until it is reinstated.
- (d) Provide temporary indoor portable toilets for residential homes and for each apartment in small apartment buildings instead of temporary sewer service bypass pumping where feasible and approved by the building owner and the Contract Administrator. Provide necessary supplies for portable toilets and clean as often as required while in use. Remove portable toilets promptly once sewer service is reinstated.
- (e) Expose sewer services for facilities with a high volume of effluent discharge that have no feasible means of intercepting the flow within the building or at a location outside the building agreed upon by the Contract Administrator and drain or pump the sewer service from that location until the sewer service is reinstated.
- (f) Excavate for sewer service exposure in accordance with CW 2030. Repair and backfill exposed sewer services in accordance with CW 2130.
- (g) Restore the surface in accordance with CW 2130 and the following specifications:
 - (i) Boulevard areas in accordance with CW 3510.
 - (ii) Concrete pavement in accordance with CW 3230.
 - (iii) Asphaltic pavement in accordance with CW 3410.
 - (iv) Concrete sidewalk and interlocking paving stone in accordance with CW 3325 and CW 3330.

E2.4.6 Sewer Preparation and Repairs Prior to Lining

- (a) Perform sewer preparation and repairs as indicated in the specification and drawings.
- (b) Complete the following internal host pipe repairs in accordance with E3 of this specification.
 - (i) Fill holes in and patch deteriorated sections of the host sewer pipe wall.
 - (ii) Fill voids in the surrounding backfill flush with the inside surface of the sewer pipe.
 - (iii) Reshape host sewer pipe invert to the original dimension and cross section at locations where the invert has completely deteriorated.
- (c) Removal of Intruding Sewer Services and Solid Debris Cutting
 - (i) Remove intruding sewer services and solid debris in accordance with CW 2140.
- (d) Sewer Service Grouting
 - (i) Fill voids around sewer services with a non-shrink, watertight cement grout, an appropriate polyurethane grout compound, or other approved grouting product to form a smooth watertight connection.

E2.4.7 External Sewer Repairs

- (a) Complete external sewer repairs as indicated in the Specifications and on the Drawings in accordance with CW 2130, SD-022A and SD-022B.

E2.4.8 Manhole and Catch Basin Repairs

- (a) Complete manhole and catch basin repairs as indicated in the Specifications and Drawings in accordance with CW 2130.
- (b) Remove and replace manhole frames, covers, rungs and risers required to facilitate the CIPP installation in accordance with CW 2130.

E2.4.9 Weather

- (a) Review the Environment Canada weather forecast with the Contract Administrator before starting CIPP lining installation.
- (b) Delay installation of CIPP when the anticipated weather conditions are such that anticipated sewer flow will exceed the flow control measures provided.

E2.4.10 Installation of CIPP

- (a) Install liners by inversion methods in accordance with ASTM F1216 or by pull-in methods in accordance with ASTM F1743-96.
 - (i) Full segment and partial full segment CIPP shall be cured by hot water or steam.
 - (ii) Internal point repair CIPP shall be ambient, hot water or steam cured.
- (b) Carry out workmanship in accordance with ASTM D5813.
- (c) Trim ends of CIPP neatly to fit flush with interior vertical surface and manhole benching and seal to make watertight.
- (d) Fill annular spaces where the CIPP does not make an adequate seal with the host pipe at manholes, termination points and sewer services due to broken or misaligned pipe with a resin mixture compatible with the CIPP.
- (e) Extend limits for internal point repairs a minimum of 300 millimetres in each direction beyond the limits of the defect to be repaired. Extend internal point repairs that terminate at sewer service services a minimum distance of 300 millimetres beyond the limit of the service.
- (f) Ensure termination points of internal point repairs provide a smooth and uniform flow transition to the host pipe for the full circumference of the repair.

E2.4.11 Reinstatement of Sewer Services

- (a) Reinststate live sewer services to 100% of the original cross sectional area.
- (b) Cut out openings for sewer services from inside the lined sewer by manual means or with a television camera and a remote controlled cutting device.
- (c) Remove sharp edges from opening cut outs and provide a smooth rounded lip.
- (d) Sewer Service Grouting
 - (i) Fill voids between the CIPP and the host pipe at sewer service openings with a non-shrink, watertight cement grout or an appropriate polyurethane grout compatible with the liner system, or other approved grouting product to form a smooth watertight connection.
 - (ii) Locations for sewer service grouting shall be identified by the Contract Administrator during review of Post Lining Video Inspection.
 - (iii) If the voids are due to the condition of the existing sewer service and host pipe, sewer service grouting shall be measured and paid for under sewer service grouting – after lining. If the voids are due to the Contractor's method of reinstatement, deficiencies in the CIPP installation, or any other reason related to the Contractor's workmanship or method of operations, they shall be filled at the Contractor's expense.
 - (iv) Repair of defective or incomplete sewer service grouting shall be at the Contractors own expense.

E2.4.12 Sewer Inspection Reports

- (a) Provide the Contract Administrator with the following sewer inspection reports prepared in accordance with CW 2145.
 - (i) Submit pre-sewer repair inspection and pre and post-lining inspection and reports before Total Performance of Work.
 - (ii) Submit a warranty inspection report before Final Acceptance of Work.

E2.4.13 Quality Control Records

- (a) Maintain the following Quality Control records of the Work and provide to the Contract Administrator after completion of the Work.
 - (i) Summary of the resin impregnation process including:
 - Volume of resin supplied.
 - Excess quantity of resin added during the wet out to account for polymerization and migration into the host pipe.
 - Roller gap setting.
 - Resin catalyst(s) used.
 - Time and location of the wet out.
 - Means taken to store and transport the resin impregnated CIPP from the wet out facility to the job site.
 - Means of curing internal point repair liners.
 - (ii) Continuous log of pressure maintained in the liner during the curing period.
 - (iii) Pulling force used to pull or winch CIPP into place in the host sewer and measured liner elongation.
 - (iv) Continuous log of temperature at boiler in and out and at all thermistors placed between the host pipe and the liner at all manholes during the initial cure, cure, and cool down periods.

E2.4.14 Confined Test Samples

- (a) Provide necessary forms of the same diameter as the host pipe and secure a minimum 200 millimetre long full diameter confined test sample from each CIPP and internal point repair.
- (b) Locate the test sample form in an intermediate manhole or at a termination point and invert through the form.
- (c) Cut the CIPP sample to coincide with multi-piece form if used for CIPP larger than 450 millimetres in diameter to facilitate removal from the manhole.
- (d) Identify the location where the liner sample is from on the form and provide to the Contract Administrator intact in the form.
- (e) The Contract Administrator will coordinate and pay for CIPP sample testing to confirm the CIPP flexural strength, flexural modulus and thickness in accordance with the requirements of ASTM D5813, D790, and ASTM D3567.
- (f) If it can be demonstrated that it is impractical to obtain confined test samples due to CIPP size and site specific conditions then results from test plate samples modified in accordance with Clause 3.13 of this specification will be used to confirm flexural strength and flexural modulus.

E2.4.15 Test Plate Samples

- (i) Obtain and provide the Contract Administrator with test plate samples of each CIPP.

- (ii) Prepare test plate samples on-site from the actual CIPP and cure in a clamped mold placed in the downtube or manhole.
- (iii) The Contract Administrator will coordinate and pay for test plate sample testing to confirm the flexural strength, flexural modulus and thickness in accordance with the requirements of ASTM D5813, D790, and D3567.
- (iv) Flexural strength and flexural modulus results obtained from test plates will be reduced by the maximum percentage difference of the confined pipe and test plate samples prepared from the same CIPP system for at least 3 previous CIPP linings on the same project.
- (v) Schedule installation of liners for which confined pipe samples are impractical to obtain after at least 3 other CIPP linings on the same project have been completed and confined pipe and test plate samples have been secured to provide collaborative testing.
- (vi) Obtain and provide the Contract Administrator with pre and post lining measurements taken in accordance with Clause E2.4.1 of this specification to confirm in-place liner thickness.
- (vii) The Contract Administrator will review liner thickness results taken from test plates or unconfined samples on a case-by-case basis.

E2.4.16 Infrared Spectroscopy

- (a) The Contract Administrator will arrange and pay for testing to compare the infrared spectrum of the resin field samples supplied from the wet-out to the reference spectrum generated from the resin sample provided by the resin manufacturer to verify installed material acceptability.

E2.4.17 Post Construction Design Review for Total Performance

- (a) The Contract Administrator will perform a post-construction design review to ensure that the completed CIPP meets the 50 year design life structural requirements prior to Total Performance. The design review will utilize the measured values for flexural strength, flexural modulus, and CIPP thickness from the confined pipe sample testing or the reduced strength/modulus values obtained from the test plate testing in circumstances where confined pipe samples are not able to be secured.
- (b) CIPP strength values will be further reduced to account for creep based on the creep reduction values recommended in the pre-qualification submissions to assess the suitability of the liner to meet the 50 year design life requirement. The use of full enhancement factors in this analysis will be limited to liners that are confirmed by visual classification to be close-fit liners based on the post-lining sewer inspection.
- (c) The Contract Administrator will advise of any discrepancies between the constructed CIPP and the design requirements.
- (d) Perform necessary remedial measures to confirm that a CIPP deemed as structurally deficient will comply with the 50 year design life requirement such as confirmation of actual ovality, determination of a more representative groundwater elevation locally through monitoring, and supplemental strength testing and thickness measurements.
- (e) Repair sections of CIPP removed for supplemental testing by placing a full circumference internal point repair of the same thickness as the full segment liner over and extending 300 millimetres beyond each side of the cut section.
- (f) Install a supplemental CIPP of the required thickness to structurally enhance the installed CIPP if supplemental testing fails to confirm the CIPP will meet the 50 year design life requirement.
- (g) Review remedial action with the Contract Administrator prior to implementation.

- (h) Perform further testing, monitoring and calculations and install structural enhancements at own cost.

E2.5 MEASUREMENT AND PAYMENT

E2.5.1 Mobilization and Demobilization

- (a) Mobilization and demobilization will be measured on a unit basis and paid for at the Contract Unit Price for "Mobilization and Demobilization". Number of units to be paid for will be the total number of units of equipment set-up and removed, personnel, office and storage facilities to the job site and site clean up supplied and delivered in accordance with this specification, accepted and measured by the Contract Administrator.
- (b) 50% of the Mobilization and Demobilization unit price will be paid on the first progress payment.
- (c) The remaining 50% of the Mobilization and Demobilization unit price will be paid subsequent to the completion of the CIPP installation and site clean up.

E2.5.2 Verification of Existing Sewer Dimensions

- (a) Verification of existing sewer dimensions including the pre-design inspection will not be measured for separate payment and will be included with CIPP installation.

E2.5.3 Submittals Before Starting Work

- (a) Submittals required before starting work including CIPP design, resin samples, operations protocol and construction protocol will not be measured for separate payment and will be included with CIPP installation.

E2.5.4 Sewer Cleaning

- (a) Sewer cleaning will be measured and paid for in accordance with CW 2140.

E2.5.5 Sewer Inspections

- (a) Sewer inspections will be measured and paid for in accordance with CW 2145.

E2.5.6 Sewer Service Reports

E2.5.7 Sewer service reports will not be measured for separate payment and will be included with CIPP installation.

E2.5.8 Flow Control

- (a) Flow control measures necessary for mainline and all sewer services will be measured on a unit basis and paid for at the Contract Unit Price for "Flow Control". Number of units to be paid for will be the total number of units supplied in accordance with this specification, accepted and measured by the Contract Administrator.
- (b) Only one unit of flow control will be paid for each sewer segment and will include all mainline and sewer service flow control requirements.
- (c) Where no flow control measures are undertaken, no payment will be made for this item of Work.

E2.5.9 Sewer Preparation and Repairs Prior to Lining

- (a) Internal sewer pipe repairs will be measured and paid for in accordance with E3.
- (b) Removal of intruding sewer services and solid debris cutting will be measured and paid for in accordance with CW 2140.
- (c) Sewer service grouting will be measured on a unit basis and paid for at the Contract Unit Price for "Sewer Service Grouting – Prior to Lining". Number of units to be paid

for will be the total number of units grouted in accordance with this specification, accepted and measured by the Contract Administrator.

E2.5.10 CIPP Installation

- (a) Liner installation will be measured on a length basis for each size and paid for at the Contract Unit Price for "Full Segment CIPP", "Partial Full Segment CIPP" or "Internal Point Repair CIPP". Length to be paid for will be the total length of CIPP supplied and installed in accordance with this specification, accepted and measured by the Contract Administrator.
- (b) Full segment CIPP measurement will be made horizontally at grade, above the centreline of the pipe from centre to centre of manholes.
- (c) Partial full segment CIPP measurement will be made from the centre of one manhole to the termination point of the CIPP as measured by the post lining video inspection. Partial full segment CIPP installed beyond the limits identified by the Contract Administrator during review of the pre-lining video shall not be measured for payment.
- (d) Internal Point Repairs CIPP measurement will be made along the length of the internal point repair CIPP as measured by the post lining video inspection. Internal point repair CIPP installed beyond the limits identified by the Contract Administrator during review of the pre-lining video shall not be measured for payment.
- (e) Eighty (80) percent of the payment will be made upon satisfactory completion of the CIPP installation work. The remaining twenty (20) percent of the payment will be made upon confirmation of the CIPP strength and delivery and acceptance of all required submissions, shop drawings, and reports.

E2.5.11 Reinstatement of Sewer Services

- (a) Reinstatement of sewer services will be measured on a unit basis and paid for at the Contract Unit Price for "Reinstatement of Sewer Services". Number of units to be paid for will be the total number of units reinstated in accordance with this specification, accepted and measured by the Contract Administrator.
- (b) Where additional payment is to be made for sewer service grouting it shall be measured and paid as "Sewer Service Grouting – After Lining".

E2.5.12 Sewer Service Grouting After Lining

- (a) Sewer service grouting will be measured on a unit basis and paid for at the Contract Unit Price for "Sewer Service Grouting – After Lining". Number of units to be paid for will be the total number of units reinstated in accordance with this specification, accepted and measured by the Contract Administrator.

E2.5.13 Sewer Inspection Reports

- (a) Sewer inspection reports measured and paid for in accordance with CW 2145.

E2.5.14 Quality Control Records

- (a) Quality control records will not be measured for separate payment and will be included with payment for CIPP installation.

E2.5.15 Test Samples

- (a) CIPP test samples will not be measured for separate payment and will be included with payment for CIPP installation.

E2.5.16 Manhole Repairs

- (a) Manhole frames, covers, rungs and risers removed and replaced to facilitate the CIPP installation will not be measured for separate payment and will be included with payment for CIPP installation.

E3. SEWER STABILIZATION

E3.1 Description

E3.1.1 Sewer stabilization shall mean the internal repair of sewers and manholes by man entry techniques. Repairs are varied and may consist of holes in sewers with voids, missing bricks in sewers, obstructions and manhole base or riser repairs. Sewer stabilization repairs shall be carried out at the locations noted in Table E2.3.6 prior to performing sewer lining.

E3.1.2 The scope of work involved in sewer stabilization is as follows:

- (a) Secure the site and provide temporary traffic control;
- (b) Obtain all necessary underground clearances
- (c) Conduct a hazard assessment, including identification and evaluation;
- (d) Develop a safe work plan;
- (e) Implement the necessary procedures and controls to control hazards and maintain a safe working environment
- (f) Enter the manhole/sewer and perform the required repairs.
- (g) Clean-up the site.

E3.2 Materials

E3.2.1 Concrete

- (a) Concrete for large internal repairs to concrete and brick sewers and manholes and internal void filling shall be in conformance with Table CW 2160.1, Type B.
- (b) Patching and grouting of repairs to concrete and brick sewers and manholes shall be with a fast hardening high strength concrete repairing compound designed for underwater use, Duro-Crete by C C Chemicals or approved equal.
- (c) Flowable cement-stabilized fill for external void filling from the ground surface shall be in conformance with Table CW 2160.1, Type D.

E3.3 Construction Methods

E3.3.1 Hazard Assessment

- (a) In conjunction with securing the site and obtaining underground clearances, the Contractor shall conduct a hazard assessment for each site requiring a stabilization repair. The assessment shall identify and evaluate the hazards, including but not be limited to review of the following as it pertains to the Work to be performed:
 - (i) nature of the defect;
 - (ii) location of the defect in the sewer/manhole;
 - (iii) structural condition and amount of debris in the remaining sewer/manhole;
 - (iv) condition of the manholes up and downstream of the required repair;
 - (v) atmospheric conditions in the manholes up and downstream of the required repair;
 - (vi) condition of adjacent downstream sewers;
 - (vii) flow in the sewer.
- (b) The hazard assessment shall be based on the Contractors review of video for the sewer(s) and site inspection of the manholes, sewers and external conditions. Prior to the inspection, the Contractor shall conduct the necessary atmospheric monitoring of the affected manholes and sewers to establish acceptable entry conditions.

- (c) Based on the results of the hazard assessment the Contractor shall determine if they can perform the stabilization repairs in a safe manner. If the Contractor decides to proceed with the internal repairs they shall prepare a Safe Work Plan in accordance with E3.3.2 complete with the necessary controls and procedures required to maintain a safe working environment for the repair. Otherwise they shall notify the Contract Administrator and jointly the Contractor and the Contract Administrator shall review the nature of the defect and determine if an external point repair shall be performed in accordance with CW2130.

E3.3.2 Safe Work Plan

- (a) Subsequent to performing a hazard assessment the Contractor shall develop a safe work plan to address the potential hazards associated with each site. In addition to addressing the potential hazards the safe work plan shall address but not be limited to the following:
 - (i) guidelines for confined space entry work established by The Manitoba Workplace Safety and Health Act;
 - (ii) provision for emergency response;
 - (iii) training and duties for entry personnel;
 - (iv) rescue and emergency services;
 - (v) requirement for purging, ingesting, flushing and/or continuous ventilation to eliminate or control atmospheric hazards;
 - (vi) requirement for and provision of supplied air;
 - (vii) communication between members of the repair crew in the pipe and on the ground's surface;
 - (viii) current and forecasted weather conditions;
 - (ix) isolating the workspace by plugging of upstream sewers and monitoring of upstream flow levels;
 - (x) provision of back-up equipment;
 - (xi) method of ingress into the sewer;
 - (xii) method of egress out of the sewer – forward and backwards.
- (b) The Contractor shall not enter the sewer or manholes to begin the Work until they have completed a hazard assessment and safe work plan for the specific repair and reviewed the plans with their designated safety officer for acceptance. The safe work plan procedures and practices shall conform to all federal, provincial and municipal codes, regulations and guidelines including Manitoba Labour "Guidelines for Confined Space Entry".

E3.3.3 Equipment Set Up

- (a) In accordance with the safe work plan for the repair, the Contractor shall set up the required safety equipment and controls to safely perform the Work.
- (b) Specialized equipment to perform the repair work, such as lights, pressure washers, drills and chipping hammers shall in no way adversely affect the operation of the safety equipment required to perform the Work.
- (c) Subsequent to completion of the repairs the Contractor shall remove all equipment from the sewers and manholes.

E3.3.4 Enter the Manhole and Sewer

- (a) The Contractor shall enter the manhole/sewer and complete the Work in accordance with their safe work plan and requirements for the repair contained herein.
- (b) If at any time during the repair the attendant and/or Contractor believes he cannot safely perform the Work they shall immediately stop the Work and evacuate the sewer

and manholes. The Contractor shall re-assess their safe work plan considering the reason for the work stoppage. The Work shall only be resumed when the Contractor has deemed it safe to return by completing a re-assessment and safe work plan revision, where necessary.

- (c) If the Contractor deems the Work cannot be safely completed by internal stabilization they shall notify the Contract Administrator and jointly the Contractor and the Contract Administrator shall review the nature of the defect and determine if an external point repair shall be performed in accordance with CW 2130.

E3.3.5 Internal Sewer Repairs

- (a) The Contractor shall repair the sewer fabric to restore the structural integrity of the sewer and provide a smooth flow surface conforming to the adjacent sewer/manhole cross-section and materials.
- (b) Large concrete repairs shall include a reasonable and limited level of surface preparation, including removal of unsound material and cleaning of the edges of the repair area, and setting of the required formwork and bracing. Concrete placement and finishing shall be done in accordance with CW 2160. All formwork and bracing shall be removed from the sewer/manhole at the completion of the Work.
- (c) Concrete patching shall include a reasonable and limited level of surface preparation, including removal of unsound material and cleaning of the edges of the repair area. The Contractor shall apply the patching material in accordance with the manufacturer's printed instructions.
- (d) Small voids in the backfill shall be filled with concrete or other approved material from the inside of the sewer prior to repairing the sewer fabric or by pressure grouting after completion of the repairs. The void shall be completely filled to prevent settlement of the backfill and provide a solid backing for the liner.
- (e) Pressure grouting shall be done in accordance with the manufacturer's printed instructions.
- (f) Large voids shall be filled from the ground surface after completion of the repairs. Holes shall be cored in the pavement or the pavement shall be saw cut and removed to permit vacuum excavation from the underside of the pavement to the void. The void shall then be completely filled with flowable cement-stabilized fill. Pavement removal and restoration shall be in accordance with CW 2130.

E3.3.6 Internal Manhole Repairs

- (a) Complete manhole repairs identified in the Specifications or on the Drawings in accordance with CW 2130.

E3.4 Quality Control

E3.4.1 Repair Acceptance

- (a) Upon completion of the designated repair the Contractor shall clean and perform the pre-lining inspection.
- (b) The Contractor shall not be responsible for defects in existing un-repaired sewer lines unless those defects are a direct result of the Contractor's operation.

E3.4.2 Correction of Deficiencies

- (a) The Contractor shall correct deficiencies found in the sewer repair at their own cost including the cost of re-cleaning and re-inspection to confirm that the deficiencies are rectified in accordance with these specifications.

E3.5 Measurement and Payment

E3.5.1 Internal Sewer Repairs

- (a) Large concrete repairs requiring formwork will be measured on a unit basis and paid for at the Contract Unit Price for "Large Concrete Repairs". Number of units to be paid for will be the total number of large concrete repairs made in accordance with this specification, accepted and measured by the Contract Administrator.
- (b) Concrete patching of sewer walls and invert up to 1.0 metre in length will be measured and paid for on a unit basis and paid for at the Contract Unit Price for "Concrete Patching – Up to 1.0 metre long". Number of units to be to be paid for will be total number of concrete patch repairs up to 1.0 metre long completed in accordance with this specification, accepted and measured by the Contract Administrator.
- (c) Concrete patching of sewer walls and inverts in excess of 1.0 metre in length will be measured and paid for on a length basis for "Concrete Patching – In Excess of 1.0 metre long". Length to be paid for will be total linear metres of concrete patch repairs completed in accordance with this specification, accepted and measured by the Contract Administrator.
- (d) Filling small voids internally will be measured and paid for on a unit basis and paid for at the Contract Unit Price for "Filling Small Voids Internally". Number of units to be to be paid for will be total number of small voids filled internally in accordance with this specification, accepted and measured by the Contract Administrator.
- (e) Filling large voids externally with flowable cement-stabilized fill will be measured and paid for on a volume basis and paid for at the Contract Unit Price for "Filling Large Voids Externally – With Cement-Stabilized Fill". Volume to be to be paid for will be total number of cubic metres of void filled externally in accordance with this specification, accepted and measured by the Contract Administrator.

E3.5.2 Internal Manhole Repairs

- (a) Internal manhole repairs will be measured and paid for in accordance with CW 2130.

E4. CATCH BASIN INSPECTIONS

E4.1 Description

- E4.1.1 This specification shall cover the cleaning and inspection of all catch basin leads connected to sewers included in this contract to be lined with a CIPP for the purposes of determining whether the catch basin lead requires repair work.

E4.2 Construction Methods

E4.2.1 Cleaning

- (a) Clean catch basin leads in accordance with CW 2140.

E4.2.2 Video Inspections and Inspection Reports

- (a) Perform video inspection from catch basin to mainline sewer and provide inspection reports in accordance with CW2145.

E4.2.3 Repair Work

- (a) The Contract Administrator will review the video inspections of the catch basin leads to determine if repair work is required.
- (b) Catch basin leads determined to require full segment, partial full segment or internal point repair CIPP lining shall be done in accordance with E2 of this specification.

- (c) Catch basin leads determined to require replacement or external point repairs will be done by others outside of this Contract.

E4.3 Measurement and Payment

E4.3.1 Cleaning

- (a) Cleaning of catch basin leads shall be measured and paid for in accordance with CW 2140.

E4.3.2 Video Inspection

- (a) Video inspection of catch basin leads shall be measured and paid for in accordance with CW 2145.

E4.3.3 CIPP Lining

- (a) Full segment, partial full segment and internal point repair CIPP lining of catch basin leads shall be measured and paid for in accordance with E2.