

Appendix B

Geotechnical Data Report (GDR)

City of Winnipeg

**Jefferson East Combined
Sewer Relief Works (Contract 5)
Semple Avenue Trunk Sewer**
Geotechnical Data Report

Prepared by:

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Date: November 29, 2019

Project #: 60599385

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Mr. Jurgen Friesen, C.E.T.
Project Coordinator
The City of Winnipeg
Water and Waste Department
110 - 1199 Pacific Avenue
Winnipeg, MB R3E 3S8

November 29, 2019

Project #
60599385

Dear Mr. Friesen:

Subject: Jefferson East Combined Sewer Relief Works – Contract 5 – Semple Avenue Trunk Sewer - Geotechnical Data Report

AECOM Canada Ltd. (AECOM) is pleased to submit this Geotechnical Data Report for the Jefferson East Combined Sewer Relief Works (Contract 5) to be constructed in Winnipeg, Manitoba. The report provides a summary of the subsurface soil, and groundwater encountered along the alignment of the Semple Avenue Trunk Sewer and the laboratory test results for the soil.

If you have any questions concerning this report, please contact the undersigned at (204) 928-7444.

Sincerely,
AECOM Canada Ltd.



Ryan Harras, B.Sc., EIT
Geotechnical Engineer-in-Training

GR:rz
Encl.

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General Statement – Normal Variability of Subsurface Conditions

The scope of the investigation presented herein is limited to an investigation of the subsurface conditions as to the suitability of the proposed project. This report has been prepared to aid in the evaluation of the site and to assist the engineer in the design of the facilities. The description of the project represents an understanding of the significant aspects of the project relative to the design and construction of earth work, foundations, and similar. In the event of any changes in the basic design or location of the structures as outlined in this report or plan, AECOM Canada Ltd. should be given the opportunity to review the changes and to modify or reaffirm, in writing, the conclusions and recommendations of this report.


The analyses and recommendations represented in this report are based on the data obtained from the test holes drilled at the locations indicated on the site plans and from other information discussed herein. This report is based on the assumption that the subsurface conditions everywhere on the site are not significantly different from those encountered at the test hole locations. However, variation in the soil conditions between the test holes may exist. Also, general groundwater levels and conditions may fluctuate from time to time. The nature and extent of the variations may not become evident until construction. If subsurface conditions different from those encountered in the exploratory borings are observed or encountered during construction, or appear to be present beneath or beyond excavations, AECOM Canada Ltd. should be advised at once so that the conditions can be observed and reviewed and, where necessary, the recommendations reconsidered.

Since it is possible for conditions to vary from those identified at the test hole locations and from those assumed in the analysis and preparation of recommendations, a contingency fund should be included in the construction budget to allow for the possibility of variations which may result in modification of the design and construction procedures.

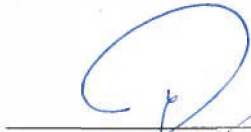
In order to observe compliance with the design concepts, specifications, or recommendations and to allow design changes in the event that subsurface conditions differ from those anticipated, it is recommended that all construction operations dealing with earthwork and the foundations be observed by an experienced geotechnical engineer. In addition, it is recommended that a qualified geotechnical engineer review the plans and specifications that have been prepared to check for substantial conformance with the conclusions and recommendations contained in the report

Quality Information

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Report Reviewed By:


Nov 29, 2019
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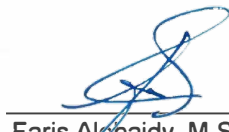

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1. Introduction

1.1 General

AECOM Canada Ltd. (AECOM) was retained by the City of Winnipeg Water and Waste Department (the City) to provide geotechnical engineering services to support the design and construction of the proposed Semple Avenue Trunk Sewer. AECOM understands that installation of the proposed Semple Avenue Trunk Sewer will be completed using one-pass or two-pass tunneling methods and pipe jacking.

This Geotechnical Data Report (GDR) presents the results of a detailed geotechnical investigation conducted by AECOM along the proposed sewer alignment. The detailed geotechnical investigation was conducted in general accordance with the American Society of Civil Engineers (ASCE) guidelines (*Essex 2007 and ASCE/CI 36-15*).

This report also provides a detailed summary of previous geotechnical investigation programs undertaken at the site and locations close in proximity to the site. The results and factual outcomes of these studies are included within Section 3 of this report.

This GDR should be read in conjunction with the Geotechnical Baseline Report (GBR). The GDR is subject to AECOM's Statement of Qualification and Limitations and General Statement regarding the Normal Variability of the Subsurface Conditions.

1.2 Aims and Objectives

The main objective of the AECOM 2019 geotechnical investigation was to determine the subsurface soil/groundwater conditions and engineering properties of the soil encountered at the test hole locations drilled along the proposed trunk sewer alignment. The primary focus of this report is to present and document the factual findings from this investigation and other relevant geotechnical investigations and laboratory testing programs. The results of AECOM's laboratory testing program and test hole logs are included within **Appendix B**, **Appendix C**, and **Appendix D** of this report.

The analyses and results presented in this report are based on the data obtained from the test holes drilled at discrete locations along the trunk sewer alignment. This report does not reflect any variations which may occur between the test hole locations. In the performance of subsurface explorations, specific information is obtained at specific locations at specific times. However, it is well known that variations in soil, bedrock, and groundwater conditions exist at most sites between test hole locations. The nature and extent of the variations may not become evident until the course of construction. If variations are then evident, it will be necessary to re-evaluate the findings and results presented in this report after performing on-site observations during the construction period and noting the characteristics of any variations.

This report is subject to the general statement regarding the normal variability of subsurface conditions provided above.

1.3 Project Details

The proposed trunk sewer will be constructed within the Mynarski ward in the northern region of Winnipeg. The proposed trunk sewer alignment extends from the west end of Semple Avenue at McKenzie Street to the east end of Semple Avenue at Scotia Street.

It is understood that the Semple Avenue trunk sewer project is an extension of the Jefferson East Combined Sewer Relief (CSR) Works. The Jefferson East Combined Sewer District was identified as needing upgrade to satisfy five-year level of service (LOS) design criteria. The proposed Semple Avenue Trunk Sewer upgrade involves disconnecting surface runoff from the existing combined sewer system in the northern portion of the Jefferson district, effectively freeing up capacity in the existing Jefferson Combined Sewer trunk and satisfying the five-year LOS design criteria for the remainder of the district. The outfall for this trunk was constructed in 2017 with the trunk temporarily terminating on Scotia Street at the east end of the proposed Semple Avenue Trunk Sewer. This outfall was installed using an open face excavator shield and pipe jacking. To minimize impact to the existing road and adjacent infrastructure at the project site, a trenchless solution is understood to be the preferred method over open-cut installation for the Semple Avenue Trunk Sewer.

Construction of the Semple Avenue Trunk Sewer will be between McKenzie Street on the west, and Scotia Street on the east as shown on **Figure 1** in **Appendix A**. A summary of the Semple Avenue Trunk Sewer lengths, sizes and installation methods are provided in **Table 1-1**.

Table 1-1: Summary of Semple Ave. Trunk Sewer Length, Size, and Proposed Installation Methods

Location	Length (m)	Size (Nominal Internal Diameter) (mm)	Installation Method
Start: McKenzie Street End: Andrews Street	400	1800 - Carrier Pipe	Tunneling with Pipe Jacking
Start: Andrews Street End: Scotia Street (East end of Semple Ave.)	1100	2100 - Carrier Pipe	Tunneling with Pipe Jacking

The proposed Semple Avenue Trunk Sewer alignment will include, at minimum, a launching shaft at the intersection of Semple Avenue and McKenzie Street and a retrieval shaft at the intersection of Semple Avenue and Scotia Street. Based on the selected tunneling method and equipment, the contractor may consider additional shafts at the following intersections: McGregor Street, Andrews Street, Powers Street, Salter Street, Aikins Street, and adjacent to Main Street (outside of the Main Street right of way). Upsizing of the 1800 mm pipe will be permitted.

New manholes will be constructed in shafts. A shaft will be located at the east end of the alignment near the connection to the existing 2100 concrete land drainage sewer (LDS) at Scotia Street. The final location, number, and size of launching and retrieving shafts are dependent on the selected trenchless construction method, as maximum drive lengths vary between each method. Based on current geotechnical information and groundwater depths, dewatering should not be required.

The overburden depth above the pipe crown varies from 3.4 m to 6.3 m along the Semple Avenue Trunk Sewer alignment. Typically, a minimum soil cover of approximately two (2) times the tunnel diameter is

required above the pipe crown. The surficial geology of the site and Semple Avenue Trunk Sewer alignment is shown on **Figure 2** in **Appendix A**.

1.4 Scope of Work

The scope of work for the detailed geotechnical investigation along the Semple Avenue trunk sewer alignment is summarized below:

- Review of geological survey maps and relevant background information.
- Obtain and review geotechnical reports available to AECOM with respect to the subject site. AECOM will also review geotechnical reports available in AECOM's library to collect information on the soil and bedrock within and near to the subject site.
- Prepare a GDR that documents the findings from AECOM's 2019 investigation and from previous geotechnical investigations and laboratory testing.
- Prepare a GBR in accordance with ASCE Guidelines for Preparation of GBR's.

2. Background Information

2.1 General Review of Existing Information

A review of available geotechnical information pertinent to the project was conducted including the geotechnical memo prepared by *AECOM Canada Ltd 2012* (AECOM 2012), a supplementary geotechnical letter prepared by *AECOM Canada Ltd 2015* (AECOM 2015), and an article about historical waterways in the vicinity of the Red River within Winnipeg. The main objective of the review was to obtain and present information specific to the subsurface and groundwater conditions with respect to the Semple Ave. Trunk Sewer alignment and areas adjacent to the site. The available memorandums and reports were also reviewed to prepare a GDR that presents factual information collected from the site investigation and laboratory testing. The following geotechnical documents were obtained and reviewed by the project team:

- AECOM Canada Ltd. (February 2012). Jefferson East Combined Sewer Relief - Sub-Surface Investigation - Geotechnical Memo.
- AECOM Canada Ltd. (October 2015). Jefferson East Combined Sewer Relief – Semple Outfall Supplementary Geotechnical Investigation - Geotechnical Letter.

The location of pertinent exploratory holes from past and existing geotechnical investigations relevant to the site are shown on **Figure 3 in Appendix A**.

In summary, a review of the background reports indicated the following:

- The soils south of Jefferson Avenue near Scotia Street consist of interlayered sand, silt, and clay underlain by deep silt deposits. (Ref. TH11-01 to TH11-03)
- Soils in other areas consist of interlayered clay fill, silt, and clay underlain by glacio-lacustrine clay soils, glacial till and carbonate bedrock (in descending order).

2.2 Historic Waterways

As part of the review of existing information AECOM reviewed an article about sixteen major streams and twenty small creeks that were present in the Winnipeg area during the time of the first European settlers (*Bernhardt 2018*). Since that time, the waterways are thought to have been drained and either filled, entombed, or re-routed to permit construction of varying infrastructure overtop of them.

Review of this article and the associated maps indicate the presence of a historic waterway named Inkster's Creek that appears to cross the proposed Semple Avenue trunk sewer alignment in a localized area between Main Street and Scotia Street. The presence of Inkster's Creek at the site was validated by an approximately 2.5 m surveyed elevation change across TH19-14 to TH19-17, as well as from topographic contours that follow the shape of a waterway. The maps from this article also suggest that the Inkster's Creek waterway network crossed through the Jefferson East Combined Sewer Relief (CSR) at various other locations, including near previously investigated test hole locations. The presence of the waterway at these previous locations was confirmed through review of topographic information obtained as well as review of observable topographic features from Google Maps Street View.

The presence of Inkster's Creek crossing at the proposed Semple Avenue trunk sewer alignment has implications related to the thickness and nature of overburden soils above the proposed pipe. Near-surface alluvial soil deposits are typical of waterways, and therefore need to be considered in the selection of appropriate tunneling methods. Additionally, the change in ground surface elevation in this localized area and the reduced overburden thickness that results will need to be assessed as part of the design and construction.

2.3 Previous Geotechnical Investigations

AECOM has reviewed the previous geotechnical investigations relevant to the Semple Avenue trunk sewer alignment and adjacent structures near the proposed trunk sewer alignment. The primary objective of the review was to collect information on the subsurface soil/bedrock conditions in the project area.

Table 2-1 summarizes the geotechnical investigations that have been completed at and in near proximity to the site.

Table 2-1: Summary of Previous Geotechnical Investigations Along Proposed Alignment

Organization	Type and Number of Investigation	Drilling Date	Associated Structure	Distance (m) and Relevancy to Semple Ave. Trunk Sewer Alignment	Comments
AECOM	SSA (2 no.)	December 13 to 14, 2011	Jefferson East CSR	Distance: 10 to 20 South of proposed alignment	TH11-11, 12

Notes: SSA- Solid Stem Auger

Geotechnical investigations which have previously been undertaken within the area adjacent to the proposed Semple Avenue trunk sewer alignment are also summarised in **Table 2-2** below.

Table 2-2: Summary of Previous Geotechnical Investigations Offset from Proposed Alignment

Organization	Type and Number of Investigation	Drilling Date	Associated Structure	Distance (m) and Relevancy to NEIS Alignment	Comments
AECOM	SSA (12 no.)	December 12 to 14, 2011	Jefferson East CSR	Distance: 60 to 900 Within Jefferson East CSR General Area	TH11-01, 02, 03, 04, 05, 06, 07, 08, 09, 10, 14, SP11-13
AECOM	SSA/RC (1 no.) SSA (1 no.)	February 24, 2015	Jefferson East CSR - Semple Outfall	Distance: 50 to 80 Southeast of proposed alignment	SI15-01, VW15-02

Notes: SSA- Solid Stem Auger; RC - Rock Core.

The locations of the exploratory holes outlined in **Table 2-1** and **Table 2-2** are shown on **Figure 3** in **Appendix A**. Test hole logs related to previous geotechnical investigations are included as **Appendix B** of this report. The laboratory testing results from the previous geotechnical investigations are provided in **Appendix D** of this report.

2.3.1 AECOM (February 2012) - Jefferson East Combined Sewer Relief - Sub-Surface Investigation - Geotechnical Memo

In support of the City's "Basement Flooding Relief Program", AECOM was engaged to provide geotechnical engineering services for the Jefferson East Combined Sewer District to facilitate the detailed design and contract administration for proposed buried pipes and outfall. As part of the scope of work, AECOM completed a geotechnical drilling investigation and laboratory testing program within the Jefferson East District to characterize sub-surface soil and groundwater conditions, and to provide general recommendations related to pipe installation.

The AECOM 2012 geotechnical investigation consisted of fourteen (14) test holes (TH11-01 to TH11-12, SP11-13, and TH11-14) spread out across the Jefferson East District and drilled to depths ranging from 12.2 m to 19.5 m below ground surface. As part of this investigation, one (1) piezometer was installed in test hole SP11-13 (see Section 3.3 of this report for details). The geotechnical testing program consisted of index classification testing and strength testing of soils. The results of the geotechnical laboratory tests are included within the AECOM 2012 memo. Further information concerning the encountered subsurface soil and groundwater conditions are provided in Section 3 of this report. A summary of the test hole drilling is provided in **Table 2-3**, below. The test hole records for the 2012 investigation are provided in **Appendix B**. The geotechnical material testing results are also provided within **Appendix D** of this report.

**Table 2-3: Summary of Jefferson East CSR – Sub-Surface Investigation
(AECOM 2012)**

Test Hole	Location	Coordinates (UTM 14)	Ground Elevation (m)	Completion Depth (m)	Completion Soil Unit
TH11-01	Scotia St. (between Seven Oaks Blvd. and Jefferson Ave.)	5532753 m N 635380 m E	229.15	12.19	Silt
TH11-02	Scotia St. at Tait Ave.	5532592 m N 635413 m E	229.06	12.19	Silt
TH11-03	Rupertsland Blvd. (West of Scotia St.)	5532453 m N 635415 m E	227.91	12.19	Clay
TH11-04	Mac St. (between Rupertsland Blvd. and Tait Ave.)	5532610 m N 635216 m E	228.68	12.19	Clay
TH11-05	Jones St. at Colleen Rd.	5532614 m N 634999 m E	229.10	12.19	Clay
TH11-06	Seven Oaks Blvd. (between Jones St. and Scotia St.)	5532780 m N 635215 m E	230.77	12.19	Clay
TH11-07	Seven Oaks Blvd. (East of Main St.)	5532903 m N 634952 m E	229.91	12.19	Clay
TH11-08	Jones St. at St. Anthony Avenue	5533006 m N 635170 m E	229.85	12.19	Clay
TH11-09	Jones St. at Hartford Ave.	5533160 m N 635238 m E	228.71	12.19	Clay

Test Hole	Location	Coordinates (UTM 14)	Ground Elevation (m)	Completion Depth (m)	Completion Soil Unit
TH11-10	Scotia St. at Belmont Ave.	5533205 m N 635370 m E	230.67	12.19	Clay
TH11-11	Semple Ave. (East of Main St.)	5533432 m N 635210 m E	230.74	12.19	Clay
TH11-12	Scotia St. at Semple Ave.	5533322 m N 635426 m E	230.89	12.19	Clay
SP11-13	Upper Outfall Area (East of Scotia St.)	5533354 m N 635497 m E	230.58	19.51	Silt (Till)
TH11-14	Lower Outfall Area (East of Scotia St.)	5533346 m N 635514 m E	226.96	15.54	Silt (Till)

The AECOM 2012 memo indicated that the subsurface ground profile within the investigated area generally consisted of (in descending order): topsoil, clay fill, upper complex zone (interlayered clays, silts, and sands), clay, and glacial silt till. No test holes were advanced into bedrock. Carbonate bedrock was encountered underlying the glacial till in all test holes. The AECOM 2012 test holes are presented in **Appendix B** of this report.

Groundwater information collected from the AECOM 2012 geotechnical investigation is summarized in Section 3.3 of this report.

2.3.2 AECOM (October 2015) – Jefferson East Combined Sewer Relief – Semple Outfall Supplementary Geotechnical Investigation - Geotechnical Letter

This letter was produced in support of the Jefferson East District storm relief program waterway application for construction of the proposed chamber and outfall pipe on the west riverbank of the Red River between 405 and 409 Scotia Street. As part of the waterway application, AECOM was engaged to provide riverbank characterization near the proposed infrastructure, complete a pre-construction slope stability analysis, and to provide long term slope monitoring prior to and post-construction. The letter summarizes the findings of the geotechnical investigation, laboratory testing, initial instrumentation monitoring, and provides the results of the completed slope stability analyses.

The AECOM 2015 geotechnical investigation consisted of two (2) test holes (SI15-01 and VW15-02) drilled on either side of the proposed outfall pipe on the west riverbank of the Red River. As part of this investigation, one (1) slope inclinometer was installed in test hole SI15-01 and two (2) vibrating wire piezometers were installed in test hole VW15-02 (see Section 3.3 of this report for details). The geotechnical testing program consisted of index classification testing and strength testing of soils. The results of the geotechnical laboratory tests are included within the AECOM 2015 memo. Further information concerning the encountered subsurface soil and groundwater conditions are provided in Section 3 of this report. A summary of the drilled test holes is provided in **Table 2-4**, below. The test hole records for the 2015 investigation are provided in **Appendix B**. The geotechnical material testing results are also provided within **Appendix D** of this report.

**Table 2-4: Summary of Jefferson East CSR – Semple Outfall Supplementary
Geotechnical Investigation
(AECOM 2015)**

Test Hole	Location	*Ground Elevation (m)	Completion Depth (m)	Completion Soil Unit
SI15-01	North of Outfall Pipe 4 m West of Lower Slope	227.00	22.61	Bedrock
VW15-02	South of Outfall Pipe 15 m West of Lower Slope	227.00	12.50	Clay

Notes: * Drilled locations not surveyed. Elevations were inferred.

The AECOM 2015 memo indicated that the subsurface ground profile within the investigated area generally consisted of (in descending order): topsoil, alluvial upper zone (silty clay, silt), lacustrine clay, and glacial silt till. Carbonate bedrock was encountered underlying the glacial till in test hole SI15-01. The AECOM 2015 test holes are presented in **Appendix B** of this report.

Groundwater information collected from the AECOM 2015 geotechnical investigation is summarized in Section 3.3.1 of this report.

A slope stability analysis was performed at the proposed outfall pipe alignment along the riverbank of the Red River. The stability models were developed using SEEP/W, SIGMA/W, and SLOPE/W modules of the GeoStudio 2007 software package. The intent of the stability analyses was to determine the existing stability of the riverbank prior to construction of the outfall pipe for normal summer and normal winter river water levels. The results of the analysis were provided to the City to assist with identifying whether slope stabilizing measures would need to be implemented to satisfy desired post-construction factors of safety.

The slope stability analysis incorporated topographic survey information and subsurface information obtained from the AECOM 2012 and 2015 investigation and material testing programs. The adopted soil strength parameters used within the slope stability analysis are summarised in **Table 2-5** below.

**Table 2-5: Soil Properties Used in Stability Modelling
(AECOM 2015)**

Soil Description	Unit Weight (kN/m ³)	Cohesion (kPa)	Friction Angle (°)	Hydraulic Conductivity (m/s)
Upper Zone (Alluvial)	17	0	25	1 x 10 ⁻⁹
Lacustrine Clay	17	5	17	1 x 10 ⁻⁹
Till	20	5	30	1 x 10 ⁻⁵

2.4 Regional Geology

2.4.1 Bedrock Geology

The shallow bedrock geology of the Winnipeg area generally comprises of carbonate rock of the Selkirk and Fort Garry Members belonging to the Red River Formation. The Red River Formation consists of

alternating layers of limestone and dolomite (with basal shale layers). The proposed Semple Avenue trunk sewer alignment is located near the geological contact between the Selkirk Member and the lower part of the Fort Garry Member of the Red River Formation (*Matile G.L.D 2004*).

The upper surface of the bedrock is generally characterised with poor rock mass characteristics and is highly fractured. Karstic features are also common within the upper zone of the carbonate bedrock. The Karst topography is typically infilled with mixtures of silt, sand and gravel till material. The Winnipeg formation underlies the Red River formation, and typically consists of sandstone and shale units. The basement bedrock geology is comprised of the Pre-Cambrian Basal Granites at depth. The actual bedrock encountered at the site are described in Section 3.0 of this report below.

2.4.2 Surficial Geology

The overlying surficial soils generally comprise of upper complex deposits, glacio-lacustrine clays and glacial till soils of varying thicknesses and compositions. The glacial till soils were laid down by the advancing and retreating glacial ice masses. The glacio-lacustrine soils are a product of fine materials deposited through suspension within the glacial lakes (*Manitoba Energy and Mines 1990*).

The glacio-lacustrine soils are typically 10 m to 12 m thick but vary spatially within the Red River Valley of southern central Manitoba from approximately 1 m to 20 m. The glacio-lacustrine soils are further sub-divided into two (2) distinct sub-units; the Upper and Lower Clay. The transition zone between the two (2) sub-units is typically located between an approximate depth of 4.6 and 7.6 m (*Graham and Shields 1985*).

Glacial till soils underlie the glacio-lacustrine soils, and the soil boundary interface is usually marked by a transition zone containing glacial till inclusions.

2.4.3 Hydrogeology

There are three (3) significant bedrock aquifers beneath the City of Winnipeg. The largest is known as the Upper Carbonate Aquifer which is generally found within the upper 7 m of the carbonate bedrock profile. This aquifer is contained in an extensive network of fractures and Karstic solution cavities formed by the dissolution of the Upper carbonate rocks. Other aquifers include the Lower and Middle Carbonate Aquifers near the base of the carbonate bedrock profile, and the underlying Winnipeg Formation sandstones. In general, these Lower and Middle aquifers are not utilized due either to the presence of saline water or the higher productivity of the Upper Carbonate Aquifer.

Groundwater flow within the Upper Carbonate Aquifer is towards the Red River (the major discharge point for this aquifer), and in particular towards the St. Boniface Industrial Park on the east side of the Red River where consumptive groundwater use occurs. West of the Red River, the water quality varies from brackish to saline, except beneath the northwest part of the city. Therefore, groundwater in this aquifer is mostly used for commercial and industrial heating and cooling. The majority of these systems recycle the water back into the subsurface and there is very little consumptive use.

Prior to the start of development of this aquifer in the late 1800's, the potentiometric surface was estimated to be approximately 3 to 6 m below ground surface in the central Winnipeg area. Extensive consumptive use of this groundwater resulted in a decline in the potentiometric surface to depths of 21 to 24 m. Consumptive use has declined since the early 1970's and since that time the potentiometric

surface has been rising. Currently in the downtown area the potentiometric surface is approximately 7 m below grade.

2.5 AECOM 2019 Geotechnical Investigation

The AECOM 2019 geotechnical investigation field program (including laboratory test results) is summarized below. The 2019 AECOM geotechnical investigation was completed to determine the subsurface conditions along the proposed Semple Avenue Trunk Sewer alignment.

2.5.1 Geotechnical Investigation

From June 20 to 21, 2019 a hydro-vac investigation was completed at seventeen (17) proposed test hole locations to a maximum depth of 3.1 m to confirm that the locations were clear of utilities. From June 24 to 27, 2019, 16 test holes (TH19-01 to TH19-08, and TH19-10 to TH19-17) were drilled at the approximate locations shown on **Figure 3 in Appendix A** and summarized in **Table 2-6** below. One (1) proposed test hole (TH19-09) could not be drilled due to the presence of underground and above ground utilities in the area. A safe work plan was prepared prior to the hydro-vac and drilling investigations, and utility clearance certificates were obtained by AECOM personnel from representatives of ClickBeforeYouDigMB and DigShaw.

**Table 2-6: Summary of Jefferson East CSR – Sub-Surface Investigation
(AECOM 2019)**

Test Hole	Location	Coordinates (UTM 14)	Ground Elevation (m)	Completion Depth (m)	Completion Soil Unit
TH19-01	Sta. 0+197.30	5533995 m N, 634036m E	231.11	231.11	Silt/Sand (Till)
TH19-02	Sta. 0+250.40	5533973 m N, 634084m E	231.28	231.28	Clay
TH19-03	Sta. 0+371.20	5533922 m N, 634193m E	231.52	231.52	Clay
TH19-04	Sta. 0+457.90	5533885 m N, 634272m E	231.54	231.54	Clay
TH19-05	Sta. 0+592.90	5533828 m N, 634394m E	231.32	231.32	Silt/Sand (Till)
TH19-06	Sta. 0+654.10	5533801 m N, 634449m E	231.23	231.23	Clay
TH19-07	Sta. 0+775.20	5533750 m N, 634559m E	231.13	231.13	Clay
TH19-08	Sta. 0+849.90	5533718 m N, 634627m E	230.97	230.97	Silt/Sand (Till)
*TH19-09	Sta. 0+197.30	5533656 m N, 634757 m E	-	-	-
TH19-10	Sta. 1+068.50	5533626 m N, 634825m E	230.73	230.73	Clay
TH19-11	Sta. 1+183.50	5533577 m N, 634929m E	230.89	230.89	Silt/Sand (Till)
TH19-12	Sta. 1+266.00	5533542 m N, 635003m E	230.76	230.76	Clay
TH19-13	Sta. 1+396.00	5533487 m N, 635121m E	230.81	230.81	Clay
TH19-14	Sta. 1+550.80	5533421 m N, 635261m E	230.65	230.65	Silt/Sand (Till)
TH19-15	Sta. 1+591.50	5533404 m N, 635298m E	230.08	230.08	Clay
TH19-16	Sta. 1+656.60	5533376 m N, 635357m E	228.55	228.55	Silt/Sand (Till)
TH19-17	Sta. 1+719.70	5533349 m N, 635414m E	230.54	230.54	Clay

Notes: * TH19-09 not drilled due to presence of underground and above ground utilities in the area

Drilling was completed by Maple Leaf Drilling using the following equipment: track-mounted Acker MP-5 drill rig equipped with 125 mm solid stem augers for test holes TH19-02 to TH19-08 and TH19-12 to TH19-17, and a truck-mounted Canterra CT-250 drill rig equipped with 125 mm solid stem augers for test holes TH19-01, TH19-10, and TH19-11. Subsurface conditions observed during drilling were visually classified and documented by AECOM geotechnical personnel. Other pertinent information such as groundwater and drilling conditions were also recorded during the field investigation.

Disturbed soil samples collected from auger cuttings and split-spoon samplers, as well as relatively undisturbed Shelby Tube samples were obtained at regular intervals. Standard penetration tests (SPTs) were completed at selected intervals in the test holes and blow counts for 300 mm penetration (SPT "N" blow counts) were recorded.

Recovered soil samples were transported to Dyregrov Robinson Inc. materials testing laboratory in Winnipeg for further visual examination and moisture content, undrained shear strength, pocket penetrometer, and bulk unit weight testing. A section of all recovered Shelby Tube samples were waxed to preserve them for further testing. Select samples were taken to H. Manalo Consulting materials testing laboratory in Winnipeg for Atterberg Limits, grain size distribution (hydrometer/sieve methods), and permeability testing. Other samples were taken to Wood Environment & Infrastructure Solutions materials testing laboratory in Winnipeg for Atterberg Limits, grain size distribution (hydrometer/sieve methods), and swell testing. All electrochemical testing was completed by ALS Environmental's Winnipeg laboratory.

Detailed test hole logs have been prepared for each test hole, and are attached as **Appendix C**. The test hole logs include description and depth of the soil units encountered, sample type, sample location, results of field and laboratory testing, and other pertinent information such as seepage and sloughing.

2.5.2 Laboratory Testing

The laboratory testing program included the determination of moisture contents, grain size distribution (hydrometer method), Atterberg Limits, undrained shear strength (unconfined compressive strength, pocket penetrometer, and torvane tests), bulk unit weight, permeability (hydraulic conductivity test), consolidation (oedometer method), swell (*ASTM D4546-14 one-dimensional swell or collapse test*), and electrochemical properties (pH, sulphate content, resistivity/conductivity). Laboratory test results are included in **Appendix D**, and the type and number of laboratory tests are summarized in **Table 2-7**.

**Table 2-7: Summary of Type and Number of Laboratory Tests
(AECOM 2019)**

Laboratory Test	Number of Tests	Data Location
Moisture Content	157	Test Hole Logs & Appendix D
Atterberg Limits	11	Test Hole Logs & Appendix D
Grain Size Distribution (Hydrometer Method)	11	Test Hole Logs & Appendix D
Undrained Shear Strength (Unconfined Compressive Strength Method)	26	Test Hole Logs & Appendix D
Pocket Penetrometer	29	Test Hole Logs & Appendix D
Torvane	29	Test Hole Logs & Appendix D
Bulk Unit Weight	27	Test Hole Logs & Appendix D
Permeability (Hydraulic Conductivity Method)	2	Appendix D
Free Swell & Swelling Pressure (One-Dimensional Swell or Collapse Method)	5	Appendix D
Electrochemical (pH, Sulphate, Resistivity/Conductivity)	5	Appendix D

The geotechnical testing program undertaken as part of the historic geotechnical investigation programs has been summarized in **Table 2-8**, below.

**Table 2-8: Summary of Type and Number of Laboratory Tests
(AECOM 2012, AECOM 2015)**

Laboratory Test	AECOM (2012) Number of Tests	AECOM (2015) Number of Tests
Moisture Content	131	20
Atterberg Limits	4	3
Grain Size Distribution (Hydrometer Method)	4	2
Undrained Shear Strength (Unconfined Compressive Strength Method)	3	Not Tested
Pocket Penetrometer	34	Not Tested
Torvane	29	13
Bulk Unit Weight	3	Not Tested

3. Subsurface Conditions

3.1 General

The following sections describe the subsurface conditions encountered during the AECOM 2019 geotechnical investigation and information referenced from review of geotechnical investigations previously carried out at the site. The results of the AECOM 2019 investigation are in general agreement with investigations carried out in the past for City owned projects in the site area. It is however prudent to note that subsurface conditions can vary significantly between test holes within the same site. It should also be noted that test holes drilled for the AECOM 2019 investigation were located within the north boulevard of Semple Avenue. As a result, information about the existing roadway pavement structure along the proposed alignment was not collected and has therefore not been discussed or presented within this document. A simplified stratigraphic profile based on the findings of the AECOM 2019 investigation and relevant historic soils data (derived from past geotechnical reports) along the Semple Avenue Trunk Sewer alignment is presented as **Figures 4A to 4E** in **Appendix A**.

Detailed descriptions of the subsurface conditions encountered at the test hole locations as part of the AECOM 2019 investigation are provided on the test hole logs presented in **Appendix C**. A description of the terms and symbols used on the test hole logs are also included in **Appendix C**. A brief description of the subsurface soil/bedrock unit encountered along the trunk sewer alignment and adjacent locations are provided in the following sections.

3.2 Subsurface Profile

Soils encountered during the investigations consisted of the following:

- Topsoil
- Fill
- Upper Complex
 - Clay
 - Silt
 - Sand
- Glacio-Lacustrine Clay
- Glacial Till
- Carbonate Bedrock

Each of these units is described below.

3.2.1 Topsoil

A layer of topsoil was encountered in all test holes drilled as part of the AECOM 2012, AECOM 2015, and AECOM 2019 geotechnical investigations ranging in thickness from 0.1 m to 0.3 m. The topsoil was classified as black, moist, and contained trace to some rootlets.

3.2.2 Fill

Fill was encountered beneath the topsoil in all test holes completed as part of the AECOM 2012 investigation except in test hole TH11-14, and all test holes completed as part of the AECOM 2019 investigation. The fill was classified as clay fill or silt fill and ranged in thickness from 0.3 m to 1.0 m (0.7 m average) when considering only test holes along the proposed trunk sewer alignment from the AECOM 2012 and AECOM 2019 investigations. The fill ranged in thickness from 0.3 m to 3.0 m when considering only test holes offset from the proposed trunk sewer alignment from the AECOM 2012 and AECOM 2015 investigations.

Clay fill was encountered in all AECOM 2012 and AECOM 2019 test holes except in test holes TH11-14, and TH19-03. The clay fill contained some silt to silty, trace to some sand, trace gravel, trace roots, and was brown to dark grey, firm, dry to moist, and of intermediate to high plasticity.

A summary of the laboratory testing results for the clay fill deposits encountered along the proposed alignment as part of the AECOM 2012 and AECOM 2019 investigations are presented in **Table 3-1** below.

**Table 3-1: Clay Fill - Summary of Laboratory Testing Along Proposed Alignment
(AECOM 2012, AECOM 2019)**

Laboratory Test	Clay Fill
Moisture Content (%)	26 to 32 (29)

Notes: (#) - Average Value

A summary of the laboratory testing results for the clay fill deposits encountered offset from the proposed alignment as part of the AECOM 2012 and AECOM 2015 investigations are presented in **Table 3-2** below.

**Table 3-2: Clay Fill - Summary of Laboratory Testing Offset from Proposed Alignment
(AECOM 2012, AECOM 2015)**

Laboratory Test	Clay Fill
Moisture Content (%)	21 to 30 (25)

Notes: (#) - Average Value

Silt fill was encountered in test holes TH19-03 and TH19-14 and was classified as sandy with trace to some clay, light brown, dry to moist, and of low plasticity. No laboratory testing was completed on the encountered silt fill.

3.2.3 Upper Complex

The upper complex is a near ground surface zone common to the Winnipeg area that typically consisting of interlayered clays, silts, sands, and organics near ground surface that are thought to be a mixture of lacustrine and alluvial sediments. Upper complex clays are generally distinguished by a lower range of moisture content when compared to glacio-lacustrine clays, which was evident from the plot of moisture content values on the AECOM 2012 and AECOM 2019 test hole logs along the proposed alignment. Upper complex deposits were encountered beneath the topsoil or fill in all AECOM 2012, AECOM 2015, and AECOM 2019 test holes ranging in total thickness from 0.5 m to 2.2 m for test holes along the proposed alignment and from 0.6 m to 11.6 m for test holes offset from the proposed alignment.

The extent of the upper complex deposit identified from test holes along the proposed trunk sewer alignment from the AECOM 2012 and AECOM 2019 investigations are outlined in **Table 3-3** below.

**Table 3-3: Upper Complex - Soil Profile Along Proposed Alignment
(AECOM 2012, AECOM 2019)**

Location	Profile	Clay	Silt	Sand
Section 1 (Station 0+202 to 0+600)	Elevation at Base (m)	228.3 to 230.0		
	Thickness (m)	0.5 to 1.2	NR to 0.9	NR to 1.0
	*Average Thickness (m)	0.7	0.6	1.0
Section 2 (Station 0+600 to 1+000)	Elevation at Base (m)	228.5 to 229.6		
	Thickness (m)	NR to 0.3	NR to 0.8	NR to 1.1
	*Average Thickness (m)	0.3	0.7	1.1
Section 3 (Station 1+000 to 1+500)	Elevation at Base (m)	228.0 to 229.4		
	Thickness (m)	NR to 0.9	0.8 to 1.2	NR
	*Average Thickness (m)	0.6	1.0	NR
Section 4 (Station 1+500 to 1+742)	Elevation at Base (m)	226.6 to 228.5		
	Thickness (m)	NR to 1.5	NR to 1.1	NR to 0.7
	*Average Thickness (m)	0.9	0.7	0.4

Notes: NR- Not Recorded; * Average thickness from test holes where encountered

The extent of the upper complex deposit identified from test holes offset from the trunk sewer alignment from the AECOM 2012 and AECOM 2015 investigations are outlined in **Table 3-4** below.

**Table 3-4: Upper Complex - Soil Profile Offset from Proposed Alignment
(AECOM 2012, AECOM 2015)**

Location	Profile	Clay	Silt	Sand
Jefferson East CSR (AECOM 2012)	Elevation at Base (m)	216.0 to 227.9		
	Thickness (m)	0.5 to 6.7	0.5 to 10.4	0.9 to 1.5
	*Average Thickness (m)	1.8	2.5	1.2
Outfall Structure (AECOM 2015)	Elevation at Base (m)	225.2 to 225.3		
	Thickness (m)	0.2 to 1.2	0.3 to 0.6	NR
	*Average Thickness (m)	0.6	0.4	NR

Notes: NR- Not Recorded; * Average thickness from test holes where encountered

A summary of the laboratory testing results for the upper complex clay, silt, and sand deposits encountered along the proposed alignment as part of the AECOM 2012 and AECOM 2019 investigations are presented in **Table 3-5** below.

**Table 3-5: Upper Complex - Summary of Laboratory Testing Along Proposed Alignment
(AECOM 2012, AECOM 2019)**

Laboratory Test	Clay	Silt	Sand
Moisture Content (%)	24 to 36 (28)	14 to 28 (22)	12 to 19 (16)
Atterberg - Plastic Limit (%)	23	NP to 16	NT
Atterberg - Liquid Limit (%)	53	NP to 24	NT
Atterberg - Plasticity Index (%)	30	NP to 8	NT
Grain Size - Gravel (%)	0	0	NT
Grain Size - Sand (%)	6	1 to 2 (2)	NT
Grain Size - Silt (%)	39	81 to 87 (84)	NT
Grain Size - Clay (%)	55	11 to 18 (15)	NT
pH	NT	9.1	NT
Resistivity (ohm*cm)	NT	4950	NT
Conductivity (mS/cm)	NT	0.2	NT
Sulphate Content (mg/kg)	NT	46	NT

Notes: NP- Non-Plastic; NT- Not Tested; (#) - Average Value

A summary of the laboratory testing results for the upper complex clay, silt, and sand deposits encountered offset from the proposed alignment as part of the AECOM 2012 and AECOM 2015 investigations are presented in **Table 3-6** below.

**Table 3-6: Upper Complex - Summary of Laboratory Testing Offset from Proposed Alignment
(AECOM 2012, AECOM 2015)**

Laboratory Test	Clay	Silt	Sand
Moisture Content (%)	18 to 38 (29)	8 to 35 (26)	29 to 35 (32)
Atterberg - Plastic Limit (%)	19 to 22 (21)	17	15
Atterberg - Liquid Limit (%)	65 to 71 (68)	34	23
Atterberg - Plasticity Index (%)	47 to 49 (48)	17	8
Grain Size - Gravel (%)	0	1	0
Grain Size - Sand (%)	6 to 7 (6)	5	62
Grain Size - Silt (%)	25 to 30 (27)	70	23
Grain Size - Clay (%)	64 to 69 (66)	25	15
Pocket Penetrometer - Undrained Shear Strength (kPa)	72 to 132 (101)	NT	NT
Torvane - Undrained Shear Strength (kPa)	59 to 79 (71)	NT	NT
Bulk Unit Weight (kN/m ³)	NT	NT	NT

Notes: NT- Not Tested; (#) - Average Value

Plots of moisture content and Atterberg Limits with elevation for the upper complex soil deposits encountered in the AECOM 2012, AECOM 2015, and AECOM 2019 investigations are illustrated in **Figure 3-1** below.

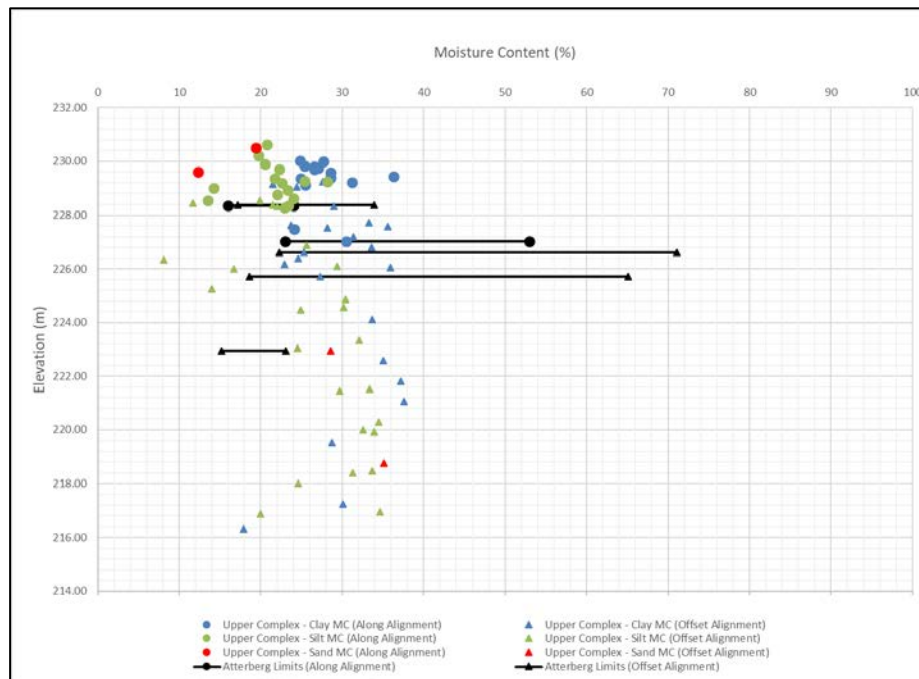


Figure 3-1: Moisture Content & Atterberg Limits with Elevation for Upper Complex (AECOM 2012, AECOM 2016, AECOM 2019)

3.2.3.1 Upper Complex – Clay

The upper complex clay contained trace silt to silty, trace to some sand, and trace to some gravel. The upper complex clay was brown to grey, soft to stiff, moist, and of intermediate to high plasticity. The upper complex clay was classified as clay and silt in test hole TH19-16. Boulder and cobble were not encountered within this layer during the investigations.

3.2.3.2 Upper Complex - Silt

The upper complex silt contained trace clay to clayey, trace sand to sandy, trace to some gravel, and was brown to grey, soft to firm, moist to wet, and ranged from non-plastic to intermediately plastic. Boulder and cobble were not encountered within this layer during the investigations.

3.2.3.3 Upper Complex - Sand

The upper complex sand was silty, contained trace to some clay, and was light brown to brown, and dry to moist. Boulder and cobble were not encountered within this layer during the investigations.

3.2.4 Glacio-Lacustrine Clay

A layer of glacio-lacustrine clay was encountered during the AECOM 2012, AECOM 2015, and AECOM 2019 investigations. These glacio-lacustrine soils are common to the Winnipeg area and have been the subject of prior investigation, research, and testing as part of the Floodway Channel project. The

subsequent sections provide a summary of the glacio-lacustrine clay properties from published literature and technical reports, as well as the results from the AECOM 2012, AECOM 2015, and AECOM 2019 investigations completed in proximity to the proposed trunk sewer alignment site.

3.2.4.1 Reported Geotechnical Properties

Published literature and technical reports were reviewed to obtain data with respect to the subsurface soils and bedrock within the Winnipeg area, specifically along the proposed trunk sewer alignment.

Geotechnical parameters of the Lake Agassiz glacio-lacustrine clay (Upper and Lower Clays) have been referenced from the Floodway Channel Pre-design Floodway Expansion Project (*KGS Group, Acres Engineering and UMA Engineering, 2004*) reports and are presented within **Table 3-7**. The Floodway Channel project is located approximately 10 to 20 km east and southeast of the proposed trunk sewer alignment and involved an extensive study of the glacio-lacustrine soils common to the Winnipeg area.

The glacio-lacustrine clay layer can be further broken down into the Upper Glacio-Lacustrine Clay (Upper Clay) and Lower Glacio-Lacustrine Clay (Lower Clay) layers. The Upper Clay is typically stiff in consistency, highly plastic, fissured, and contains gypsum pockets. The Lower Clay is typically soft to firm in consistency and has an intermediate to high plasticity. Fine to coarse grained gravel and boulders are occasionally found in the Lower Clay near the glacial till interface (*Graham and Shields, 1985*). The clay content was between 67 and 81 percent of the total composition of the Lake Agassiz glacio-lacustrine clay in Winnipeg. The clay size fractions typically consist of up to 75 percent montmorillonite, 10 percent illite, 10 percent kaolinite, and approximately 5 percent quartz mineral.

The typical soil index classification and undrained shear strength compressive strength parameters presented as part of the published literature and technical reports are summarized in **Table 3-7**.

Table 3-7: Glacio-Lacustrine Clay - Published Geotechnical Soil Parameters

Soil Property	Typical Range of Values
Moisture Content (%)	40 to 60- Upper and Lower Clay
Liquid Limit (%)	80 to 110- Upper Clay 65 to 95- Lower Clay
Plasticity Index (%)	60 to 80- Upper Clay 40 to 65- Lower Clay
Undrained Shear Strength (kPa)	70 to 100- Upper Clay 25 to 40- Lower Clay

Notes: Based on *Graham & Shields (1985)*

Effective shear strength parameters of the Upper and Lower Clay obtained from consolidated undrained compression triaxial strength testing on a large number of relatively undisturbed samples yielded intact peak strengths of:

- Upper Clay- $c' = 19.6$ kPa and $\phi' = 20.5^\circ$ and
- Lower Clay- $c' = 29.8$ kPa and $\phi' = 15.8^\circ$.

The effective large strain shear strength (fully softened) parameters for the Upper and Lower Clay were reported as follows:

- Upper Clay- $c' = 14.5$ kPa and $\phi' = 13.3^\circ$ and

- Lower Clay- $c' = 7.7$ kPa and $\phi' = 15.7^\circ$.

Typical industry accepted effective shear strength parameters used in the Winnipeg area for the glacio-lacustrine clay are summarised in **Table 3-8**.

Table 3-8: Glacio-Lacustrine Clay - Published Effective Shear Strength Parameters

Parameter	Value
Effective Cohesion (c'), kPa	5.0
Effective Friction Angle (ϕ'), degrees	14.0

3.2.4.2 Geotechnical Investigation Findings

A layer of glacio-lacustrine clay was encountered beneath the Upper Complex in all test holes completed as part of the AECOM 2012, AECOM 2015, and AECOM 2019 investigations except for test holes TH11-01 and TH11-02. The glacio-lacustrine clay ranged in thickness from 10.7 m to 15.7 m (13.3 m average) in test holes along the proposed trunk sewer alignment that were advanced through the glacio-lacustrine clay layer into the underlying till. The glacio-lacustrine clay ranged in thickness from 13.1 m to 15.8 m (14.2 m average) in test holes offset from the proposed trunk sewer alignment that were advanced through the glacio-lacustrine clay layer into the underlying till.

The glacio-lacustrine clay generally contained trace silt to silty, trace sand, trace gravel, and was brown to grey, soft to stiff, and of high plasticity. In test hole TH11-14 a 0.9 m silt interlayer with a moisture content of 13% was encountered within the glacio-lacustrine clay layer. In test hole TH19-05 suspected gravel and/or cobble was inferred from the deformed shape of the recovered Shelby Tube pushed at an elevation of 217.6 m.

A summary of the laboratory testing results for the glacio-lacustrine clay layers encountered along the proposed alignment as part of the AECOM 2012 and AECOM 2019 investigations are presented in **Table 3-9**.

**Table 3-9: Glacio-Lacustrine Clay - Summary of Laboratory Testing Along Proposed Alignment
(AECOM 2012, AECOM 2019)**

Laboratory Test	Minimum	Average	Maximum
Moisture Content (%)	22	47	64
Atterberg - Plastic Limit (%)	14	23	31
Atterberg - Liquid Limit (%)	50	72	90
Atterberg – Plasticity Index (%)	35	49	69
Grain Size - Gravel (%)	0	0	2
Grain Size - Sand (%)	0	3	19
Grain Size - Silt (%)	12	23	35
Grain Size - Clay (%)	44	74	88
Unconfined Compressive Strength - Undrained Shear Strength (kPa)	17	39	63
Pocket Penetrometer - Undrained Shear Strength (kPa)	12	50	79
Torvane - Undrained Shear Strength (kPa)	25	50	66
Bulk Unit Weight (kN/m ³)	15.6	16.7	18.8
Permeability (cm/s)	1.52 x 10 ⁻⁸	2.25 x 10 ⁻⁸	2.98 x 10 ⁻⁸
Free Swell (%)	1.9	2.6	3.4
Swelling Pressure (kPa)	35	68	120
pH	8.0	8.1	8.3
Resistivity (ohm*cm)	561	1620	3580
Conductivity (mS/cm)	0.3	1.8	1.0
Sulphate Content (mg/kg)	30	590	927

A summary of the laboratory testing results for the glacio-lacustrine clay layers encountered offset from the proposed alignment as part of the AECOM 2012 and AECOM 2015 investigations are presented in Table 3-10.

**Table 3-10: Glacio-Lacustrine Clay - Summary of Laboratory Testing Offset from Proposed
Alignment
(AECOM 2012, AECOM 2015)**

Laboratory Test	Minimum	Average	Maximum
Moisture Content (%)	27	48	62
Atterberg - Plastic Limit (%)	24	29	31
Atterberg - Liquid Limit (%)	80	85	92
Atterberg – Plasticity Index (%)	53	57	62
Grain Size - Gravel (%)		0	
Grain Size - Sand (%)	0	1	2
Grain Size - Silt (%)	15	16	17
Grain Size - Clay (%)	81	83	85
Unconfined Compressive Strength - Undrained Shear Strength (kPa)	53	68	93
Pocket Penetrometer - Undrained Shear Strength (kPa)	12	42	110
Torvane - Undrained Shear Strength (kPa)	12	38	93
Bulk Unit Weight (kN/m ³)	17.1	17.2	17.3

Plots of moisture content, Atterberg Limits, and undrained shear strength with elevation for the glacio-lacustrine soil deposits encountered in the AECOM 2012, AECOM 2015, and AECOM 2019 investigations are illustrated in **Figure 3-2** and **Figure 3-3** below

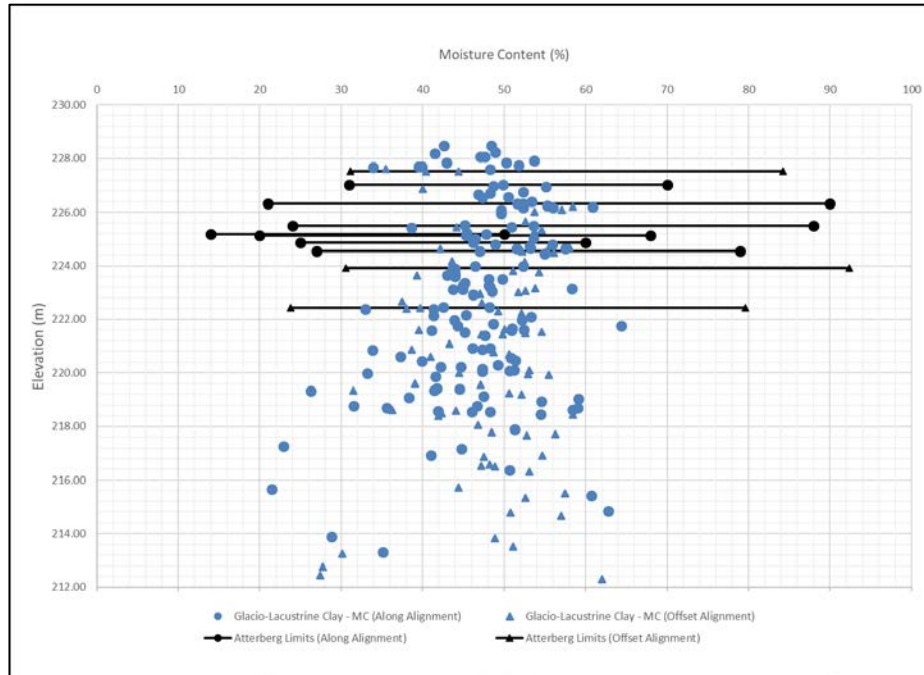


Figure 3-2: Moisture Content & Atterberg Limits with Elevation for Glacio-Lacustrine Clay (AECOM 2012, AECOM 2015, AECOM 2019)

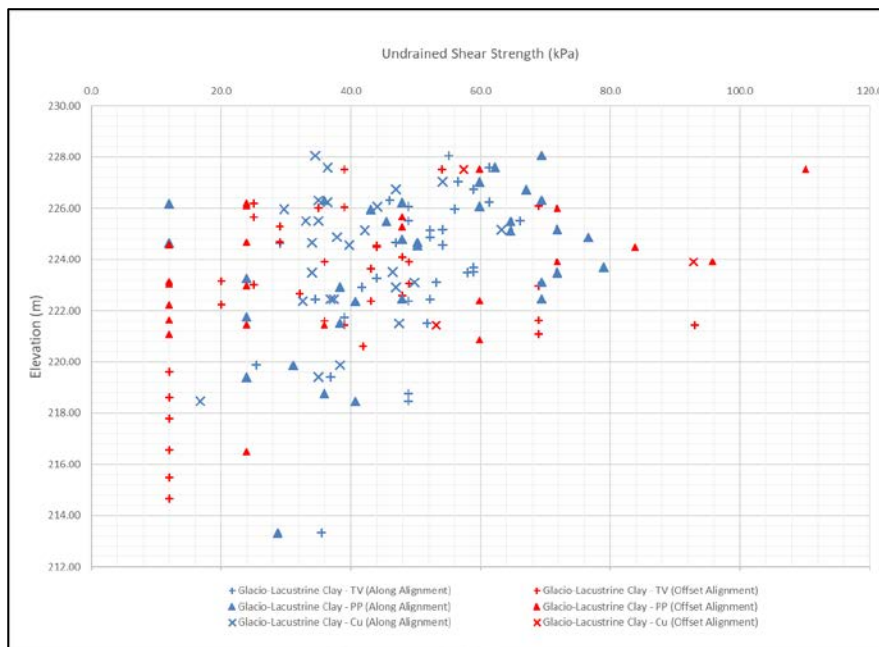


Figure 3-3: Undrained Shear Strength with Elevation for Glacio-Lacustrine Clay (AECOM 2012, AECOM 2015, AECOM 2019)

The reported laboratory test results are generally consistent with the published findings for the glacio-lacustrine clay within the Winnipeg area. The trend of the undrained shear strength profile (as shown in Figure 3-3) for the glacio-lacustrine clay showed lower undrained shear strength values closer to the clay/glacial till boundary.

3.2.5 Glacial Till

A glacial till layer was encountered during the AECOM 2012, AECOM 2015, and AECOM 2019 investigations. When considering test holes along the proposed alignment drilled during the AECOM 2019 investigation, the contact elevation of the glacial till layer was noted to be highest at the west end of the proposed alignment and generally decreased in elevation along the alignment towards the river. The glacial till was noted to overlie the carbonate bedrock in test hole SI15-01.

The profile of the glacial till layer encountered as part of the AECOM 2012 and AECOM 2019 investigations along the proposed alignment are outlined in **Table 3-11**.

**Table 3-11: Glacial Till - Soil Profile Along Proposed Alignment
(AECOM 2012, AECOM 2019)**

Location	Test Hole	Depth (m BGS)	Till Contact Elevation (m)
Section 1 (Station 0+202 to 0+600)	TH19-01	12.5	218.6
	TH19-05	14.3	217.0
Section 2 (Station 0+600 to 1+000)	TH19-08	15.1	215.9
Section 3 (Station 1+000 to 1+500)	TH19-11	16.2	214.7
Section 4 (Station 1+500 to 1+742)	TH19-14	18.1	212.5
	TH19-16	16.0	212.6

Notes: BGS – Below Ground Surface

The profile of the glacial till layer encountered as part of the AECOM 2012 and AECOM 2015 investigation offset from the proposed alignment are outlined in **Table 3-12**.

**Table 3-12: Glacial Till - Soil Profile Offset from Proposed Alignment
(AECOM 2012, AECOM 2015)**

Location	Test Hole	Depth (m BGS)	Till Contact Elevation (m)
Jefferson East CSR (AECOM 2012)	SP11-13	18.6	212.0
	TH11-14	14.0	212.9
Outfall Structure (AECOM 2015)	SI15-01	15.5	211.5*

Notes: BGS – Below Ground Surface; * Drilled locations not surveyed. Elevations were inferred.

3.2.5.1 Reported Geotechnical Properties

Within the Winnipeg area, the composition of the glacial till deposit is highly variable and its density varies both with depth and distance. Near the glacio-lacustrine/glacial till interface, the upper zone of the till is typically characterized by a softer sub-unit (locally termed “putty till”) and has a typical moisture content

ranging from 10 and 15 percent. The lower sub-unit has typical in-situ moisture content values of between 7 and 10 percent.

Reported unconfined compressive strength values of the very dense tills (with in-situ moisture contents of 5 percent) range between 3.4 and 3.6 MPa (*Baracos, A.G. Shields, D.H., and Kjartenson, B. 1983*). The elastic modulus of the glacial till soils has also been reported at a range of between 170 and 240 MPa (*Baracos, A.G. Shields, D.H., and Kjartenson, B. 1983*). These parameters are based upon the results of past material testing performed on representative samples of glacial till deposits from within the Winnipeg area.

3.2.5.2 Geotechnical Investigation Findings

The glacial till was generally described as silt and sand containing some clay to clayey, trace to some gravel, and was light brown, compact to very dense, dry to wet, and of low plasticity. The consistency of the glacial till generally increased in strength with depth. Whilst not confirmed during the advancement of the AECOM 2012, AECOM 2015, and AECOM 2019 test holes, the glacial till is suspected to contain cobble and boulder size obstructions.

A summary of the laboratory testing results for the glacial till layer encountered along the proposed alignment as part of the AECOM 2012 and AECOM 2019 investigations are presented in **Table 3-13**.

**Table 3-13: Glacial Till - Summary of Laboratory Testing Along Proposed Alignment
 (AECOM 2012, AECOM 2019)**

Laboratory Test	Minimum	Average	Maximum
Moisture Content (%)	9	15	38
SPT 'N' Blow Counts (uncorrected)	17	45	≥ 50
Atterberg - Plastic Limit (%)		10	
Atterberg - Liquid Limit (%)		22	
Grain Size - Gravel (%)		0	
Grain Size - Sand (%)		35	
Grain Size - Silt (%)		44	
Grain Size - Clay (%)		21	
Pocket Penetrometer - Undrained Shear Strength (kPa)		36	
Torvane - Undrained Shear Strength (kPa)		49	

A summary of the laboratory testing results for the glacio-lacustrine clay layers encountered offset from the proposed alignment as part of the AECOM 2012 and AECOM 2015 investigations are presented in **Table 3-14**.

Table 3-14: Glacial Till - Summary of Laboratory Testing Offset from Proposed Alignment (AECOM 2012, AECOM 2015)

Laboratory Test	Minimum	Average	Maximum
Moisture Content (%)	12	28	54
Torvane - Undrained Shear Strength (kPa)	12		

Plots of moisture content and Atterberg Limits with elevation for the glacial till encountered in the AECOM 2012, AECOM 2015, and AECOM 2019 investigations are illustrated in **Figure 3-4** below

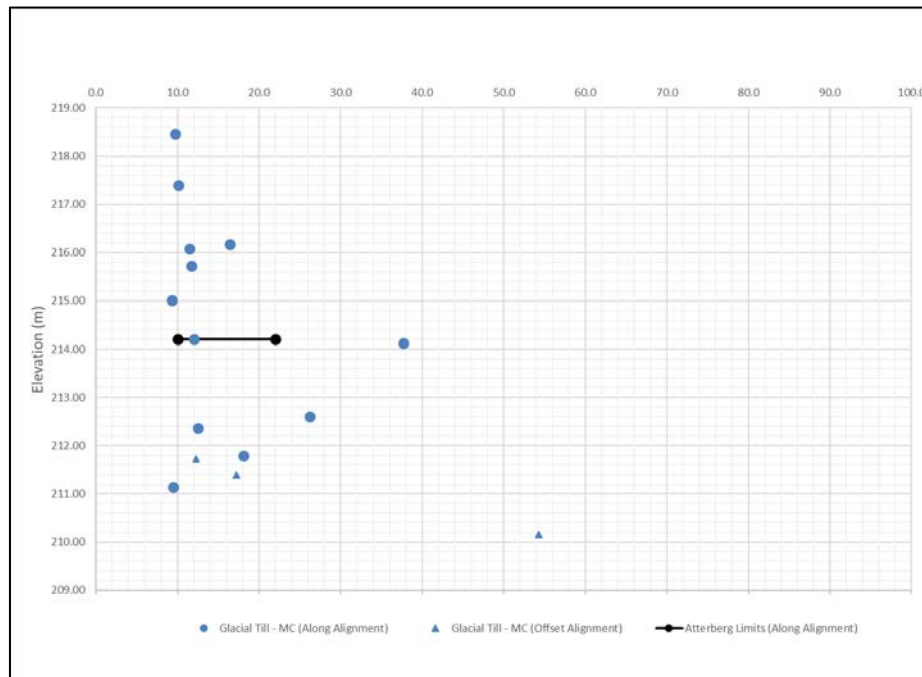


Figure 3-4: Moisture Content & Atterberg Limits with Elevation for Glacial Till (AECOM 2012, AECOM 2015, AECOM 2019)

3.2.6 Carbonate Bedrock

Carbonate bedrock was encountered below the glacial till in one of the AECOM 2015 test holes drilled offset from the proposed alignment. The carbonate bedrock from test hole SI15-01 was encountered at an elevation of 205.2 m and was classified as limestone. These findings are generally consistent with the pre-established bedrock mapping of the area and published literature.

3.3 Groundwater Conditions

Groundwater depths were measured within the monitoring wells installed as part of the AECOM 2019 geotechnical investigation and are summarized in the following section. Groundwater monitoring records from previous geotechnical investigations are also included.

3.3.1 AECOM 2019 Geotechnical Investigation

To assess groundwater levels along the proposed trunk sewer alignment, three (3) standpipe piezometers were installed in test holes TH19-01, TH19-05, and TH19-16 at varying depths and within varying soil units. Short term monitoring results of the groundwater level (GWL) from the instruments installed at the site as part of the AECOM 2019 investigation are provided in **Table 3-15**. Sloughing was observed from the glacial till layer within test holes TH19-01 and TH19-16, and from the Upper Complex silt layer in TH19-05. It should be noted that groundwater levels and subsequently sloughing may change seasonally, annually or as a result of construction activities.

**Table 3-15: Summary of GWL Monitoring Results
(AECOM 2019)**

Location	Test Hole ID	Ground Elev. (m)	Tip Elev. (m)	Soil Unit	Monitoring Date	Depth (m BGS)	GWL Elev. (m)
Section 1 (Station 0+202 to 0+600)	TH19-01	231.11	218.41	Glacial Till	Aug-06-2019	4.13	226.98
					Aug-20-2019	4.21	226.90
					Sept-03-2019	4.17	226.94
	TH19-05	231.32	217.60	Glacial Till	Aug-06-2019	7.76	223.55
					Aug-20-2019	6.55	224.76
					Sept-03-2019	6.11	225.20
Section 4 (Station 1+500 to 1+742)	TH19-16	228.55	221.23	Glacio. Clay	Aug-06-2019	2.91	225.64
					Aug-20-2019	2.93	225.63
					Sept-03-2019	2.94	225.61

Notes: BGS – Below Ground Surface

3.3.2 Previous Geotechnical Investigations

One (1) standpipe piezometer was installed in test hole SP11-13 as part of the AECOM 2012 investigation, and one (1) vibrating wire piezometer was installed as part of the AECOM 2015 investigation.

Results for the vibrating wire piezometers over the reported period indicated nearly constant negative piezometric head (i.e. piezometric elevation is below tip elevation). The development of negative head is likely not credible and may be attributed to instruments malfunction. As a result, the monitoring results of the vibrating wire piezometer have not been presented in this report. The groundwater monitoring results from the AECOM 2012 standpipe piezometer are summarized in **Table 3-16**.

**Table 3-16: Summary of GWL Monitoring Results
(AECOM 2012)**

Location	Test Hole ID	Ground Elev. (m)	Tip Elev. (m)	Soil Unit	Monitoring Date	Depth (m BGS)	GWL Elev. (m)
Jefferson East CSR	SP11-13	230.58	211.08	Glacial Till	Jan-06-2012	7.80	222.78
					Feb-24-2015	7.50	223.03
					Mar-13-2015	7.50	223.03
					May-19-2015	7.40	223.14
					Aug-28-2015	7.50	223.12
					Oct-07-2015	7.70	222.91
					Dec-07-2015	7.70	222.91
					Feb-03-2016	7.70	222.88

Notes: BGS – Below Ground Surface

4. References

- 1- AECOM Canada Ltd. (2012). City of Winnipeg Jefferson East Sub-Surface Investigation Geotechnical Memo.
- 2- AECOM Canada Ltd. (2015). City of Winnipeg Construction of Outfall Chamber and Piping Jefferson East CSR – Waterway Application – Supplementary Geotechnical Investigation Letter.
- 3- Bernhardt, Darren (2018). Ghost creeks: Winnipeg buried many waterways that could have changed city's shape. CBC News.
- 4- Essex, R.J. (2007). Geotechnical Baseline Reports for Construction, Suggested Guidelines. American Society of Civil Engineers.
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- 6- Matile, G.L.D. (2004). Surficial Geology, Winnipeg, Manitoba Geoscientific Map MAP2003-7.
- 7- Manitoba Energy and Mines (1990). Bedrock Geology Compilation Map Series NTS 62H.
- 8- Graham, J., and Shields, D.H (1985). Influence of geology and geological processes on the geotechnical properties of a plastic clay. Engineering Geology.
- 9- ASTM D4546-14. Standard Test Methods for One-Dimensional Swell or Collapse of Soils.
- 10- Baracos, A.G. Shields, D.H., and Kjartenson, B. (1983). Geological Engineering Report for Urban Development of Winnipeg, University of Manitoba- Department of Geological Engineering.

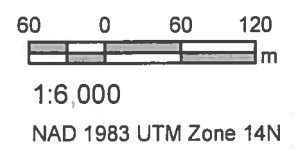
Appendix **A**

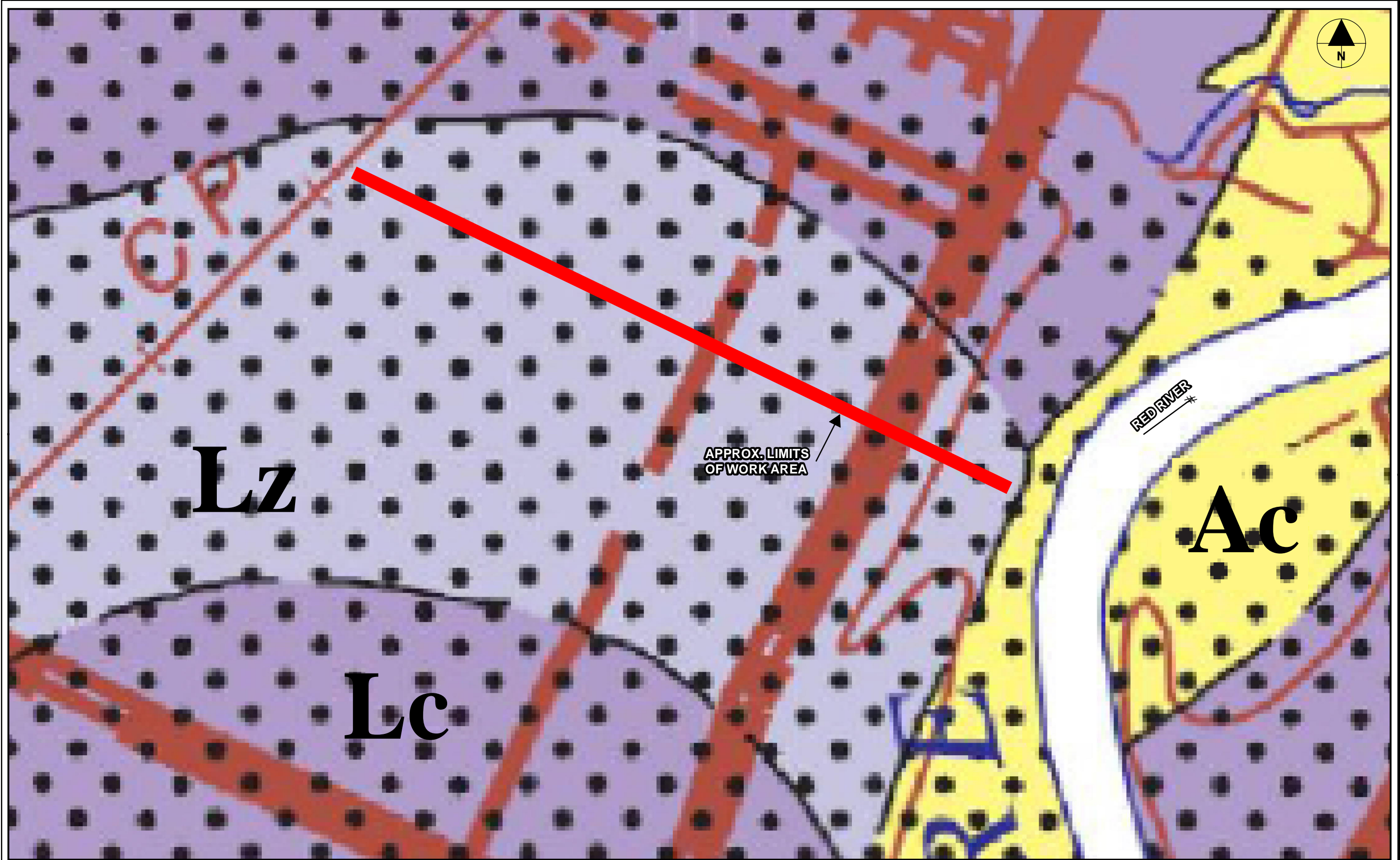
Figures

- Figure 1: Site Location Plan and Semple Avenue Trunk Sewer Alignment
- Figure 2: Surficial Geology
- Figure 3: Test Hole Location Plan
- Figure 4A to 4E: Stratigraphic Section of Semple Avenue Trunk Sewer Alignment



- LEGEND**
- SEMPLE AVE - TRUNK SEWER ALIGNMENT
 - EXISTING LDS AND OUTFALL
 - MANHOLE





SURFICIAL GEOLOGY LEGEND

LITHOLOGY (MATERIAL)	
	CHANNEL DEPOSITS
	CLAY TO SILTY CLAY
	CLAYEY TO SANDY SILT

LITHOGENESIS (ORIGINS OF MATERIAL)

ALLUVIAL SEDIMENTS: SAND AND GRAVEL, SAND, SILT, CLAY, ORGANIC DETRITUS; 1-20 m THICK; CHANNEL AND OVERBANK SEDIMENTS; DEPOSITED BY POSTGLACIAL RIVERS.
OFFSHORE GLACIOLACUSTRINE SEDIMENTS: CLAY, SILT, MINOR SAND; 1-20 m THICK; VERY LOW RELIEF MASSIVE AND LAMINATED DEPOSITS; DEPOSITED FROM SUSPENSION IN OFFSHORE, DEEP WATER OF GLACIAL LAKE AGASSIZ, COMMONLY SCOURED AND HOMOGENIZED BY ICEBERGS.
OFFSHORE GLACIOLACUSTRINE SEDIMENTS: CLAY, SILT, MINOR SAND; 1-20 m THICK; VERY LOW RELIEF MASSIVE AND LAMINATED DEPOSITS; DEPOSITED FROM SUSPENSION IN OFFSHORE, DEEP WATER OF GLACIAL LAKE AGASSIZ, COMMONLY SCOURED AND HOMOGENIZED BY ICEBERGS.

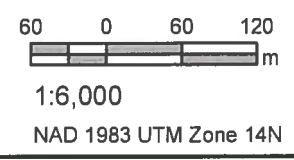
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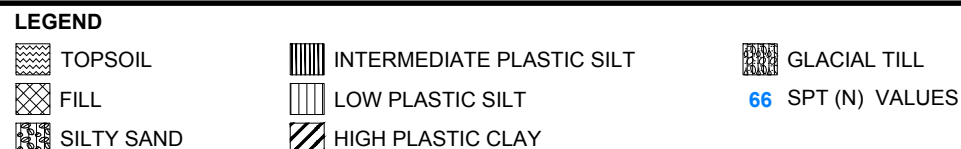
LEGEND

- SEMPLE AVE - TRUNK SEWER ALIGNMENT
- EXISTING LDS AND OUTFALL
- MANHOLE
- ⊕ TEST HOLE (AECOM 2012)
- ⊕ TEST HOLE (AECOM 2015)
- ⊕ TEST HOLE (AECOM 2019)
- ⊕ PIEZOMETER TEST HOLE (AECOM 2012)
- ⊕ PIEZOMETER TEST HOLE (AECOM 2015)
- ⊕ PIEZOMETER TEST HOLE (AECOM 2019)

NOTE:
LOCATION OF AECOM 2015
TEST HOLES ARE APPROXIMATE.



TEST HOLE LOCATION PLAN

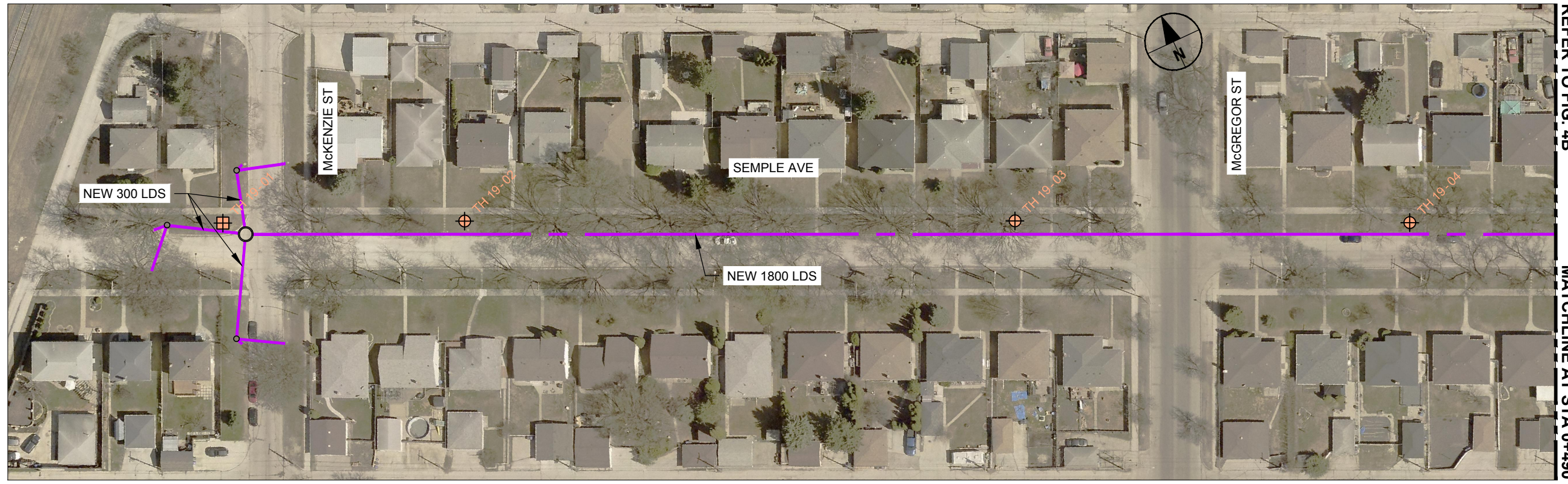
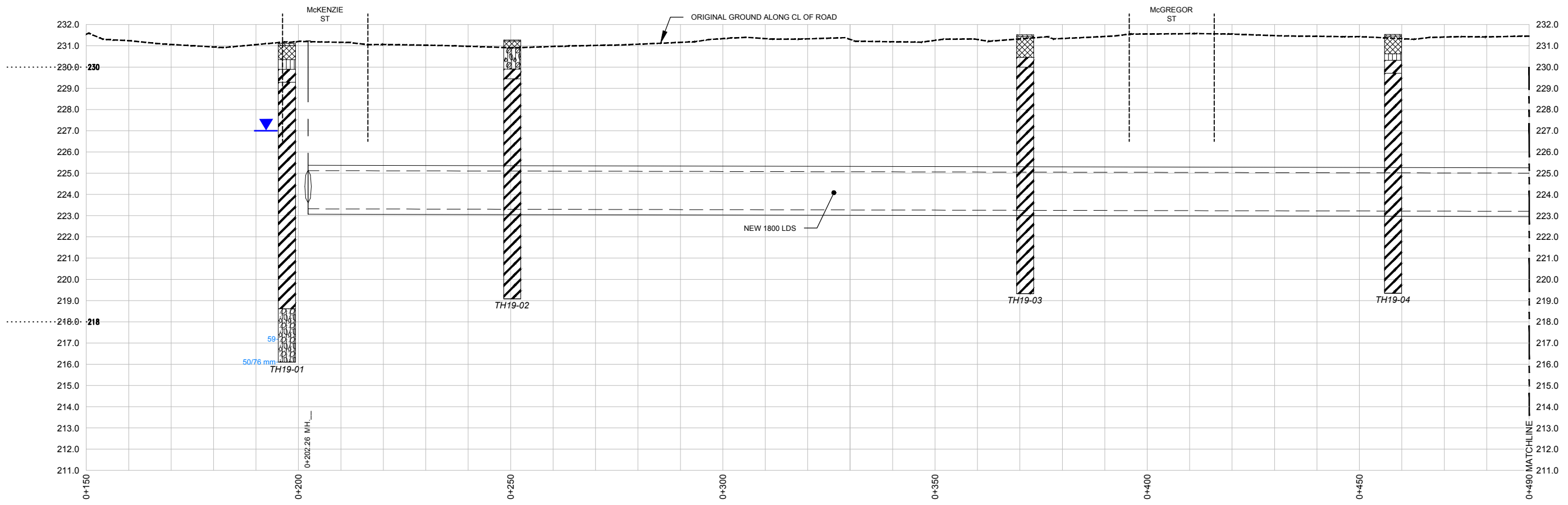


NOTE 1: THIS FIGURE SHOULD BE USED FOR BASELINE PURPOSES ONLY AND SHOULD BE READ IN CONJUNCTION WITH THE GEOTECHNICAL BASELINE (GBR) AND DATA REPORT (GDR). THIS FIGURE PROVIDES BASELINE STRATIGRAPHIC CROSS SECTION ALONG THE TUNNEL SECTIONS ONLY. FOR BASELINE STRATIGRAPHIC CONDITIONS AT THE SHAFT LOCATIONS, REFER TO GBR.

NOTE 2: SUBSURFACE CONDITIONS ARE KNOWN ONLY AT THE TEST HOLE LOCATIONS. THE ACTUAL GROUND CONDITIONS BETWEEN THE TEST HOLES MAY VARY.

NOTE 3: DETAILED DESCRIPTIONS OF MATERIALS, CHARACTERISTICS AND VARIABILITY ANTICIPATED WITHIN EACH SOIL UNIT ARE PRESENTED IN THE GDR AND GBR. FOR DETAILS OF THE TEST HOLE LOGS AND GROUNDWATER MEASUREMENTS REFER TO GDR.








NOTE 4: LOCATION OF EXISTING UTILITIES ARE NOT SHOWN. FOR DETAILS ON EXISTING UTILITY LOCATIONS REFER TO DRAWINGS/SPECS.



REFER TO FIG. 4B

MATCHLINE AT STA 0+490

LEGEND

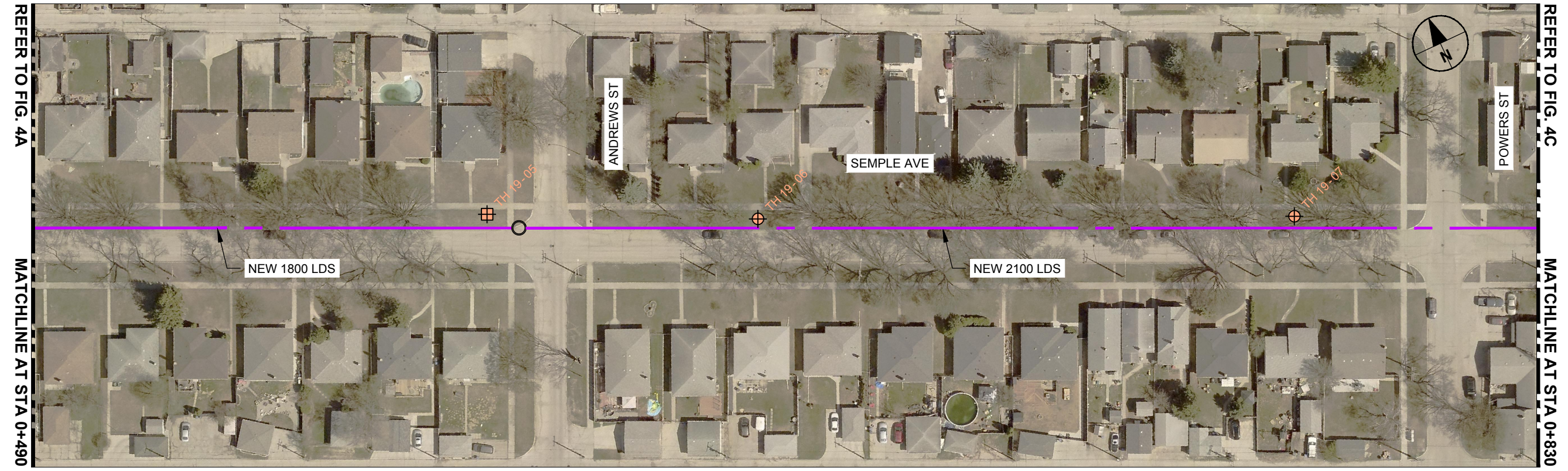
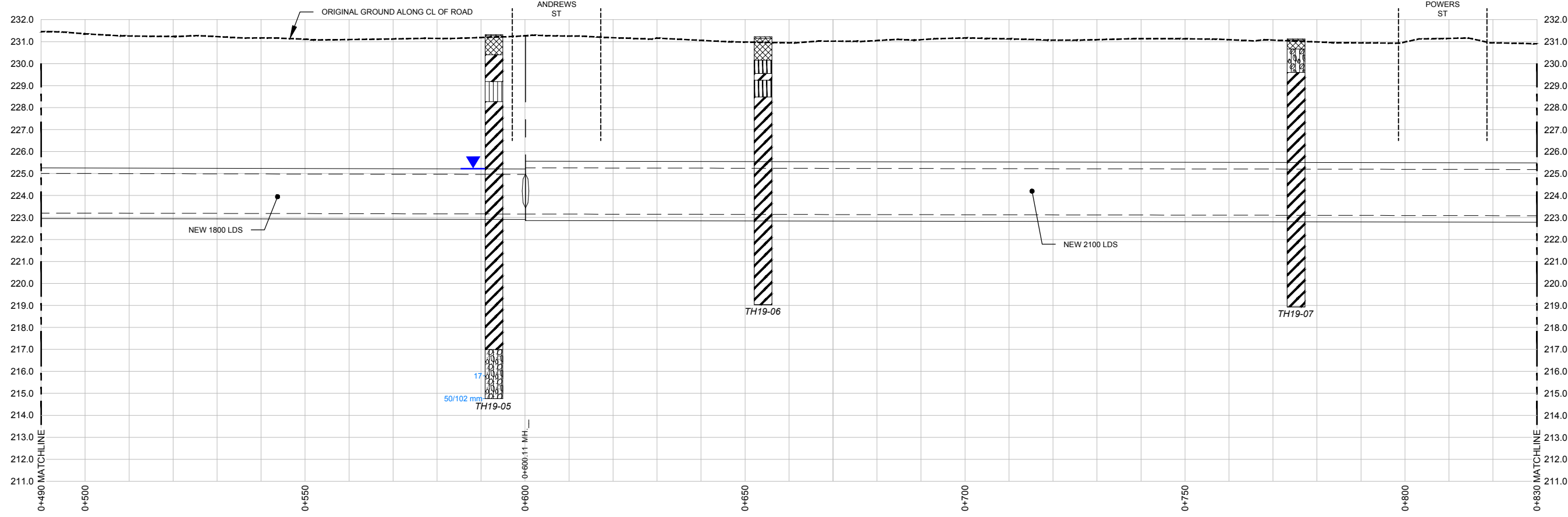
-  TOPSOIL
-  FILL
-  SILTY SAND
-  INTERMEDIATE PLASTIC SILT
-  LOW PLASTIC SILT
-  HIGH PLASTIC CLAY
-  GLACIAL TILL
- 66** SPT (N) VALUES

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NOTE 2: SUBSURFACE CONDITIONS ARE KNOWN ONLY AT THE TEST HOLE LOCATIONS. THE ACTUAL GROUND CONDITIONS BETWEEN THE TEST HOLES MAY VARY.

NOTE 3: DETAILED DESCRIPTIONS OF MATERIALS, CHARACTERISTICS AND VARIABILITY ANTICIPATED WITHIN EACH SOIL UNIT ARE PRESENTED IN THE GDR AND GBR. FOR DETAILS OF THE TEST HOLE LOGS AND GROUNDWATER MEASUREMENTS REFER TO GDR.

NOTE 4: LOCATION OF EXISTING UTILITIES ARE NOT SHOWN. FOR DETAILS ON EXISTING UTILITY LOCATIONS REFER TO DRAWINGS/SPECS.











REFER TO FIG. 4A

MATCHLINE AT STA 0+490

REFER TO FIG. 4C

MATCHLINE AT STA 0+830

LEGEND

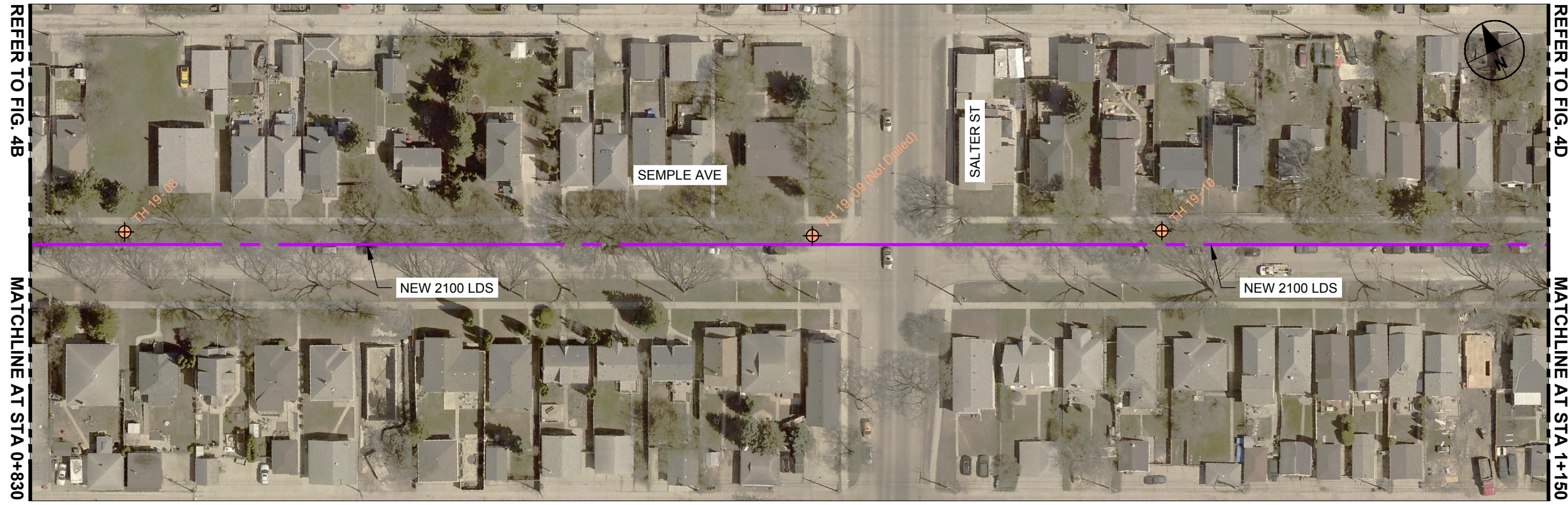
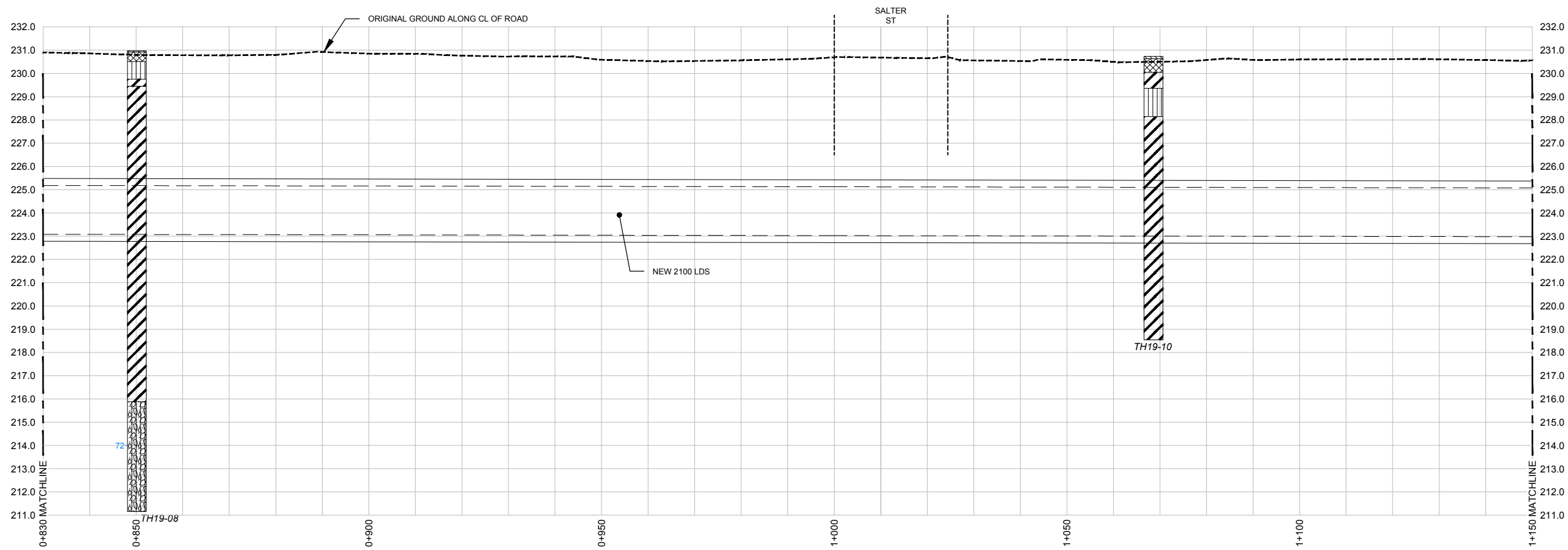
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-  FILL
-  SILTY SAND
-  INTERMEDIATE PLASTIC SILT
-  LOW PLASTIC SILT
-  HIGH PLASTIC CLAY
-  GLACIAL TILL
-  66 SPT (N) VALUES

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NOTE 2: SUBSURFACE CONDITIONS ARE KNOWN ONLY AT THE TEST HOLE LOCATIONS. THE ACTUAL GROUND CONDITIONS BETWEEN THE TEST HOLES MAY VARY.

NOTE 3: DETAILED DESCRIPTIONS OF MATERIALS, CHARACTERISTICS AND VARIABILITY ANTICIPATED WITHIN EACH SOIL UNIT ARE PRESENTED IN THE GDR AND GBR. FOR DETAILS OF THE TEST HOLE LOGS AND GROUNDWATER MEASUREMENTS REFER TO GDR.



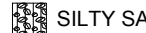




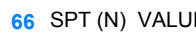
NOTE 4: LOCATION OF EXISTING UTILITIES ARE NOT SHOWN. FOR DETAILS ON EXISTING UTILITY LOCATIONS REFER TO DRAWINGS/SPECS.



STRATIGRAPHIC SECTION OF SEMPLE AVENUE
TRUNK SEWER ALIGNMENT
STN 0+830 TO STN 1+150

SEMPLA AVENUE TRUNK SEWER
GEOTECHNICAL DATA REPORT
CITY OF WINNIPEG, WATER AND WASTE DEPARTMENT

LEGEND

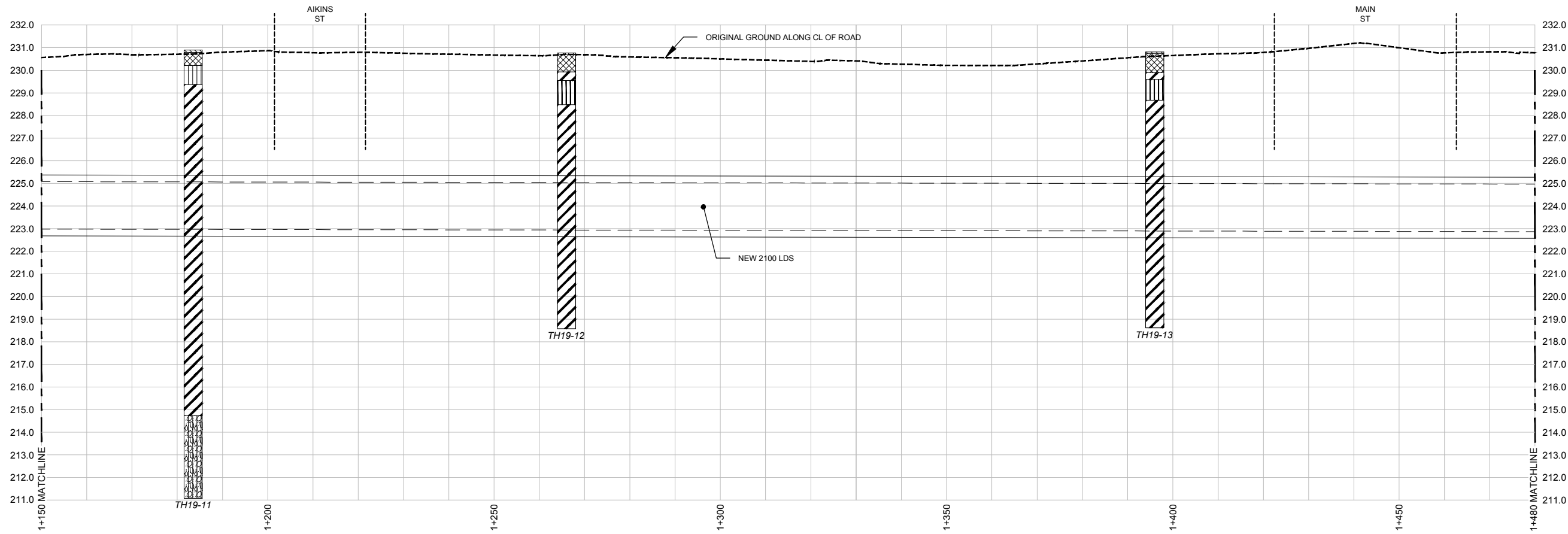
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-  INTERMEDIATE PLASTIC SILT
-  LOW PLASTIC SILT
-  HIGH PLASTIC CLAY
-  GLACIAL TILL
-  66 SPT (N) VALUES

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NOTE 2: SUBSURFACE CONDITIONS ARE KNOWN ONLY AT THE TEST HOLE LOCATIONS. THE ACTUAL GROUND CONDITIONS BETWEEN THE TEST HOLES MAY VARY.

NOTE 3: DETAILED DESCRIPTIONS OF MATERIALS, CHARACTERISTICS AND VARIABILITY ANTICIPATED WITHIN EACH SOIL UNIT ARE PRESENTED IN THE GDR AND GBR. FOR DETAILS OF THE TEST HOLE LOGS AND GROUNDWATER MEASUREMENTS REFER TO GDR.

NOTE 4: LOCATION OF EXISTING UTILITIES ARE NOT SHOWN. FOR DETAILS ON EXISTING UTILITY LOCATIONS REFER TO DRAWINGS/SPECS.



REFER TO FIG. 4C

MATCHLINE AT STA 1+150





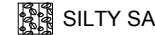




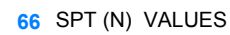
REFER TO FIG. 4E

MATCHLINE AT STA 1+480

**STRATIGRAPHIC SECTION OF SEMPLE AVENUE
TRUNK SEWER ALIGNMENT
STN 1+150 TO STN 1+480**

**SEMPLE AVENUE TRUNK SEWER
GEOTECHNICAL DATA REPORT
CITY OF WINNIPEG, WATER AND WASTE DEPARTMENT**

LEGEND

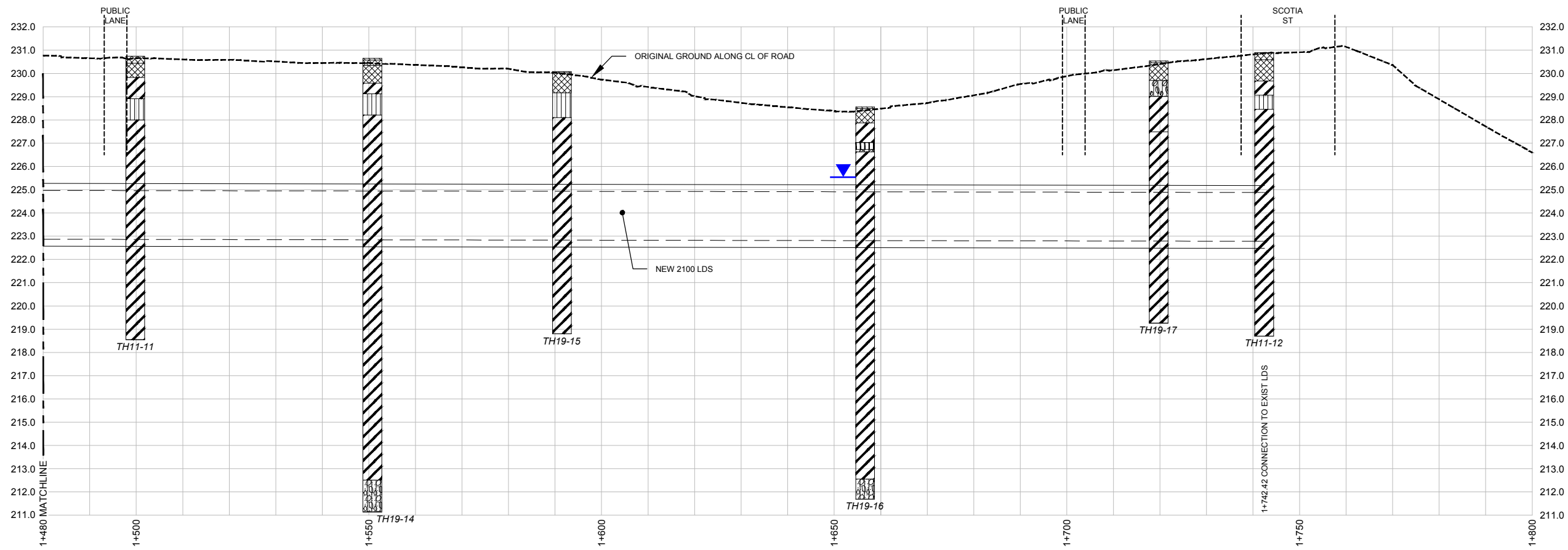
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-  INTERMEDIATE PLASTIC SILT
-  LOW PLASTIC SILT
-  HIGH PLASTIC CLAY
-  GLACIAL TILL
-  66 SPT (N) VALUES

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NOTE 3: DETAILED DESCRIPTIONS OF MATERIALS, CHARACTERISTICS AND VARIABILITY ANTICIPATED WITHIN EACH SOIL UNIT ARE PRESENTED IN THE GDR AND GBR. FOR DETAILS OF THE TEST HOLE LOGS AND GROUNDWATER MEASUREMENTS REFER TO GDR.

NOTE 4: LOCATION OF EXISTING UTILITIES ARE NOT SHOWN. FOR DETAILS ON EXISTING UTILITY LOCATIONS REFER TO DRAWINGS/SPECS.



REFER TO FIG. 4D

MATCHLINE AT STA 1+480

STRATIGRAPHIC SECTION OF SEMPLE AVENUE
TRUNK SEWER ALIGNMENT
STN 1+480 TO STN 1+800

SEMPLE AVENUE TRUNK SEWER
GEOTECHNICAL DATA REPORT
CITY OF WINNIPEG, WATER AND WASTE DEPARTMENT

Appendix **B**

Previous Geotechnical Investigations Test Hole Logs

- B-1: AECOM (February 2012) Test Hole Logs
- B-2: AECOM (October 2015) Test Hole Logs

AECOM Canada Ltd.

GENERAL STATEMENT

NORMAL VARIABILITY OF SUBSURFACE CONDITIONS

The scope of the investigation presented herein is limited to an investigation of the subsurface conditions as to suitability for the proposed project. This report has been prepared to aid in the evaluation of the site and to assist the engineer in the design of the facilities. Our description of the project represents our understanding of the significant aspects of the project relevant to the design and construction of earth work, foundations and similar. In the event of any changes in the basic design or location of the structures as outlined in this report or plan, we should be given the opportunity to review the changes and to modify or reaffirm in writing the conclusions and recommendations of this report.

The analysis and recommendations presented in this report are based on the data obtained from the borings and test pit excavations made at the locations indicated on the site plans and from other information discussed herein. This report is based on the assumption that the subsurface conditions everywhere are not significantly different from those disclosed by the borings and excavations. However, variations in soil conditions may exist between the excavations and, also, general groundwater levels and conditions may fluctuate from time to time. The nature and extent of the variations may not become evident until construction. If subsurface conditions differ from those encountered in the exploratory borings and excavations, are observed or encountered during construction, or appear to be present beneath or beyond excavations, we should be advised at once so that we can observe and review these conditions and reconsider our recommendations where necessary.

Since it is possible for conditions to vary from those assumed in the analysis and upon which our conclusions and recommendations are based, a contingency fund should be included in the construction budget to allow for the possibility of variations which may result in modification of the design and construction procedures.

In order to observe compliance with the design concepts, specifications or recommendations and to allow design changes in the event that subsurface conditions differ from those anticipated, we recommend that all construction operations dealing with earth work and the foundations be observed by an experienced soils engineer. We can be retained to provide these services for you during construction. In addition, we can be retained to review the plans and specifications that have been prepared to check for substantial conformance with the conclusions and recommendations contained in our report.

EXPLANATION OF FIELD & LABORATORY TEST DATA

The field and laboratory test results, as shown for each hole, are described below.

1. NATURAL MOISTURE CONTENT

The relationship between the natural moisture content and depth is significant in determining the subsurface moisture conditions. The Atterberg Limits for a sample should be compared to its natural moisture content and plotted on the Plasticity Chart in order to determine the soil classification.

2. SOIL PROFILE AND DESCRIPTION

Each soil stratum is classified and described noting any special conditions. The Modified Unified Classification System (MUCS) is used. The soil profile refers to the existing ground level at the time the hole was done. Where available, the ground elevation is shown. The soil symbols used are shown in detail on the soil classification chart.

3. TESTS ON SOIL SAMPLES

Laboratory and field tests are identified by the following and are on the logs:

- N - Standard Penetration Test (SPT) Blow Count. The SPT is conducted in the field to assess the in-situ consistency of cohesive soils and the relative density of non-cohesive soils. The N value recorded is the number of blows from a 63.5 kg hammer dropped 760 mm which is required to drive a 51 mm split spoon sampler 300 mm into the soil.

- SO₄ - Water Soluble Sulphate Content. Expressed in percent. Conducted primarily to determine requirements for the use of sulphate resistant cement. Further details on the water-soluble sulphate content are given in Section 6.

- γ_D - Dry Unit Weight. Usually expressed in kN/m³.

- γ_T - Total Unit Weight. Usually expressed in kN/m³.

- Q_u - Unconfined Compressive Strength. Usually expressed in kPa and may be used in determining allowable bearing capacity of the soil.

- C_u - Undrained Shear Strength. Usually expressed in kPa. This value is determined by either a direct shear test or by an unconfined compression test and may also be used in determining the allowable bearing capacity of the soil.
- C_{PEN} - Pocket Penetrometer Reading. Usually expressed in kPa. Estimate of the undrained shear strength as determined by a pocket penetrometer.

The following tests may also be performed on selected soil samples and the results are given on separate sheets enclosed with the logs:

- Grain Size Analysis
- Standard or Modified Proctor Compaction Test
- California Bearing Ratio Test
- Direct Shear Test
- Permeability Test
- Consolidation Test
- Triaxial Test

4. SOIL DENSITY AND CONSISTENCY

The SPT test described above may be used to estimate the consistency of cohesive soils and the density of cohesionless soils. These approximate relationships are summarized in the following tables:

Table 1 Cohesive Soils

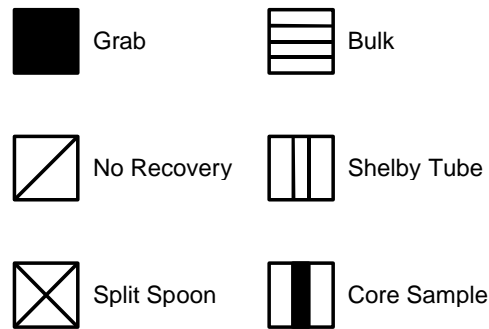
N	Consistency	C _u (kPa) approx.
0 - 1	Very Soft	<10
1 - 4	Soft	10 - 25
4 - 8	Firm	25 - 50
8 - 15	Stiff	50 - 100
15 - 30	Very Stiff	100 - 200
30 - 60	Hard	200 - 300
>60	Very Hard	>300

Table 2 Cohesionless Soils

N	Density
0 - 5	Very Loose
5 - 10	Loose
10 - 30	Compact
30 - 50	Dense
>50	Very Dense

5. SAMPLE CONDITION AND TYPE

The depth, type, and condition of samples are indicated on the logs by the following symbols:



6. WATER SOLUBLE SULPHATE CONCENTRATION

The following table, from CSA Standard A23.1-14, indicates the requirements for concrete subjected to sulphate attack based upon the percentage of water-soluble sulphate as presented on the logs. CSA Standard A23.1-14 should be read in conjunction with the table.

Table 3 Requirements for Concrete Subjected to Sulphate Attack*

Class of exposure	Degree of exposure	Water-soluble sulphate (SO ₄) [†] in soil sample, %	Sulphate (SO ₄) [‡] in groundwater samples, mg/L [‡]	Water soluble sulphate (SO ₄) in recycled aggregate sample, %	Cementing materials to be used ^{§††}	Performance requirements ^{§,§§}		
						Maximum expansion when tested using CSA A3004-C8 Procedure A at 23 °C, %		Maximum expansion when tested using CSA A3004-C8 Procedure B at 5 °C, % ^{†††}
						At 6 months	At 12 months ^{††}	At 18 months ^{‡‡}
S-1	Very severe	> 2.0	> 10 000	> 2.0	HS ^{**} , HSb, HSLb ^{***} or HSe	0.05	0.10	0.10
S-2	Severe	0.20–2.0	1500–10 000	0.60–2.0	HS ^{**} , HSb, HSLb ^{***} or HSe	0.05	0.10	0.10
S-3	Moderate (including seawater exposure*)	0.10–0.20	150–1500	0.20–0.60	MS, MSb, MSe, MSLb ^{***} , LH, LHb, HS ^{**} , HSb, HSLb ^{***} or HSe	0.10		0.10

*For sea water exposure, also see Clause 4.1.1.5.

[†]In accordance with CSA A23.2-3B.

[‡]In accordance with CSA A23.2-2B.

[§]Where combinations of supplementary cementing materials and portland or blended hydraulic cements are to be used in the concrete mix design instead of the cementing materials listed, and provided they meet the performance requirements demonstrating equivalent performance against sulphate exposure, they shall be designated as MS equivalent (MSe) or HS equivalent (HSe) in the relevant sulphate exposures (see Clauses 4.1.1.6.2, 4.2.1.1, and 4.2.1.3, and 4.2.1.4).

^{**}Type HS cement shall not be used in reinforced concrete exposed to both chlorides and sulphates, including seawater. See Clause 4.1.1.6.3.

††The requirement for testing at 5 °C does not apply to MS, HS, MSb, HSb, and MSe and HSe combinations made without portland limestone cement.

‡‡ If the increase in expansion between 12 and 18 months exceeds 0.03%, the sulphate expansion at 24 months shall not exceed 0.10% in order for the cement to be deemed to have passed the sulphate resistance requirement.

§§For demonstrating equivalent performance, use the testing frequency in Table 1 of CSA A3004-A1 and see the applicable notes to Table A3 in A3001 with regard to re-establishing compliance if the composition of the cementing materials used to establish compliance changes.

***Where MSLb or HSLb cements are proposed for use, or where MSe or HSe combinations include Portland-limestone cement, they must also contain a minimum of 25% Type F fly ash or 40% slag or 15% metakaolin (meeting Type N pozzolan requirements) or a combination of 5% Type SF silica fume with 25% slag or a combination of 5% Type SF silica fume with 20% Type F fly ash. For some proposed MSLb, HSLb, and MSe or HSe combinations that include Portland-limestone cement, higher SCM replacement levels may be required to meet the A3004-C8 Procedure B expansion limits. Due to the 18-month test period, SCM replacements higher than the identified minimum levels should also be tested. In addition, sulphate resistance testing shall be run on MSLb and HSLb cement and MSe or HSe combinations that include Portland-limestone cement at both 23 °C and 5 °C as specified in the table.

†††If the expansion is greater than 0.05% at 6 months but less than 0.10% at 1 year, the cementing materials combination under test shall be considered to have passed.

7. SOIL CORROSIVITY

The following table, from the Handbook of Corrosion Engineering (Roberge, 1999) indicates the corrosivity rating can be obtained from the soil resistivity, presented on the logs.

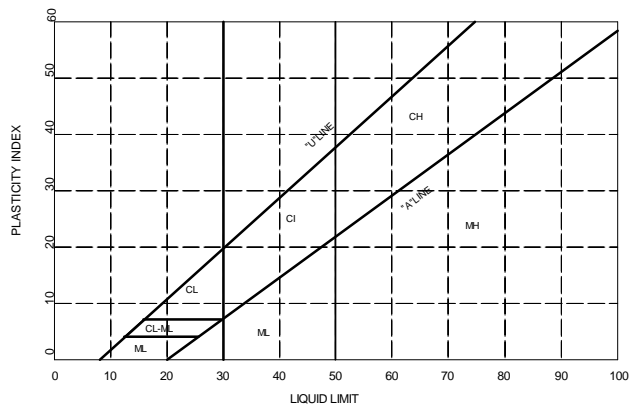
Table 4 Corrosivity Ratings Based on Soil Resistivity

Soil Resistivity (ohm-cm)	Corrosivity Rating
>20,000	Essentially non-corrosive
10,000 – 20,000	Mildly corrosive
5,000 – 10,000	Moderately corrosive
3,000 – 5,000	Corrosive
1,000 – 3,000	Highly corrosive
<1,000	Extremely corrosive

8. GROUNDWATER TABLE

The groundwater table is indicated by the equilibrium level of water in a standpipe installed in a testhole or test pit. This level is generally taken at least 24 hours after installation of the standpipe. The groundwater level is subject to seasonal variations and is usually highest in the spring. The symbol on the logs indicating the groundwater level is an inverted solid triangle (▼).

MAJOR DIVISION		LOG SYMBOLS	UCS	TYPICAL DESCRIPTION	LABORATORY CLASSIFICATION CRITERIA
COARSE GRAINED SOILS	GRAVELS (MORE THAN HALF COARSE GRAINS LARGER THAN 4.75 mm)	CLEAN GRAVELS (LITTLE OR NO FINES)	GW	WELL GRADED GRAVELS, LITTLE OR NO FINES	$C_u - \frac{D_{60}}{D_{10}} > 4$ $C_c - \frac{(D_{30})^2}{D_{10} \times D_{60}} = 1 \text{ to } 3$
		GRAVELS WITH FINES	GP	POORLY GRADED GRAVELS AND GRAVEL-SAND MIXTURES, LITTLE OR NO FINES	NOT MEETING ABOVE REQUIREMENTS
			GM	SILTY GRAVELS, GRAVEL-SAND-SILT MIXTURES	CONTENT OF FINES EXCEEDS 12%
		GC	CLAYEY GRAVELS, GRAVEL-SAND-CLAY MIXTURES	ATTERBERG LIMITS BELOW 'A' LINE W_p LESS THAN 4 ATTERBERG LIMITS ABOVE 'A' LINE W_p MORE THAN 7	
	SANDS (MORE THAN HALF COARSE GRAINS SMALLER THAN 4.75 mm)	CLEAN SANDS (LITTLE R NO FINES)	SW	WELL GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES	$C_u - \frac{D_{60}}{D_{10}} > 6$ $C_c - \frac{(D_{30})^2}{D_{10} \times D_{60}} = 1 \text{ to } 3$
			SP	POORLY GRADED SANDS, LITTLE OR NO FINES	NOT MEETING ABOVE REQUIREMENTS
		SANDS WITH FINES	SM	SILTY SANDS, SAND-SILT MIXTURES	CONTENT OF FINES EXCEEDS 12%
			SC	CLAYEY SANDS, SAND-CLAY MIXTURES	
FINE GRAINED SOILS	SILTS (BELOW 'A' LINE NEGLIGIBLE ORGANIC CONTENT)	$W_L < 50$	ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY SANDS OF SLIGHT PLASTICITY	CLASSIFICATION IS BASED UPON PLASTICITY CHART (SEE BELOW) WHENEVER THE NATURE OF THE FINE CONTENT HAS NOT BEEN DETERMINED, IT IS DESIGNATED BY THE LETTER 'F'. E.G. SF IS A MIXTURE OF SAND WITH SILT OR CLAY
		$W_L > 50$	MH	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SANDY OR SILTY SOILS	
	CLAYS (ABOVE 'A' LINE NEGLIGIBLE ORGANIC CONTENT)	$W_L < 30$	CL	INORGANIC CLAYS OF LOW PLASTICITY, GRAVELLY, SANDY, OR SILTY CLAYS, LEAN CLAYS	
		$30 < W_L < 50$	CI	INORGANIC CLAYS OF MEDIUM PLASTICITY, SILTY CLAYS	
		$W_L > 50$	CH	INORGANIC CLAYS OF HIGH PLASTICITY, FAT CLAYS	
	ORGANIC SILTS & CLAYS (BELOW 'A' LINE)	$W_L < 50$	OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY	
		$W_L > 50$	OH	ORGANIC CLAYS OF HIGH PLASTICITY	
	HIGHLY ORGANIC SOILS			Pt	
BEDROCK			BR	SEE REPORT DESCRIPTION	
FILL			FILL	SEE REPORT DESCRIPTION	



NOTE:
1. BOUNDARY CLASSIFICATION POSSESSING CHARACTERISTICS OF TWO GROUPS ARE GIVEN GROUP SYMBOLS, E.G. GW-GC IS A WELL GRADED GRAVEL MIXTURE WITH CLAY BINDER BETWEEN 5% AND 12%

SOIL COMPONENTS					
FRACTION		SIEVE SIZE (mm)		DEFINING RANGES OF PERCENTAGE BY WEIGHT OF MINOR COMPONENTS	
		PASSING	RETAINED	PERCENT	IDENTIFIER
GRAVEL	COARSE	75	19	50 - 35	AND
	FINE	19	4.75		
SAND	COARSE	4.75	2.00	35 - 20	Y
	MEDIUM	2.00	0.425		
	FINE	0.425	0.080		
SILT (non-plastic) or CLAY (plastic)		0.080		20 - 10	SOME
				10 - 1	TRACE
OVERSIZE MATERIALS					
ROUNDED OR SUB-ROUNDED COBBLES 75 mm TO 200 mm BOULDERS >200 mm			ANGULAR ROCK FRAGMENTS ROCKS > 0.75 m3 IN VOLUME		

MODIFIED UNIFIED SOIL CLASSIFICATION SYSTEM

August 2015

PROJECT: Jefferson East CSR D&C	CLIENT: City of Winnipeg	TESTHOLE NO: TH 11-01
LOCATION: Scotia Street; Between Seven Oaks Boulevard and Jefferson Avenue (UTM: 14 N, 5 532 752.6, 635 379.8)		PROJECT NO.: 60219315
CONTRACTOR: Paddock Drilling Ltd.	METHOD: RM-30, 125 mm SSA	ELEVATION (m): 229.15
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE	

DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	PENETRATION TESTS		UNDRAINED SHEAR STRENGTH		COMMENTS	ELEVATION
					* Becker * ◇ Dynamic Cone ◇ ◆ SPT (Standard Pen Test) ◆ (Blows/300mm) Total Unit Wt (kN/m ³) Plastic MC Liquid	+ Torvane + × QU × □ Lab Vane □ △ Pocket Pen. △ ⊕ Field Vane ⊕ (kPa)				
0		TOPSOIL (FILL)								229
0-1		CLAY (FILL) - some silt to silty, trace to some sand, brown, moist, firm, intermediate to high plasticity								
1-2		CLAY - silty - brown, moist, firm - intermediate plasticity		G1	●					228
2-3		SILT - some clay, trace sand - light brown, moist, soft - low plasticity		G2	●					227
3-4				G3	●					226
4-5		- some sand, trace clay at 4.6 m		G4	●					225
5-6				G5	●					224
6-7		- some clay, some sand, grey below 6.7 m		G6	●					223
7-8				G7	●					222
8-9				G8	●					221
9-10										220
10-11										219
11-12										218
12-13		END OF TEST HOLE AT 12.2 m IN SILT Notes: 1. No sloughing observed. 2. Seepage observed at 6.4 m below surface. 3. Test hole backfilled with auger cuttings and bentonite pellets.								217
13-14										216
14-15										215

LOG OF TEST HOLE JEFFERSON EAST LOGS.GPJ UMA WINN.GDT 1/23/12



LOGGED BY: Stephen Petsche	COMPLETION DEPTH: 12.19 m
REVIEWED BY: Jared Baldwin	COMPLETION DATE: 12/12/11
PROJECT ENGINEER: Eymond Toupin	Page 1 of 1

PROJECT: Jefferson East CSR D&C	CLIENT: City of Winnipeg	TESTHOLE NO: TH 11-02
LOCATION: Scotia Street at Tait Avenue (UTM: 14 N, 5 532 592.2, 635 412.6)		PROJECT NO.: 60219315
CONTRACTOR: Paddock Drilling Ltd.	METHOD: RM-30, 125 mm SSA	ELEVATION (m): 229.06
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK	<input checked="" type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE

DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	PENETRATION TESTS		UNDRAINED SHEAR STRENGTH		COMMENTS	ELEVATION
					SPT (Standard Pen Test) (Blows/300mm)	Total Unit Wt (kN/m ³)	+	×		
0		TOPSOIL (FILL)								
0-1		CLAY (FILL) - some silt to silty, trace to some sand, brown, moist, firm, intermediate to high plasticity								
1-2		CLAY - trace silt - dark brown, moist, firm - high plasticity		G9	●					228
2-3		SILT - clayey, some sand - brown, moist, soft to firm - intermediate plasticity		G10	●					227
3-4		- trace clay, soft below 3.7 m		G11	●					226
4-5				G12	●					225
5-6				G13	●					224
6-7		SAND - silty, some clay - brown, moist, loose		G14	●					223
7-8		SILT - some clay, some sand - grey, moist, soft to firm - intermediate plasticity		G15	●				Gravel: 0.0%, Sand: 61.5%, Silt: 23.1%, Clay: 15.4%	222
8-9				G16	●					221
9-10										220
10-11										219
11-12										218
12-13		- some gravel, wet below 11.9 m								217
13-15		END OF TEST HOLE AT 12.2 m IN SILT Notes: 1. No sloughing observed. 2. Seepage observed at 11.9 m below surface. 3. Test hole backfilled with auger cuttings and bentonite pellets.								216
14-15										215

LOG OF TEST HOLE JEFFERSON EAST LOGS.GPJ UMA WINN.GDT 1/23/12



LOGGED BY: Stephen Petsche	COMPLETION DEPTH: 12.19 m
REVIEWED BY: Jared Baldwin	COMPLETION DATE: 12/12/11
PROJECT ENGINEER: Eymond Toupin	Page 1 of 1

PROJECT: Jefferson East CSR D&C	CLIENT: City of Winnipeg	TESTHOLE NO: TH 11-03
LOCATION: Rupertsland Boulevard; West of Scotia Street (UTM: 14 N, 5 532 453.2, 635 414.9)		PROJECT NO.: 60219315
CONTRACTOR: Paddock Drilling Ltd.	METHOD: RM-30, 125 mm SSA	ELEVATION (m): 227.91
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE	

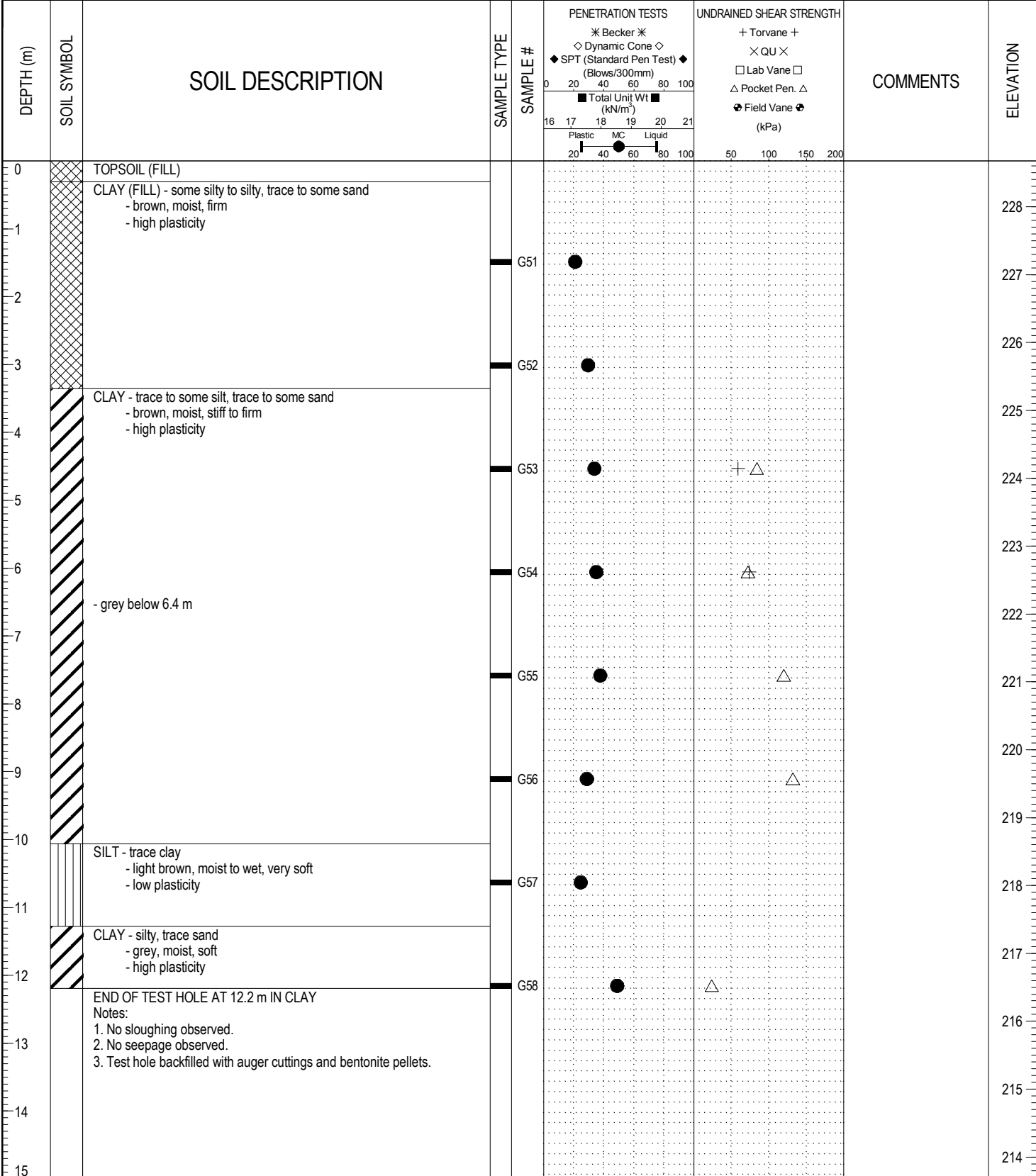
DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	PENETRATION TESTS		UNDRAINED SHEAR STRENGTH		COMMENTS	ELEVATION
					* Becker * ◇ Dynamic Cone ◇ ◆ SPT (Standard Pen Test) ◆ (Blows/300mm) ■ Total Unit Wt (kN/m ³)	+ Torvane + × QU × □ Lab Vane □ △ Pocket Pen. △ ⊕ Field Vane ⊕ (kPa)				
0		TOPSOIL (FILL)								
0-1		CLAY (FILL) - some silt to silty, trace to some sand, brown, moist, firm, intermediate to high plasticity								227
1-2		CLAY - silty, trace to some sand - brown, moist, soft - intermediate plasticity		G17	●					226
2-3				G18	●					225
3-4		SILT - trace clay, trace sand - brown, moist, soft - low plasticity		G19	●					224
4-5				G20	●					223
5-6				G21	●					222
6-7		CLAY - silty, trace to some sand - grey, moist, firm - intermediate to high plasticity		G22	●					221
7-8		SILT - sandy, trace clay - grey, moist, soft - low to intermediate plasticity		G23	●					220
8-9				G24	●					219
9-10		SAND - silty - brown, moist, loose		G25	●					218
10-11		CLAY - silty - grey, moist, soft - intermediate plasticity								217
11-12		- some gravel below 11.6 m								216
12		CLAY - some silt, trace sand, grey, moist, firm, high plasticity								215
12-13		END OF TEST HOLE AT 12.2 m IN CLAY Notes: 1. No sloughing observed. 2. Seepage observed at 5.5 m below surface. 3. Test hole backfilled with auger cuttings and bentonite pellets.								214
13-14										213

LOG OF TEST HOLE JEFFERSON EAST LOGS.GPJ UMA WINN.GDT 1/23/12



LOGGED BY: Stephen Petsche	COMPLETION DEPTH: 12.19 m
REVIEWED BY: Jared Baldwin	COMPLETION DATE: 12/12/11
PROJECT ENGINEER: Eymond Toupin	Page 1 of 1

PROJECT: Jefferson East CSR D&C	CLIENT: City of Winnipeg	TESTHOLE NO: TH 11-04
LOCATION: Mac Steet; Between Rupertsland Boulevard and Tait Avenue (UTM: 14 N, 5 532 609.5, 635 216.2)		PROJECT NO.: 60219315
CONTRACTOR: Paddock Drilling Ltd.	METHOD: RM-30, 125 mm SSA	ELEVATION (m): 228.68
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE	



LOG OF TEST HOLE JEFFERSON EAST LOGS.GPJ UMA WINN.GDT 1/23/12



LOGGED BY: Stephen Petsche	COMPLETION DEPTH: 12.19 m
REVIEWED BY: Jared Baldwin	COMPLETION DATE: 12/13/11
PROJECT ENGINEER: Eymond Toupin	Page 1 of 1

PROJECT: Jefferson East CSR D&C CLIENT: City of Winnipeg TESTHOLE NO: TH 11-05
 LOCATION: Jones Street at Colleen Road (UTM: 14 N, 5 532 613.7, 634 999.3) PROJECT NO.: 60219315
 CONTRACTOR: Paddock Drilling Ltd. METHOD: RM-30, 125 mm SSA ELEVATION (m): 229.10

SAMPLE TYPE GRAB SHELBY TUBE SPLIT SPOON BULK NO RECOVERY CORE

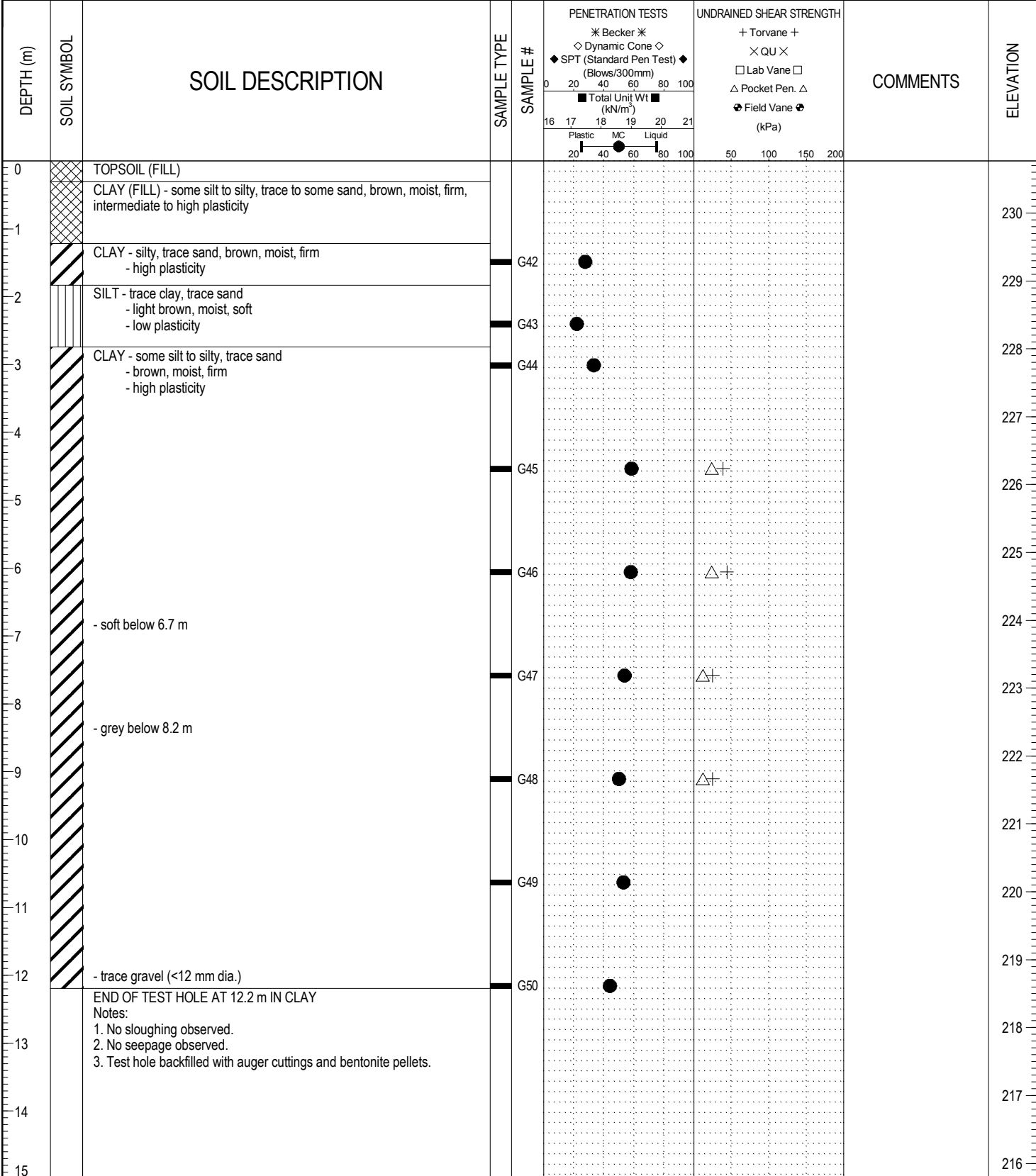
DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	PENETRATION TESTS		UNDRAINED SHEAR STRENGTH		COMMENTS	ELEVATION
					SPT (Standard Pen Test) (Blows/300mm)	Total Unit Wt (kN/m ³)	Lab Vane	Field Vane		
0		TOPSOIL (FILL)								229
0		CLAY (FILL) - some silt to silty, trace to some sand, brown, moist, firm, intermediate to high plasticity								228
1		CLAY - silty, trace sand - dark grey, moist, firm - high plasticity - brown below 1.5 m		G26	●					227
3		- trace silt, firm to soft below 3.7 m		G27	●		+△			226
5				G28	●		+			225
6				G29	●		+			224
7		- soft below 6.7 m		G30	●					223
8				G31	●					222
9				G32	●					221
10				G33	●					220
11										219
12										218
12.2		END OF TEST HOLE AT 12.2 m IN CLAY								217
13		Notes: 1. No sloughing observed. 2. No seepage observed. 3. Test hole backfilled with auger cuttings and bentonite pellets.								216
14										215
15										215

LOG OF TEST HOLE JEFFERSON EAST LOGS.GPJ UMA WINN.GDT 1/23/12



LOGGED BY: Stephen Petsche COMPLETION DEPTH: 12.19 m
 REVIEWED BY: Jared Baldwin COMPLETION DATE: 12/12/11
 PROJECT ENGINEER: Eymond Toupin Page 1 of 1

PROJECT: Jefferson East CSR D&C	CLIENT: City of Winnipeg	TESTHOLE NO: TH 11-06
LOCATION: Seven Oaks Boulevard; Between Jones Street and Scotia Street (UTM: 14 N, 5 532 779.6, 635 215.3)		PROJECT NO.: 60219315
CONTRACTOR: Paddock Drilling Ltd.	METHOD: RM-30, 125 mm SSA	ELEVATION (m): 230.77
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE	

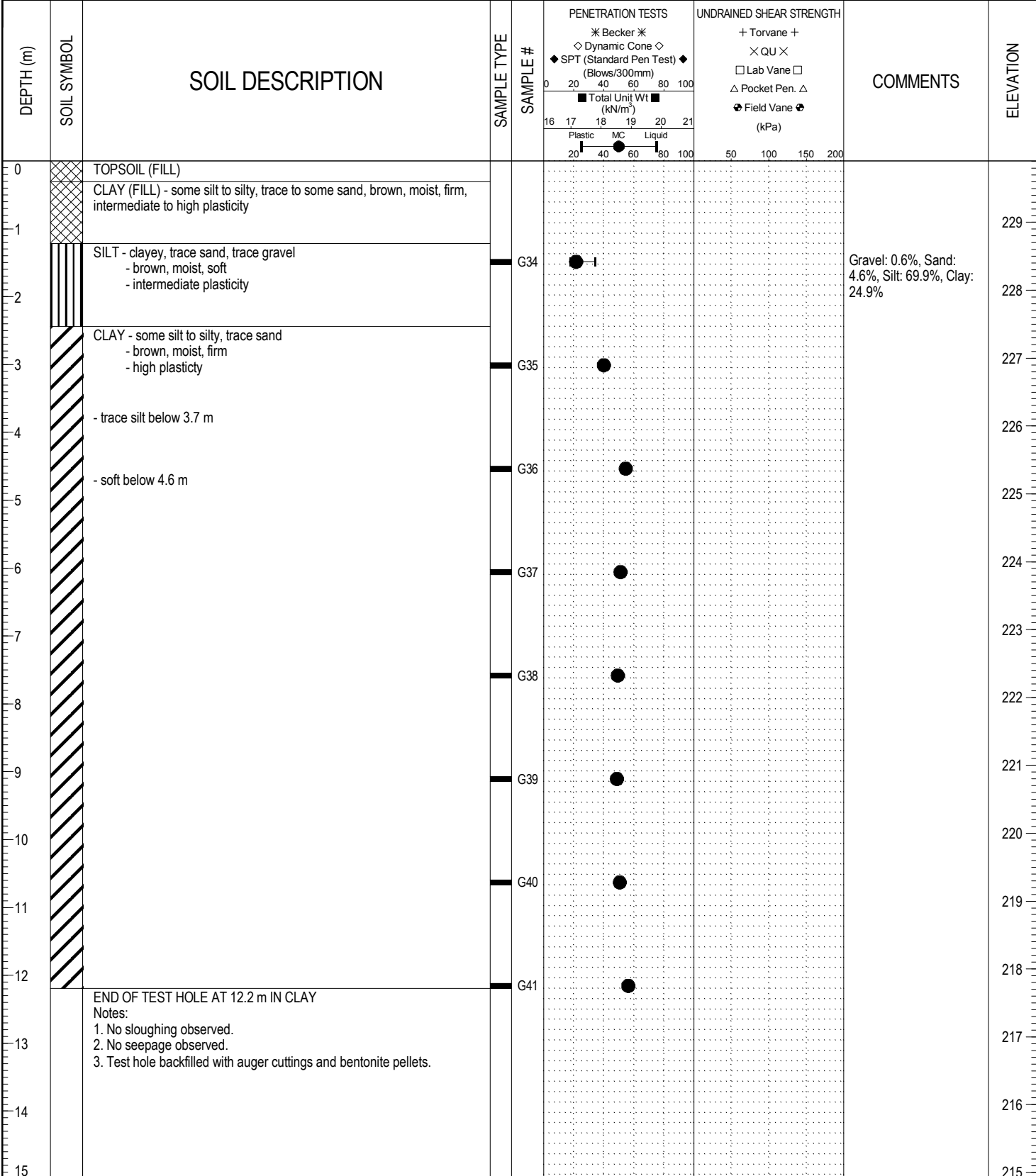


LOG OF TEST HOLE JEFFERSON EAST LOGS.GPJ UMA WINN.GDT 1/23/12



LOGGED BY: Stephen Petsche	COMPLETION DEPTH: 12.19 m
REVIEWED BY: Jared Baldwin	COMPLETION DATE: 12/13/11
PROJECT ENGINEER: Eymond Toupin	Page 1 of 1

PROJECT: Jefferson East CSR D&C	CLIENT: City of Winnipeg	TESTHOLE NO: TH 11-07
LOCATION: Seven Oaks Boulevard; East of Main Street (UTM: 14 N, 5 532 903.1, 634 952.4)		PROJECT NO.: 60219315
CONTRACTOR: Paddock Drilling Ltd.	METHOD: RM-30, 125 mm SSA	ELEVATION (m): 229.91
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE	



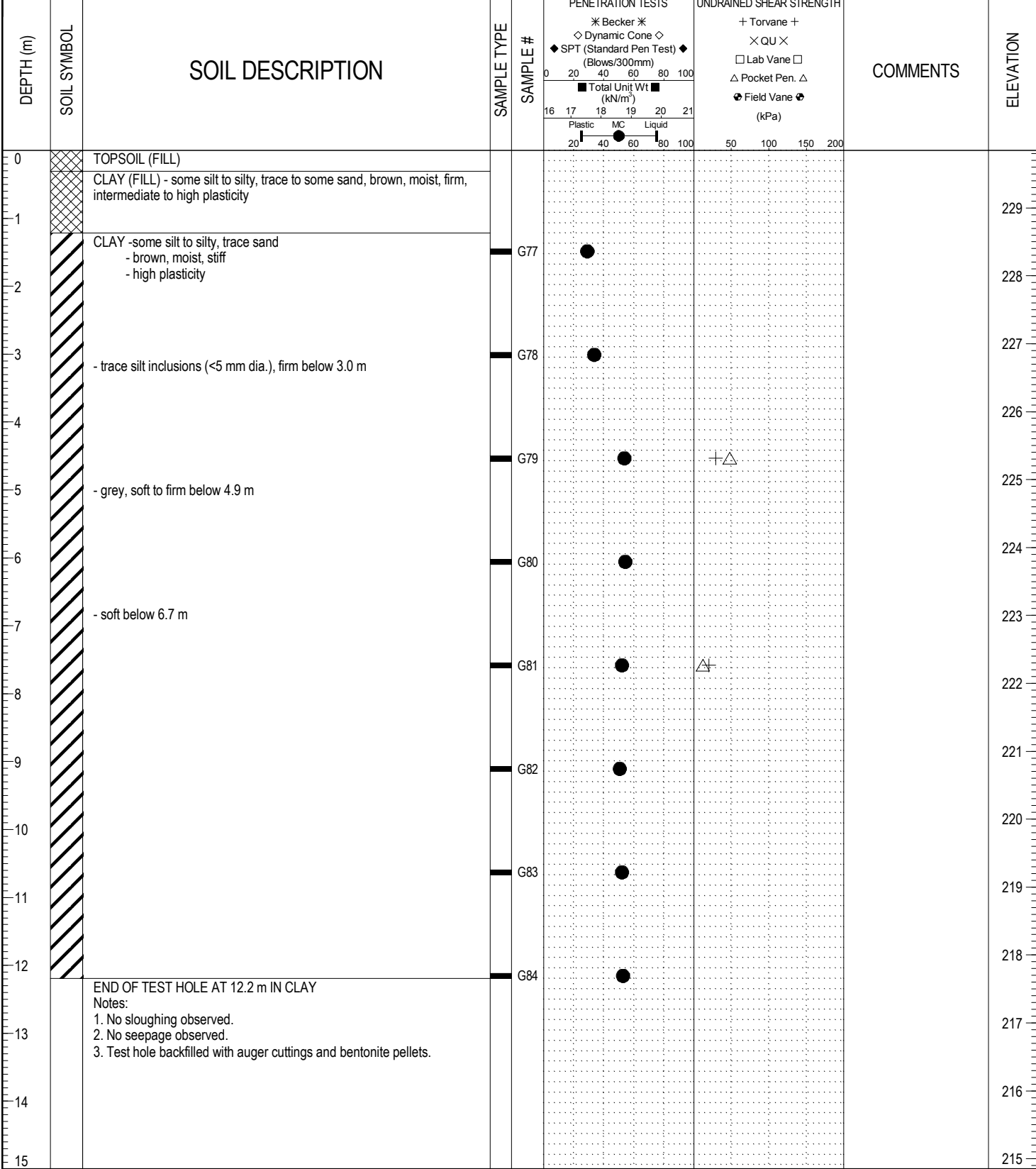
LOG OF TEST HOLE JEFFERSON EAST LOGS.GPJ UMA WINN.GDT 1/23/12



LOGGED BY: Stephen Petsche	COMPLETION DEPTH: 12.19 m
REVIEWED BY: Jared Baldwin	COMPLETION DATE: 12/12/11
PROJECT ENGINEER: Eymond Toupin	Page 1 of 1

PROJECT: Jefferson East CSR D&C CLIENT: City of Winnipeg TESTHOLE NO: TH 11-08
 LOCATION: Jones Street at St. Anthony Avenue (UTM: 14 N, 5 533 005.7, 635 169.6) PROJECT NO.: 60219315
 CONTRACTOR: Paddock Drilling Ltd. METHOD: RM-30, 125 mm SSA ELEVATION (m): 229.85

SAMPLE TYPE GRAB SHELBY TUBE SPLIT SPOON BULK NO RECOVERY CORE

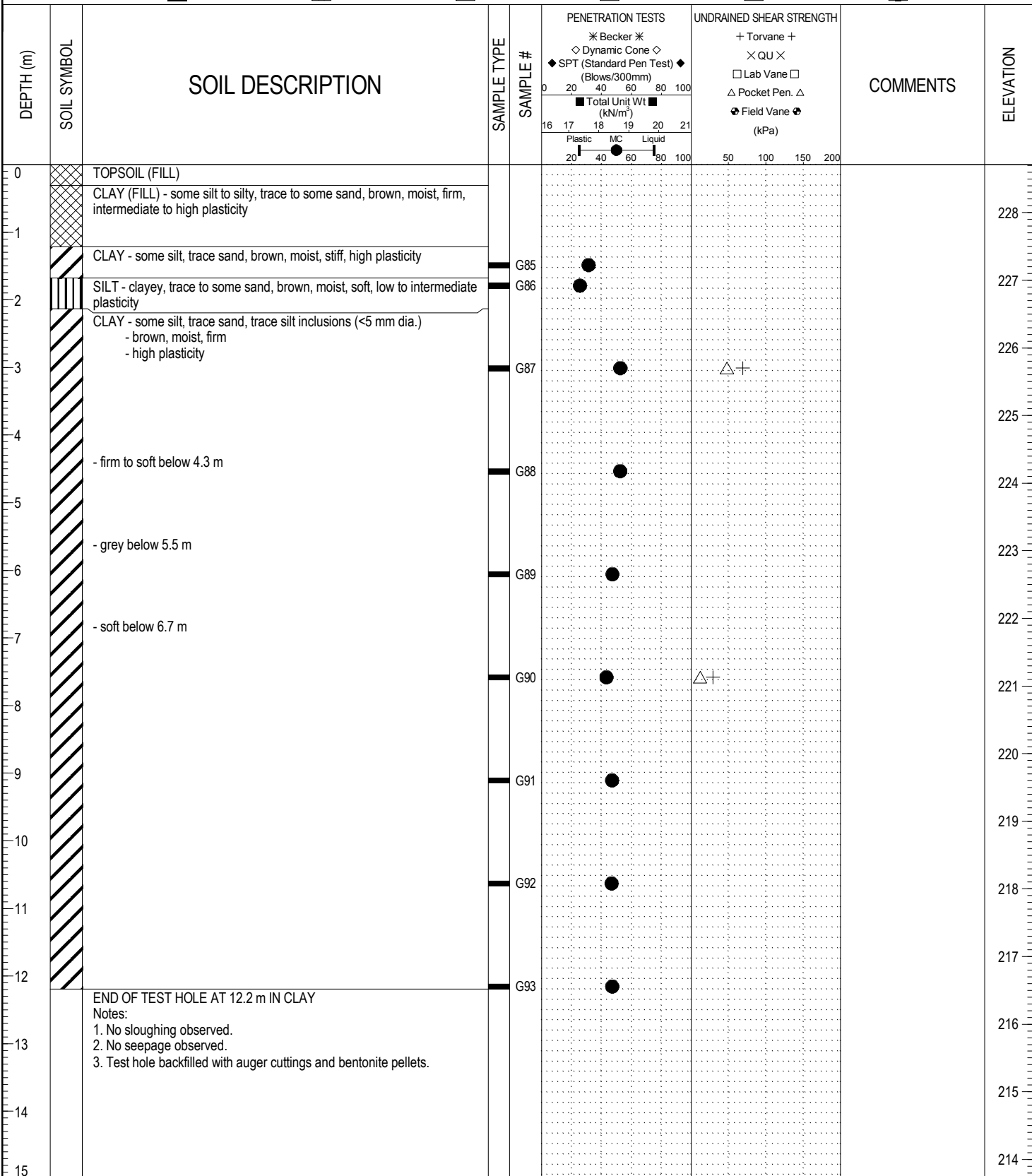


LOG OF TEST HOLE JEFFERSON EAST LOGS.GPJ UMA WINN.GDT 1/23/12



LOGGED BY: Stephen Petsche COMPLETION DEPTH: 12.19 m
 REVIEWED BY: Jared Baldwin COMPLETION DATE: 12/13/11
 PROJECT ENGINEER: Eymond Toupin Page 1 of 1

PROJECT: Jefferson East CSR D&C CLIENT: City of Winnipeg TESTHOLE NO: TH 11-09
 LOCATION: Jones Street at Hartford Avenue (UTM: 14 N, 5 533 159.7, 635 238.0) PROJECT NO.: 60219315
 CONTRACTOR: Paddock Drilling Ltd. METHOD: RM-30, 125 mm SSA ELEVATION (m): 228.71
 SAMPLE TYPE GRAB SHELBY TUBE SPLIT SPOON BULK NO RECOVERY CORE



LOG OF TEST HOLE JEFFERSON EAST LOGS.GPJ UMA WINN.GDT 1/23/12



LOGGED BY: Stephen Petsche COMPLETION DEPTH: 12.19 m
 REVIEWED BY: Jared Baldwin COMPLETION DATE: 12/13/11
 PROJECT ENGINEER: Eymond Toupin Page 1 of 1

PROJECT: Jefferson East CSR D&C	CLIENT: City of Winnipeg	TESTHOLE NO: TH 11-10
LOCATION: Scotia Street at Belmont Avenue (UTM: 14 N, 5 533 204.8, 635 369.7)		PROJECT NO.: 60219315
CONTRACTOR: Paddock Drilling Ltd.	METHOD: RM-30, 125 mm SSA	ELEVATION (m): 230.67
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK	<input checked="" type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE

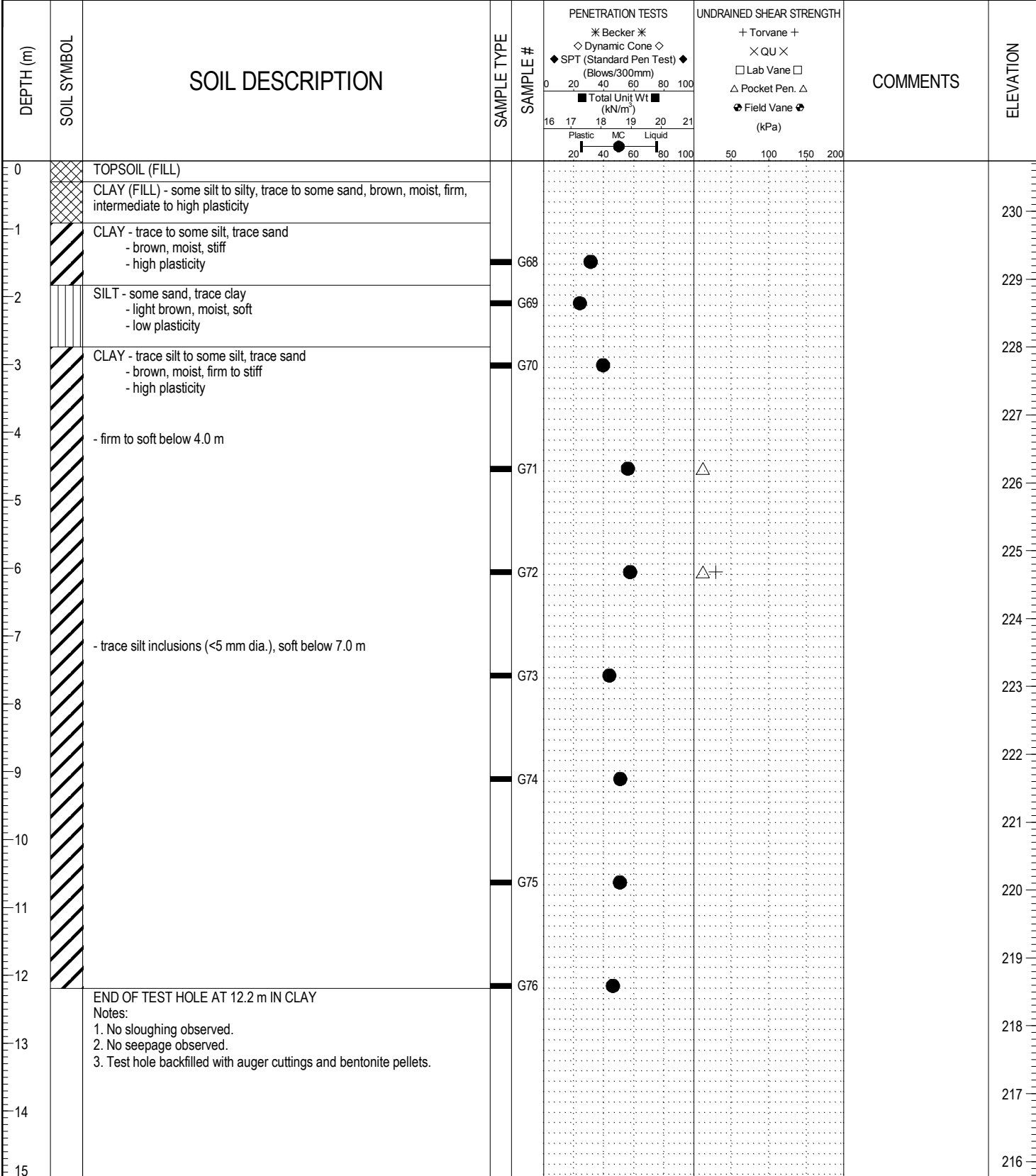
DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	PENETRATION TESTS		UNDRAINED SHEAR STRENGTH		COMMENTS	ELEVATION
					* Becker * ◇ Dynamic Cone ◇ ◆ SPT (Standard Pen Test) ◆ (Blows/300mm) ■ Total Unit Wt (kN/m ³)	+ Torvane + × QU × □ Lab Vane □ △ Pocket Pen. △ ⊕ Field Vane ⊕ (kPa)				
0		TOPSOIL (FILL)								
0		CLAY (FILL) - some silt to silty, trace to some sand, brown, moist, firm, intermediate to high plasticity								230
1		CLAY - some silt to silty, trace sand, brown, moist, firm - high plasticity		G94	●					229
2		SILT - trace to some clay, trace sand - light brown, moist, soft - low plasticity		G95	●					228
3		CLAY - trace to some silt, trace sand - brown, moist, firm - high plasticity		G96	●					227
4		- grey, soft below 4.0 m		G97	●		△			226
6				G98	●		△			225
8				G99	●		△			223
10				G100	●					221
11				G101	●					220
12				G102	●					219
12.19		END OF TEST HOLE AT 12.2 m IN CLAY								218
13		Notes: 1. No sloughing observed. 2. No seepage observed. 3. Test hole backfilled with auger cuttings and bentonite pellets.								217
14										216
15										216

LOG OF TEST HOLE JEFFERSON EAST LOGS.GPJ UMA WINN.GDT 1/23/12



LOGGED BY: Stephen Petsche	COMPLETION DEPTH: 12.19 m
REVIEWED BY: Jared Baldwin	COMPLETION DATE: 12/14/11
PROJECT ENGINEER: Eymond Toupin	Page 1 of 1

PROJECT: Jefferson East CSR D&C	CLIENT: City of Winnipeg	TESTHOLE NO: TH 11-11
LOCATION: Semple Avenue; East of Main Street (UTM: 14 N, 5 533 432.1, 635 210.0)		PROJECT NO.: 60219315
CONTRACTOR: Paddock Drilling Ltd.	METHOD: RM-30, 125 mm SSA	ELEVATION (m): 230.74
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK	<input checked="" type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE

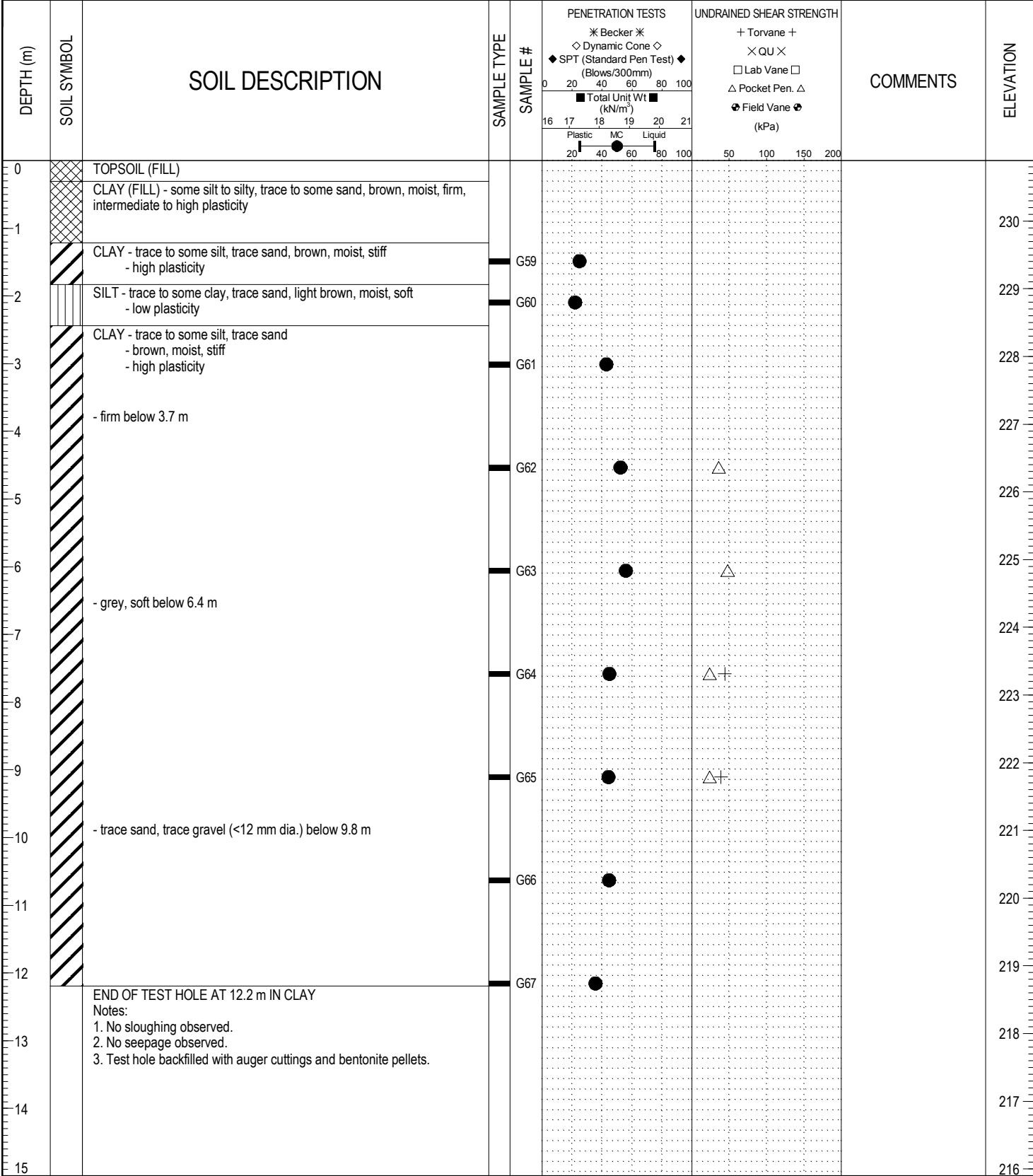


LOG OF TEST HOLE JEFFERSON EAST LOGS.GPJ UMA WINN.GDT 1/23/12



LOGGED BY: Stephen Petsche	COMPLETION DEPTH: 12.19 m
REVIEWED BY: Jared Baldwin	COMPLETION DATE: 12/13/11
PROJECT ENGINEER: Eymond Toupin	Page 1 of 1

PROJECT: Jefferson East CSR D&C	CLIENT: City of Winnipeg	TESTHOLE NO: TH 11-12
LOCATION: Scotia Street at Semple Avenue (UTM: 14 N, 5 533 321.9, 635 426.4)		PROJECT NO.: 60219315
CONTRACTOR: Paddock Drilling Ltd.	METHOD: RM-30, 125 mm SSA	ELEVATION (m): 230.89
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK	<input checked="" type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE

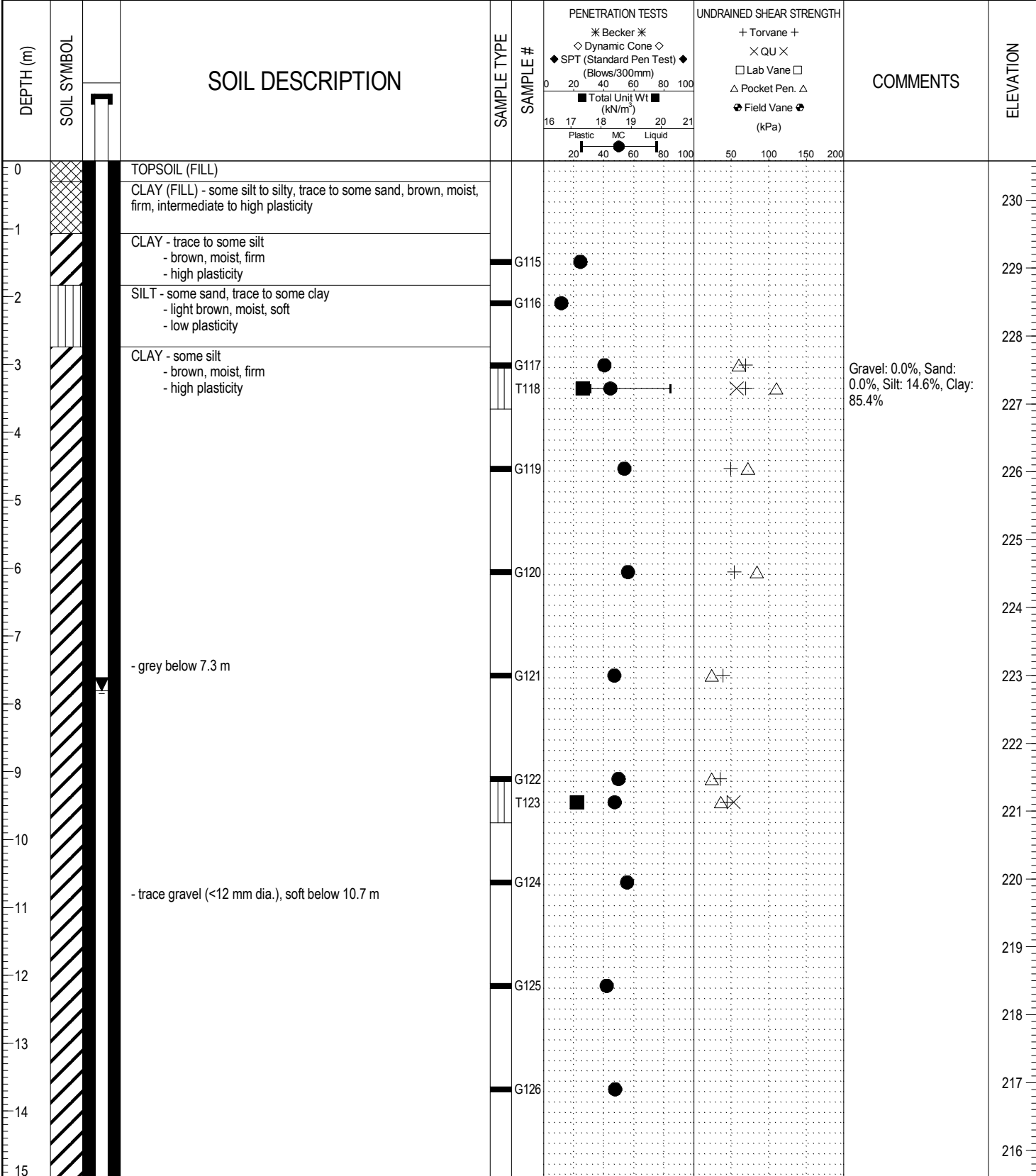


LOG OF TEST HOLE JEFFERSON EAST LOGS.GPJ UMA WINN.GDT 1/23/12



LOGGED BY: Stephen Petsche	COMPLETION DEPTH: 12.19 m
REVIEWED BY: Jared Baldwin	COMPLETION DATE: 12/13/11
PROJECT ENGINEER: Eymond Toupin	Page 1 of 1

PROJECT: Jefferson East CSR D&C		CLIENT: City of Winnipeg		TESTHOLE NO: SP 11-13		
LOCATION: Upper Outfall Area; East of Scotia Street (UTM: 14 N, 5 533 354.1, 635 496.6)				PROJECT NO.: 60219315		
CONTRACTOR: Paddock Drilling Ltd.		METHOD: RM-30, 125 mm SSA		ELEVATION (m): 230.58		
SAMPLE TYPE	GRAB	SHELBY TUBE	SPLIT SPOON	BULK	NO RECOVERY	CORE
BACKFILL TYPE	BENTONITE	GRAVEL	SLOUGH	GROUT	CUTTINGS	SAND

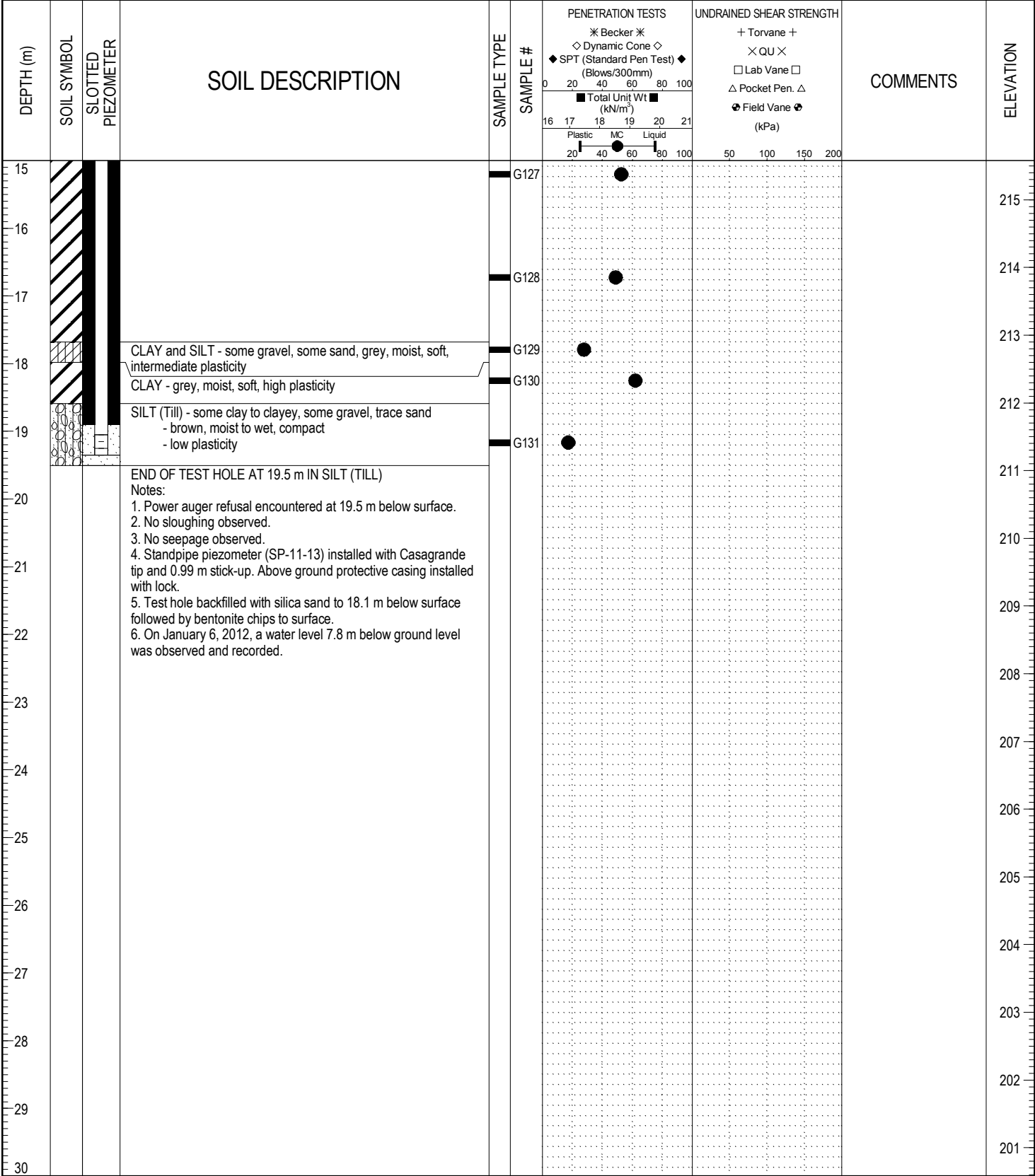


LOG OF TEST HOLE JEFFERSON EAST LOGS.GPJ UMA WINN.GDT 1/23/12



LOGGED BY: Stephen Petsche	COMPLETION DEPTH: 19.51 m
REVIEWED BY: Jared Baldwin	COMPLETION DATE: 12/14/11
PROJECT ENGINEER: Eymond Toupin	Page 1 of 2

PROJECT: Jefferson East CSR D&C		CLIENT: City of Winnipeg		TESTHOLE NO: SP 11-13		
LOCATION: Upper Outfall Area; East of Scotia Street (UTM: 14 N, 5 533 354.1, 635 496.6)				PROJECT NO.: 60219315		
CONTRACTOR: Paddock Drilling Ltd.			METHOD: RM-30, 125 mm SSA		ELEVATION (m): 230.58	
SAMPLE TYPE	GRAB	SHELBY TUBE	SPLIT SPOON	BULK	NO RECOVERY	CORE
BACKFILL TYPE	BENTONITE	GRAVEL	SLOUGH	GROUT	CUTTINGS	SAND

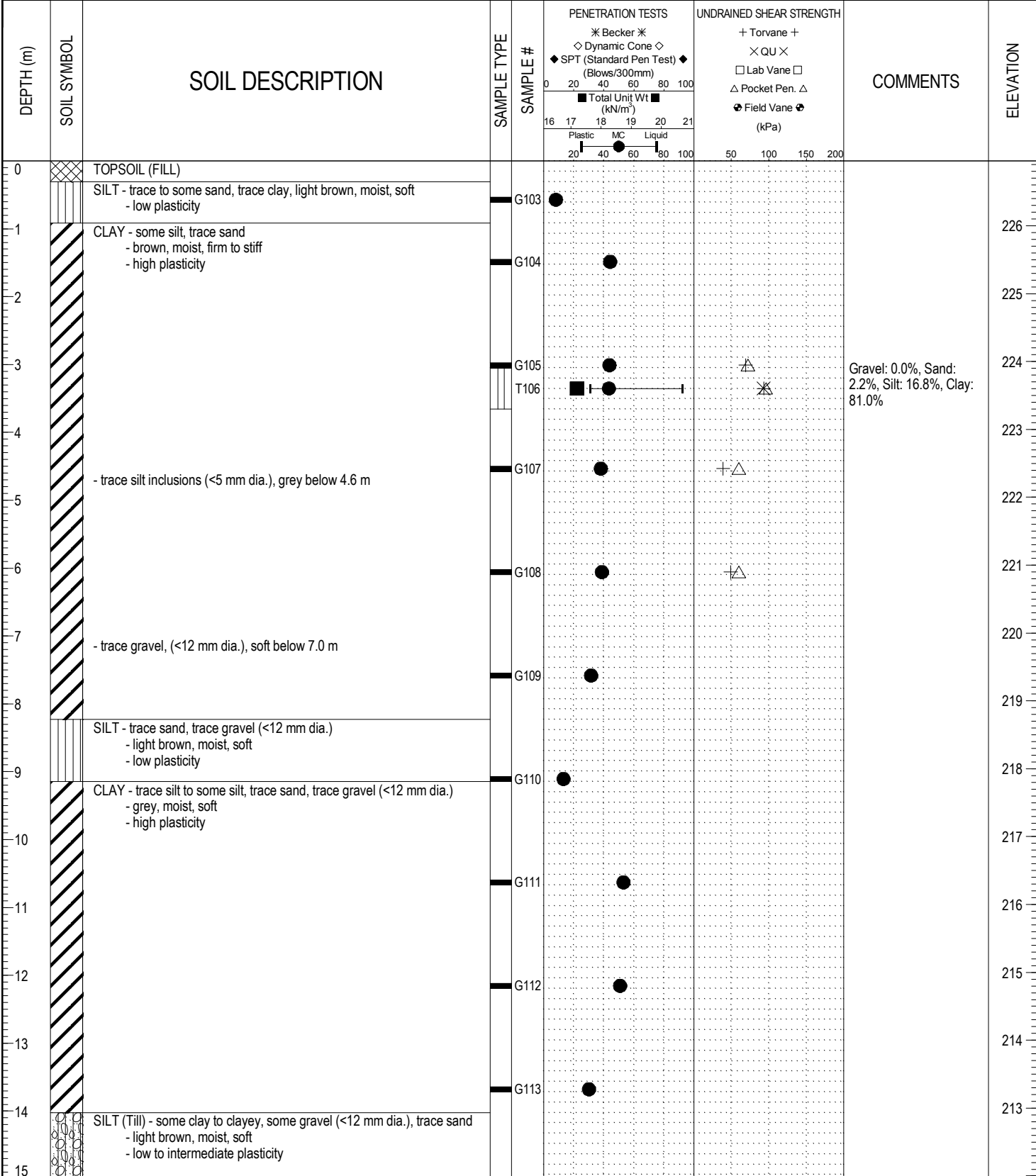


LOG OF TEST HOLE JEFFERSON EAST LOGS.GPJ UMA WINNI.GDT 1/23/12



LOGGED BY: Stephen Petsche	COMPLETION DEPTH: 19.51 m
REVIEWED BY: Jared Baldwin	COMPLETION DATE: 12/14/11
PROJECT ENGINEER: Eymond Toupin	Page 2 of 2

PROJECT: Jefferson East CSR D&C	CLIENT: City of Winnipeg	TESTHOLE NO: TH 11-14
LOCATION: Lower Outfall Area; East of Scotia Street (UTM: 14 N, 5 533 345.7, 635 513.5)		PROJECT NO.: 60219315
CONTRACTOR: Paddock Drilling Ltd.	METHOD: RM-30, 125 mm SSA	ELEVATION (m): 226.96
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE	



LOG OF TEST HOLE JEFFERSON EAST LOGS.GPJ UMA WINNI.GDT 1/23/12



LOGGED BY: Stephen Petsche	COMPLETION DEPTH: 15.54 m
REVIEWED BY: Jared Baldwin	COMPLETION DATE: 12/14/11
PROJECT ENGINEER: Eymond Toupin	Page 1 of 2

PROJECT: Jefferson East CSR D&C	CLIENT: City of Winnipeg	TESTHOLE NO: TH 11-14
LOCATION: Lower Outfall Area; East of Scotia Street (UTM: 14 N, 5 533 345.7, 635 513.5)	PROJECT NO.: 60219315	
CONTRACTOR: Paddock Drilling Ltd.	METHOD: RM-30, 125 mm SSA	ELEVATION (m): 226.96
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input checked="" type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE	

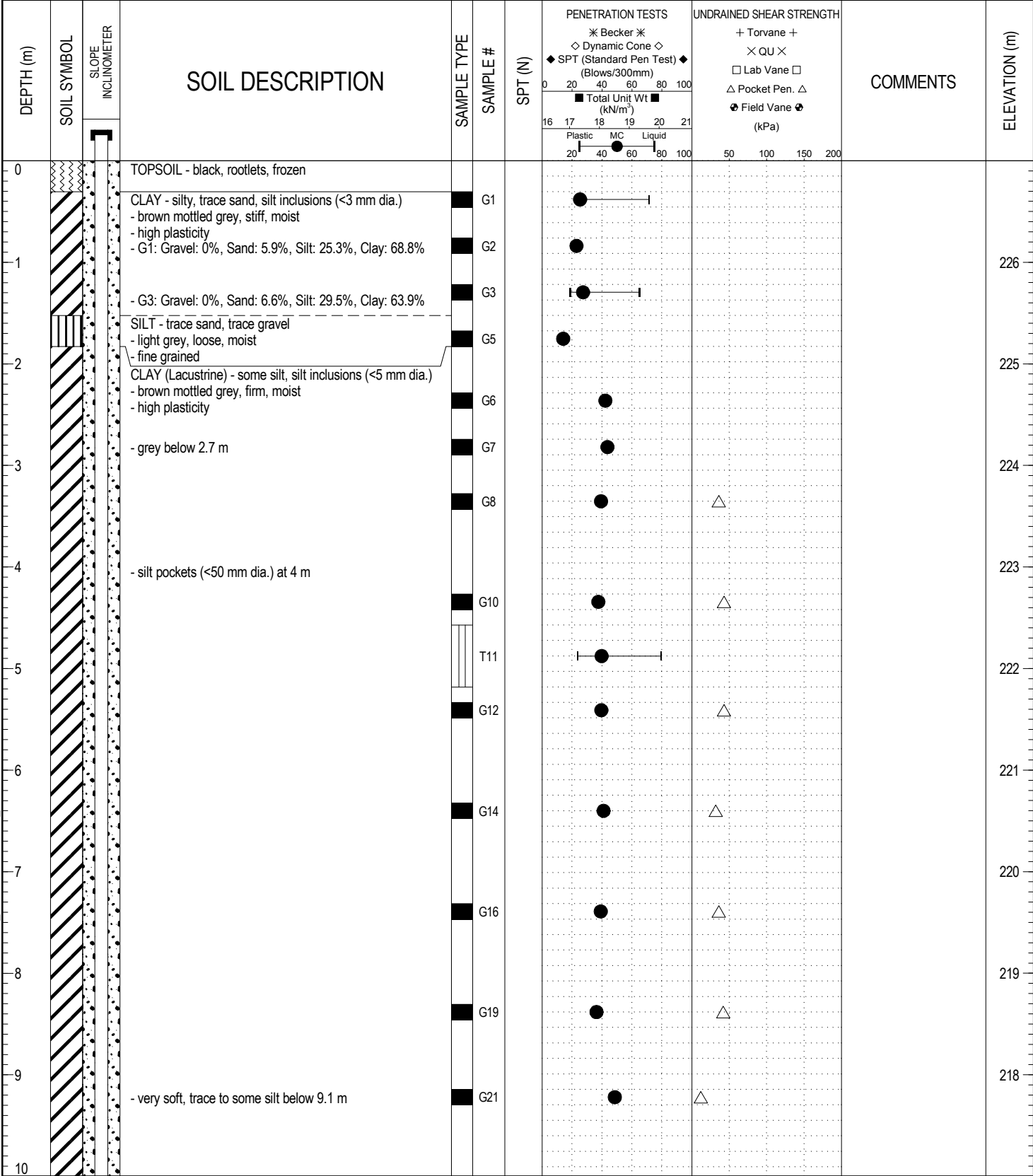
DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	PENETRATION TESTS	UNDRAINED SHEAR STRENGTH	COMMENTS	ELEVATION	
15	●	END OF TEST HOLE AT 15.5 m IN SILT (TILL) Notes: 1. Power auger refusal encountered at 15.5 m below surface. 2. No sloughing observed. 3. No seepage observed. 4. Test hole backfilled with auger cuttings and bentonite pellets.		G114	* Becker * ◇ Dynamic Cone ◇ ◆ SPT (Standard Pen Test) ◆ (Blows/300mm) 0 20 40 60 80 100 ■ Total Unit Wt ■ (kN/m ³) 16 17 18 19 20 21 Plastic MC Liquid 20 40 60 80 100	+ Torvane + × QU × <input type="checkbox"/> Lab Vane <input type="checkbox"/> △ Pocket Pen. △ ⊗ Field Vane ⊗ (kPa) 50 100 150 200			211
16								210	
17								209	
18								208	
19								207	
20								206	
21								205	
22								204	
23								203	
24								202	
25								201	
26								200	
27								199	
28								198	
29									
30									

LOG OF TEST HOLE JEFFERSON EAST LOGS.GPJ UMA WINN.GDT 1/23/12



LOGGED BY: Stephen Petsche	COMPLETION DEPTH: 15.54 m
REVIEWED BY: Jared Baldwin	COMPLETION DATE: 12/14/11
PROJECT ENGINEER: Eymond Toupin	Page 2 of 2

PROJECT: Jefferson East CSR - Sample Outfall		CLIENT: City of Winnipeg		TESTHOLE NO: S115-01		
LOCATION: North of the Proposed Outfall Pipe, 4 m West of Lower Slope				PROJECT NO.: 60219315		
CONTRACTOR: Maple Leaf Drilling Ltd.		METHOD: MP5 Track Mounted-125mm SSA/HQ Barrel		ELEVATION (m): 227.00		
SAMPLE TYPE	GRAB	SHELBY TUBE	SPLIT SPOON	BULK	NO RECOVERY	CORE
BACKFILL TYPE	BENTONITE	GRAVEL	SLOUGH	GROUT	CUTTINGS	SAND

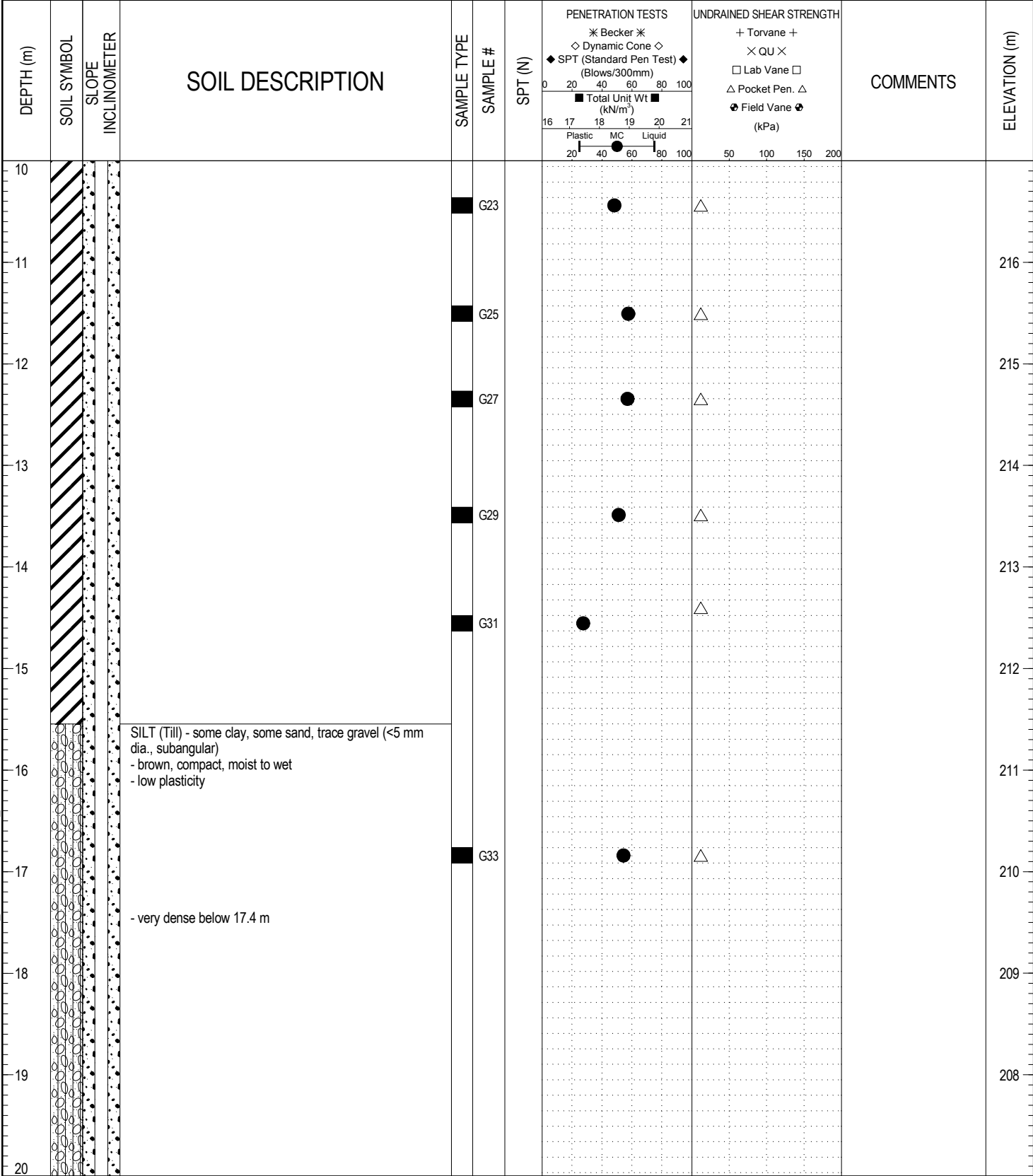


LOG OF TEST HOLE JEFFERSON CSR - SAMPLE OUTFALL TEST HOLE LOGS_REV 02.GPJ UMA WINN.GDT 10/26/15



LOGGED BY: Mustafa Alkiki	COMPLETION DEPTH: 22.61 m
REVIEWED BY: Zeyad Shukri	COMPLETION DATE: 2/24/15
PROJECT ENGINEER: Eymond Toupin	Page 1 of 3

PROJECT: Jefferson East CSR - Sample Outfall	CLIENT: City of Winnipeg	TESTHOLE NO: S115-01
LOCATION: North of the Proposed Outfall Pipe, 4 m West of Lower Slope		PROJECT NO.: 60219315
CONTRACTOR: Maple Leaf Drilling Ltd.	METHOD: MP5 Track Mounted-125mm SSA/HQ Barrel	ELEVATION (m): 227.00
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE	
BACKFILL TYPE	<input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> GRAVEL <input type="checkbox"/> SLOUGH <input type="checkbox"/> GROUT <input type="checkbox"/> CUTTINGS <input type="checkbox"/> SAND	



LOG OF TEST HOLE JEFFERSON CSR - SAMPLE OUTFALL TEST HOLE LOGS - REV 02.GPJ UMA WINN.GDT 10/26/15



LOGGED BY: Mustafa Alkiki	COMPLETION DEPTH: 22.61 m
REVIEWED BY: Zeyad Shukri	COMPLETION DATE: 2/24/15
PROJECT ENGINEER: Eymond Toupin	Page 2 of 3

PROJECT: Jefferson East CSR - Sample Outfall	CLIENT: City of Winnipeg	TESTHOLE NO: SI15-01
LOCATION: North of the Proposed Outfall Pipe, 4 m West of Lower Slope		PROJECT NO.: 60219315
CONTRACTOR: Maple Leaf Drilling Ltd.	METHOD: MP5 Track Mounted-125mm SSA/HQ Barrel	ELEVATION (m): 227.00
SAMPLE TYPE	<input type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE	
BACKFILL TYPE	<input type="checkbox"/> BENTONITE <input type="checkbox"/> GRAVEL <input type="checkbox"/> SLOUGH <input type="checkbox"/> GROUT <input type="checkbox"/> CUTTINGS <input type="checkbox"/> SAND	

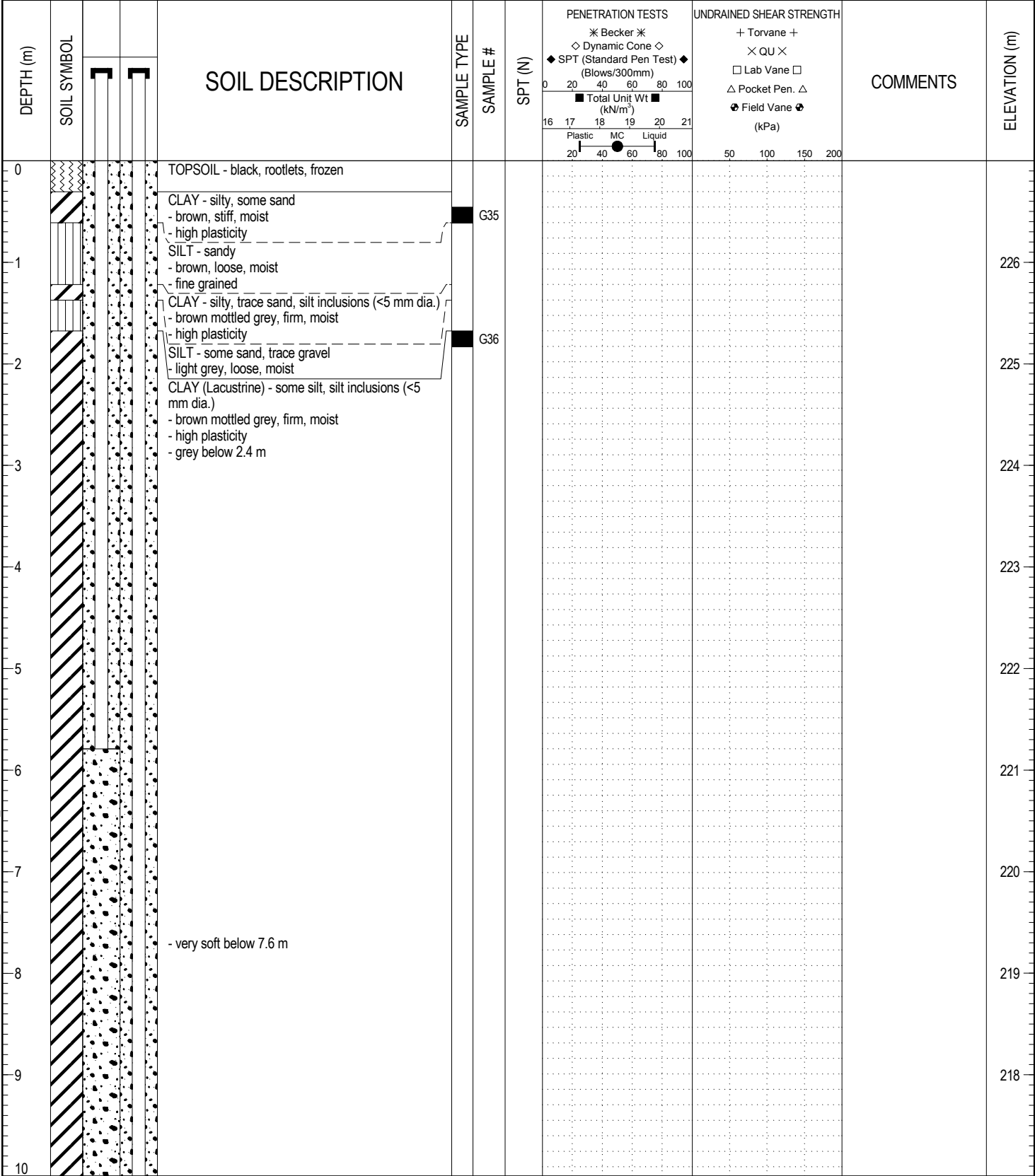
DEPTH (m)	SOIL SYMBOL	SLOPE INCLINOMETER	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	SPT (N)	PENETRATION TESTS		UNDRAINED SHEAR STRENGTH		COMMENTS	ELEVATION (m)
							* Becker * ◇ Dynamic Cone ◇ ◆ SPT (Standard Pen Test) ◆ (Blows/300mm) Total Unit Wt (kN/m ³)	+ Torvane + × QU × □ Lab Vane □ △ Pocket Pen. △ ⊕ Field Vane ⊕ (kPa)				
20												206
21												206
22			BEDROCK (Limestone)									205
23			END OF TEST HOLE AT 22.61 m IN BEDROCK (LIMESTONE) Notes: 1. Power auger refusal at 17.4 m below grade. 2. Switched to HQ barrel below 17.4 m. 3. No sloughing observed during drilling. 4. Seepage observed at 9 and 12.8 m below grade. 5. Installed slope inclinometer (SI15-01) to 22.6 m. 6. Test hole backfilled with cement/grout to ground surface.									204
24												203
25												202
26												201
27												200
28												199
29												198
30												198

LOG OF TEST HOLE - JEFFERSON CSR - SAMPLE OUTFALL - TEST HOLE LOGS - REV 02.GPJ UMA WINN.GDT 10/26/15



LOGGED BY: Mustafa Alkiki	COMPLETION DEPTH: 22.61 m
REVIEWED BY: Zeyad Shukri	COMPLETION DATE: 2/24/15
PROJECT ENGINEER: Eymond Toupin	Page 3 of 3

PROJECT: Jefferson East CSR - Sample Outfall		CLIENT: City of Winnipeg		TESTHOLE NO: VW15-02		
LOCATION: South of the Proposed Outfall Pipe, 15 m West of Lower Slope				PROJECT NO.: 60219315		
CONTRACTOR: Maple Leaf Drilling Ltd.		METHOD: MP5 Track Mounted-125mm SSA/HQ Barrel		ELEVATION (m): 227.00		
SAMPLE TYPE	GRAB	SHELBY TUBE	SPLIT SPOON	BULK	NO RECOVERY	CORE
BACKFILL TYPE	BENTONITE	GRAVEL	SLOUGH	GROUT	CUTTINGS	SAND



LOG OF TEST HOLE - JEFFERSON CSR - SAMPLE OUTFALL - TEST HOLE LOGS - REV 02.GPJ UMA WINN.GDT 10/26/15



LOGGED BY: Mustafa Alkiki	COMPLETION DEPTH: 12.50 m
REVIEWED BY: Zeyad Shukri	COMPLETION DATE: 2/24/15
PROJECT ENGINEER: Eymond Toupin	Page 1 of 2

PROJECT: Jefferson East CSR - Sample Outfall		CLIENT: City of Winnipeg		TESTHOLE NO: VW15-02	
LOCATION: South of the Proposed Outfall Pipe, 15 m West of Lower Slope				PROJECT NO.: 60219315	
CONTRACTOR: Maple Leaf Drilling Ltd.		METHOD: MP5 Track Mounted-125mm SSA/HQ Barrel		ELEVATION (m): 227.00	
SAMPLE TYPE		GRAB	SHELBY TUBE	SPLIT SPOON	BULK
BACKFILL TYPE		BENTONITE	GRAVEL	SLOUGH	GROUT
				NO RECOVERY	CORE
				CUTTINGS	SAND

DEPTH (m)	SOIL SYMBOL	VW PIEZOMETER	VW PIEZOMETER	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	SPT (N)	PENETRATION TESTS		UNDRAINED SHEAR STRENGTH		COMMENTS	ELEVATION (m)
								* Becker * ◇ Dynamic Cone ◇ ◆ SPT (Standard Pen Test) ◆ (Blows/300mm) Total Unit Wt (kN/m ³)	+ Torvane + × QU × □ Lab Vane □ △ Pocket Pen. △ ⊕ Field Vane ⊕ (kPa)				
10													
11													216
12													215
13				END OF TEST HOLE AT 12.50 m IN CLAY (Lacustrine)									214
14				Notes: 1. No sloughing observed during drilling. 2. Seepage observed ay 9.6 m below grade. 3. Squeezing below 8.3 m 4. Installed VW15-02 and VW15-03 in test hole at 5.8 and 12 m, respectively. 5. Test hole backfilled with cement/grout (full depth).									213
15													212
16													211
17													210
18													209
19													208
20													

LOG OF TEST HOLE - JEFFERSON CSR - SAMPLE OUTFALL - TEST HOLE LOGS - REV 02.GPJ UMA WINN.GDT 10/26/15



LOGGED BY: Mustafa Alkiki	COMPLETION DEPTH: 12.50 m
REVIEWED BY: Zeyad Shukri	COMPLETION DATE: 2/24/15
PROJECT ENGINEER: Eymond Toupin	Page 2 of 2

Appendix **C**

AECOM (June 2019) Geotechnical Investigation Test Hole Logs

- AECOM (June 2019) Geotechnical Investigation Test Hole Logs

AECOM Canada Ltd.

GENERAL STATEMENT

NORMAL VARIABILITY OF SUBSURFACE CONDITIONS

The scope of the investigation presented herein is limited to an investigation of the subsurface conditions as to suitability for the proposed project. This report has been prepared to aid in the evaluation of the site and to assist the engineer in the design of the facilities. Our description of the project represents our understanding of the significant aspects of the project relevant to the design and construction of earth work, foundations and similar. In the event of any changes in the basic design or location of the structures as outlined in this report or plan, we should be given the opportunity to review the changes and to modify or reaffirm in writing the conclusions and recommendations of this report.

The analysis and recommendations presented in this report are based on the data obtained from the borings and test pit excavations made at the locations indicated on the site plans and from other information discussed herein. This report is based on the assumption that the subsurface conditions everywhere are not significantly different from those disclosed by the borings and excavations. However, variations in soil conditions may exist between the excavations and, also, general groundwater levels and conditions may fluctuate from time to time. The nature and extent of the variations may not become evident until construction. If subsurface conditions differ from those encountered in the exploratory borings and excavations, are observed or encountered during construction, or appear to be present beneath or beyond excavations, we should be advised at once so that we can observe and review these conditions and reconsider our recommendations where necessary.

Since it is possible for conditions to vary from those assumed in the analysis and upon which our conclusions and recommendations are based, a contingency fund should be included in the construction budget to allow for the possibility of variations which may result in modification of the design and construction procedures.

In order to observe compliance with the design concepts, specifications or recommendations and to allow design changes in the event that subsurface conditions differ from those anticipated, we recommend that all construction operations dealing with earth work and the foundations be observed by an experienced soils engineer. We can be retained to provide these services for you during construction. In addition, we can be retained to review the plans and specifications that have been prepared to check for substantial conformance with the conclusions and recommendations contained in our report.

EXPLANATION OF FIELD & LABORATORY TEST DATA

The field and laboratory test results, as shown for each hole, are described below.

1. NATURAL MOISTURE CONTENT

The relationship between the natural moisture content and depth is significant in determining the subsurface moisture conditions. The Atterberg Limits for a sample should be compared to its natural moisture content and plotted on the Plasticity Chart in order to determine the soil classification.

2. SOIL PROFILE AND DESCRIPTION

Each soil stratum is classified and described noting any special conditions. The Modified Unified Classification System (MUCS) is used. The soil profile refers to the existing ground level at the time the hole was done. Where available, the ground elevation is shown. The soil symbols used are shown in detail on the soil classification chart.

3. TESTS ON SOIL SAMPLES

Laboratory and field tests are identified by the following and are on the logs:

- N - Standard Penetration Test (SPT) Blow Count. The SPT is conducted in the field to assess the in-situ consistency of cohesive soils and the relative density of non-cohesive soils. The N value recorded is the number of blows from a 63.5 kg hammer dropped 760 mm which is required to drive a 51 mm split spoon sampler 300 mm into the soil.

- SO₄ - Water Soluble Sulphate Content. Expressed in percent. Conducted primarily to determine requirements for the use of sulphate resistant cement. Further details on the water-soluble sulphate content are given in Section 6.

- γ_D - Dry Unit Weight. Usually expressed in kN/m³.

- γ_T - Total Unit Weight. Usually expressed in kN/m³.

- Q_u - Unconfined Compressive Strength. Usually expressed in kPa and may be used in determining allowable bearing capacity of the soil.

- C_u - Undrained Shear Strength. Usually expressed in kPa. This value is determined by either a direct shear test or by an unconfined compression test and may also be used in determining the allowable bearing capacity of the soil.
- C_{PEN} - Pocket Penetrometer Reading. Usually expressed in kPa. Estimate of the undrained shear strength as determined by a pocket penetrometer.

The following tests may also be performed on selected soil samples and the results are given on separate sheets enclosed with the logs:

- Grain Size Analysis
- Standard or Modified Proctor Compaction Test
- California Bearing Ratio Test
- Direct Shear Test
- Permeability Test
- Consolidation Test
- Triaxial Test

4. SOIL DENSITY AND CONSISTENCY

The SPT test described above may be used to estimate the consistency of cohesive soils and the density of cohesionless soils. These approximate relationships are summarized in the following tables:

Table 1 Cohesive Soils

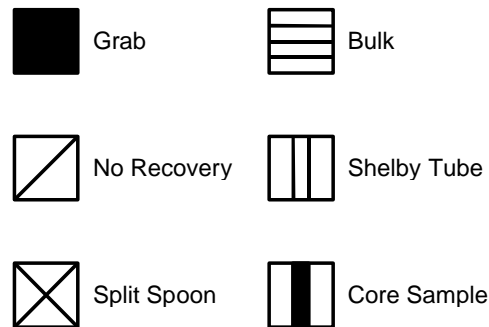
N	Consistency	C _u (kPa) approx.
0 - 1	Very Soft	<10
1 - 4	Soft	10 - 25
4 - 8	Firm	25 - 50
8 - 15	Stiff	50 - 100
15 - 30	Very Stiff	100 - 200
30 - 60	Hard	200 - 300
>60	Very Hard	>300

Table 2 Cohesionless Soils

N	Density
0 - 5	Very Loose
5 - 10	Loose
10 - 30	Compact
30 - 50	Dense
>50	Very Dense

5. SAMPLE CONDITION AND TYPE

The depth, type, and condition of samples are indicated on the logs by the following symbols:



6. WATER SOLUBLE SULPHATE CONCENTRATION

The following table, from CSA Standard A23.1-14, indicates the requirements for concrete subjected to sulphate attack based upon the percentage of water-soluble sulphate as presented on the logs. CSA Standard A23.1-14 should be read in conjunction with the table.

Table 3 Requirements for Concrete Subjected to Sulphate Attack*

Class of exposure	Degree of exposure	Water-soluble sulphate (SO ₄) [†] in soil sample, %	Sulphate (SO ₄) [‡] in groundwater samples, mg/L [‡]	Water soluble sulphate (SO ₄) in recycled aggregate sample, %	Cementing materials to be used ^{§††}	Performance requirements ^{§,§§}		
						Maximum expansion when tested using CSA A3004-C8 Procedure A at 23 °C, %		Maximum expansion when tested using CSA A3004-C8 Procedure B at 5 °C, % ^{†††}
						At 6 months	At 12 months ^{††}	At 18 months ^{‡‡}
S-1	Very severe	> 2.0	> 10 000	> 2.0	HS ^{**} , HSb, HSLb ^{***} or HSe	0.05	0.10	0.10
S-2	Severe	0.20–2.0	1500–10 000	0.60–2.0	HS ^{**} , HSb, HSLb ^{***} or HSe	0.05	0.10	0.10
S-3	Moderate (including seawater exposure*)	0.10–0.20	150–1500	0.20–0.60	MS, MSb, MSe, MSLb ^{***} , LH, LHb, HS ^{**} , HSb, HSLb ^{***} or HSe	0.10		0.10

*For sea water exposure, also see Clause 4.1.1.5.

[†]In accordance with CSA A23.2-3B.

[‡]In accordance with CSA A23.2-2B.

[§]Where combinations of supplementary cementing materials and portland or blended hydraulic cements are to be used in the concrete mix design instead of the cementing materials listed, and provided they meet the performance requirements demonstrating equivalent performance against sulphate exposure, they shall be designated as MS equivalent (MSe) or HS equivalent (HSe) in the relevant sulphate exposures (see Clauses 4.1.1.6.2, 4.2.1.1, and 4.2.1.3, and 4.2.1.4).

^{**}Type HS cement shall not be used in reinforced concrete exposed to both chlorides and sulphates, including seawater. See Clause 4.1.1.6.3.

††The requirement for testing at 5 °C does not apply to MS, HS, MSb, HSb, and MSe and HSe combinations made without portland limestone cement.

‡‡ If the increase in expansion between 12 and 18 months exceeds 0.03%, the sulphate expansion at 24 months shall not exceed 0.10% in order for the cement to be deemed to have passed the sulphate resistance requirement.

§§For demonstrating equivalent performance, use the testing frequency in Table 1 of CSA A3004-A1 and see the applicable notes to Table A3 in A3001 with regard to re-establishing compliance if the composition of the cementing materials used to establish compliance changes.

***Where MSLb or HSLb cements are proposed for use, or where MSe or HSe combinations include Portland-limestone cement, they must also contain a minimum of 25% Type F fly ash or 40% slag or 15% metakaolin (meeting Type N pozzolan requirements) or a combination of 5% Type SF silica fume with 25% slag or a combination of 5% Type SF silica fume with 20% Type F fly ash. For some proposed MSLb, HSLb, and MSe or HSe combinations that include Portland-limestone cement, higher SCM replacement levels may be required to meet the A3004-C8 Procedure B expansion limits. Due to the 18-month test period, SCM replacements higher than the identified minimum levels should also be tested. In addition, sulphate resistance testing shall be run on MSLb and HSLb cement and MSe or HSe combinations that include Portland-limestone cement at both 23 °C and 5 °C as specified in the table.

†††If the expansion is greater than 0.05% at 6 months but less than 0.10% at 1 year, the cementing materials combination under test shall be considered to have passed.

7. SOIL CORROSIVITY

The following table, from the Handbook of Corrosion Engineering (Roberge, 1999) indicates the corrosivity rating can be obtained from the soil resistivity, presented on the logs.

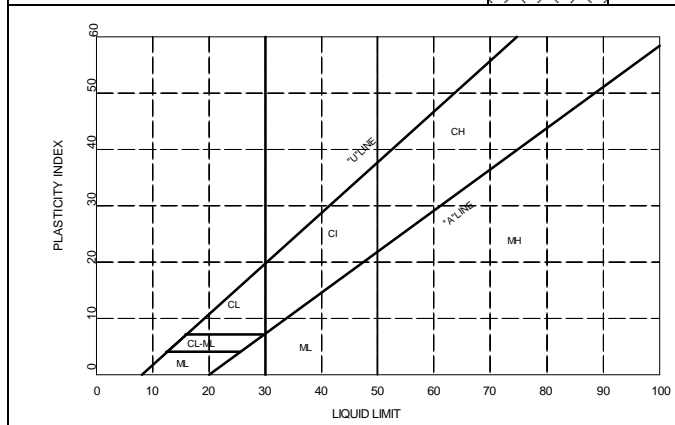
Table 4 Corrosivity Ratings Based on Soil Resistivity

Soil Resistivity (ohm-cm)	Corrosivity Rating
>20,000	Essentially non-corrosive
10,000 – 20,000	Mildly corrosive
5,000 – 10,000	Moderately corrosive
3,000 – 5,000	Corrosive
1,000 – 3,000	Highly corrosive
<1,000	Extremely corrosive

8. GROUNDWATER TABLE

The groundwater table is indicated by the equilibrium level of water in a standpipe installed in a testhole or test pit. This level is generally taken at least 24 hours after installation of the standpipe. The groundwater level is subject to seasonal variations and is usually highest in the spring. The symbol on the logs indicating the groundwater level is an inverted solid triangle (▼).

MAJOR DIVISION		LOG SYMBOLS	UCS	TYPICAL DESCRIPTION	LABORATORY CLASSIFICATION CRITERIA			
COARSE GRAINED SOILS	GRAVELS (MORE THAN HALF COARSE GRAINS LARGER THAN 4.75 mm)	CLEAN GRAVELS (LITTLE OR NO FINES)	GW	WELL GRADED GRAVELS, LITTLE OR NO FINES	$C_u - \frac{D_{60}}{D_{10}} > 4$ $C_c - \frac{(D_{30})^2}{D_{10} \times D_{60}} = 1$ to 3			
		GRAVELS WITH FINES	GP	POORLY GRADED GRAVELS AND GRAVEL-SAND MIXTURES, LITTLE OR NO FINES	NOT MEETING ABOVE REQUIREMENTS			
			GM	SILTY GRAVELS, GRAVEL-SAND-SILT MIXTURES	CONTENT OF FINES EXCEEDS 12% ATTERBERG LIMITS BELOW 'A' LINE W_p LESS THAN 4 ATTERBERG LIMITS ABOVE 'A' LINE W_p MORE THAN 7			
		GC	CLAYEY GRAVELS, GRAVEL-SAND-CLAY MIXTURES					
	SANDS (MORE THAN HALF COARSE GRAINS SMALLER THAN 4.75 mm)	CLEAN SANDS (LITTLE R NO FINES)	SW	WELL GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES	$C_u - \frac{D_{60}}{D_{10}} > 6$ $C_c - \frac{(D_{30})^2}{D_{10} \times D_{60}} = 1$ to 3			
			SP	POORLY GRADED SANDS, LITTLE OR NO FINES	NOT MEETING ABOVE REQUIREMENTS			
		SANDS WITH FINES	SM	SILTY SANDS, SAND-SILT MIXTURES	CONTENT OF FINES EXCEEDS 12% ATTERBERG LIMITS BELOW 'A' LINE W_p LESS THAN 4 ATTERBERG LIMITS ABOVE 'A' LINE W_p MORE THAN 7			
			SC	CLAYEY SANDS, SAND-CLAY MIXTURES				
			HIGHLY ORGANIC SOILS			Pt	PEAT AND OTHER HIGHLY ORGANIC SOILS	STRONG COLOUR OR ODOUR, AND OFTEN FIBROUS TEXTURE
			BEDROCK			BR	SEE REPORT DESCRIPTION	
FILL		FILL	SEE REPORT DESCRIPTION					



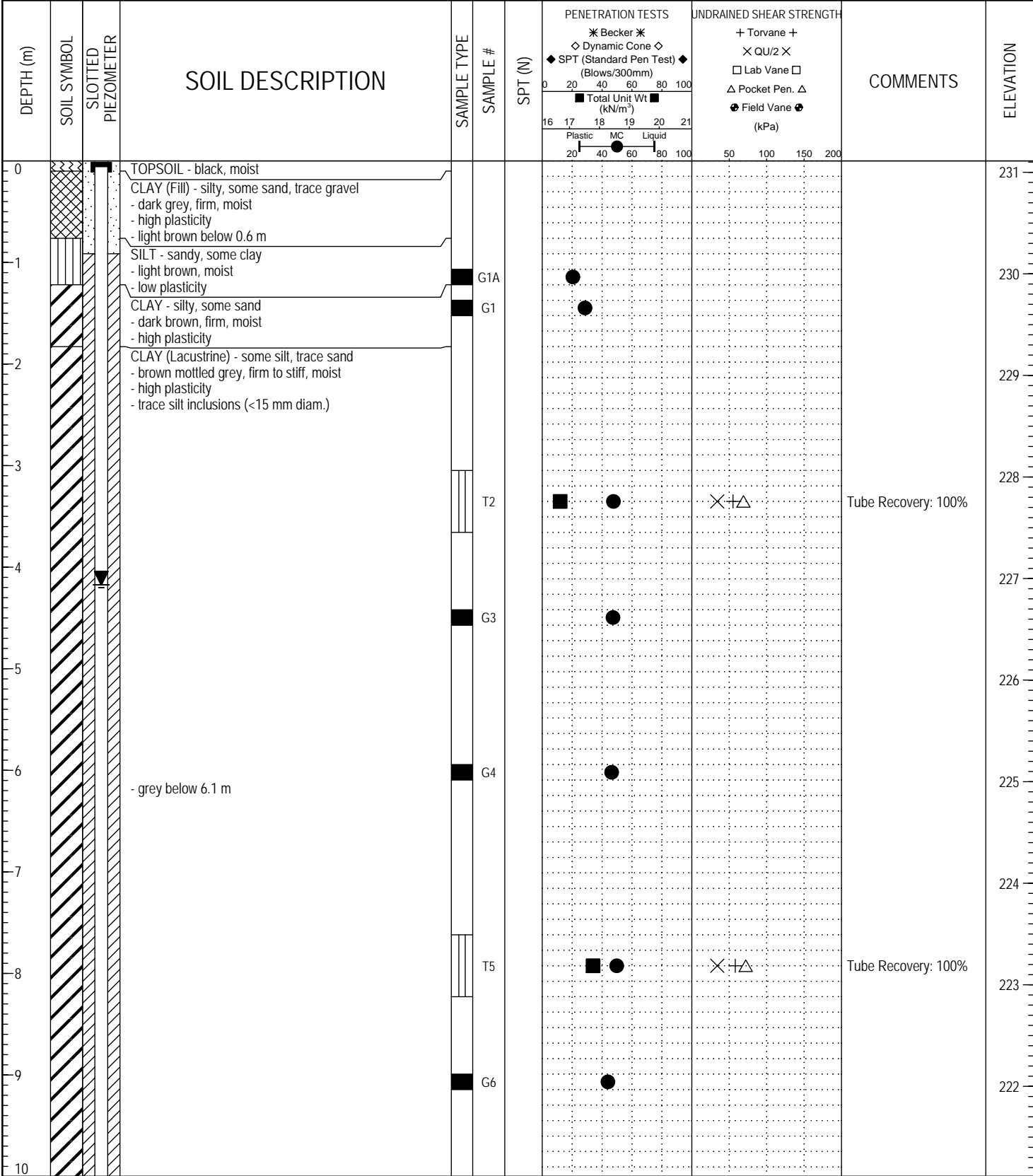
NOTE:
 1. BOUNDARY CLASSIFICATION POSSESSING CHARACTERISTICS OF TWO GROUPS ARE GIVEN GROUP SYMBOLS, E.G. GW-GC IS A WELL GRADED GRAVEL MIXTURE WITH CLAY BINDER BETWEEN 5% AND 12%

SOIL COMPONENTS					
FRACTION		SIEVE SIZE (mm)		DEFINING RANGES OF PERCENTAGE BY WEIGHT OF MINOR COMPONENTS	
		PASSING	RETAINED	PERCENT	IDENTIFIER
GRAVEL	COARSE	75	19	50 - 35	AND
	FINE	19	4.75		
SAND	COARSE	4.75	2.00	35 - 20	Y
	MEDIUM	2.00	0.425		
	FINE	0.425	0.080		
SILT (non-plastic) or CLAY (plastic)		0.080		20 - 10	SOME
				10 - 1	TRACE
OVERSIZE MATERIALS					
ROUNDED OR SUB-ROUNDED COBBLES 75 mm TO 200 mm BOULDERS >200 mm			ANGULAR ROCK FRAGMENTS ROCKS > 0.75 m3 IN VOLUME		

MODIFIED UNIFIED SOIL CLASSIFICATION SYSTEM

August 2015

PROJECT: Jefferson East CSR Works (Contract 5)		CLIENT: City of Winnipeg WWD		TESTHOLE NO: TH19-01		
LOCATION: UTM 14 - 5533995 m N, 634036 m E				PROJECT NO.: 60599385		
CONTRACTOR: Maple Leaf Drilling			METHOD: Canterra CT-250 - 125 mm SSA		ELEVATION (m): 231.11	
SAMPLE TYPE	GRAB	SHELBY TUBE	SPLIT SPOON	BULK	NO RECOVERY	CORE
BACKFILL TYPE	BENTONITE	GRAVEL	SLOUGH	GROUT	CUTTINGS	SAND

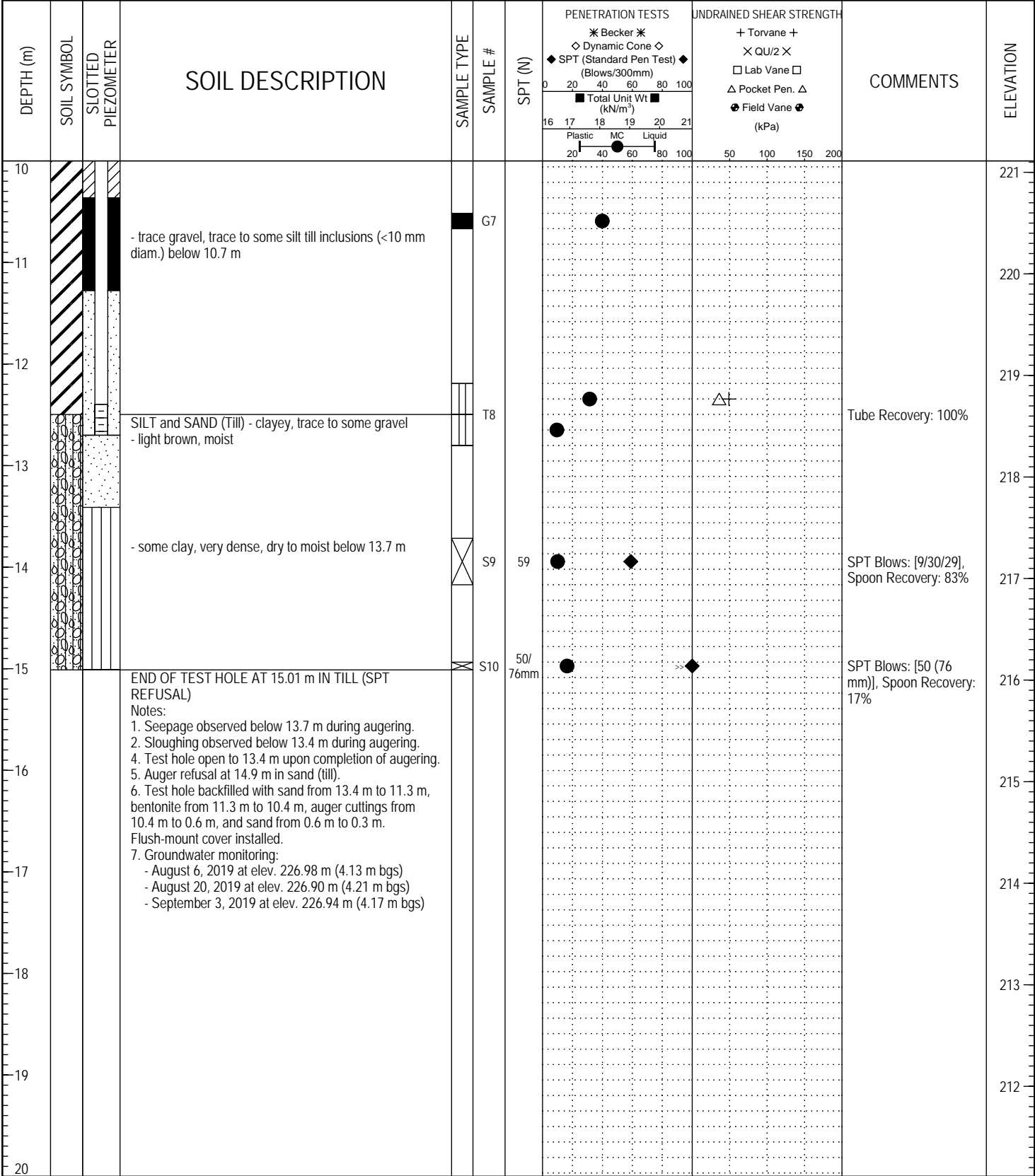


LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 15.01 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/27/19
PROJECT ENGINEER: Jordan T.	Page 1 of 2

PROJECT: Jefferson East CSR Works (Contract 5)		CLIENT: City of Winnipeg WWD		TESTHOLE NO: TH19-01		
LOCATION: UTM 14 - 5533995 m N, 634036 m E				PROJECT NO.: 60599385		
CONTRACTOR: Maple Leaf Drilling			METHOD: Canterra CT-250 - 125 mm SSA		ELEVATION (m): 231.11	
SAMPLE TYPE	GRAB	SHELBY TUBE	SPLIT SPOON	BULK	NO RECOVERY	CORE
BACKFILL TYPE	BENTONITE	GRAVEL	SLOUGH	GROUT	CUTTINGS	SAND



LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 15.01 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/27/19
PROJECT ENGINEER: Jordan T.	Page 2 of 2

PROJECT: Jefferson East CSR Works (Contract 5) CLIENT: City of Winnipeg WWD TESTHOLE NO: TH19-02
 LOCATION: UTM 14 - 5533973 m N, 634084 m E PROJECT NO.: 60599385
 CONTRACTOR: Maple Leaf Drilling METHOD: Acker MP-5 - 125 mm SSA ELEVATION (m): 231.28

SAMPLE TYPE GRAB SHELBY TUBE SPLIT SPOON BULK NO RECOVERY CORE

DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	SPT (N)	PENETRATION TESTS		UNDRAINED SHEAR STRENGTH (kPa)	COMMENTS	ELEVATION
						Becker * Dynamic Cone ◊ SPT (Standard Pen Test) ◆ (Blows/300mm)	Total Unit Wt (kN/m ³)			
0		TOPSOIL - black, moist								231
0		CLAY (Fill) - silty, some sand, trace gravel, trace roots - dark grey, firm, moist, high plasticity								
0		SAND - silty, trace clay - light brown, moist								
1				G16A						230
1		CLAY - silty, some sand - dark brown, firm, moist - high plasticity		G16						229
2		CLAY (Lacustrine) - some silt to silty, trace sand, trace gravel - brown mottled grey, firm, moist - high plasticity - trace silt inclusions (<15 mm diam.)								228
3				G17						227
4										226
5		- grey below 4.6 m		G18						225
6										224
7		- silty, some sand, stiff below 6.1 m		T19					Tube Recovery: 100%, (T19) Gravel: 2.0%, Sand: 19.0%, Silt: 35.0%, Clay: 44.0%, Swell: (3.1%, 120 kPa)	223
8				G20						222
9				G21						222

LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras COMPLETION DEPTH: 12.19 m
 REVIEWED BY: Faris Alobaidy COMPLETION DATE: 6/25/19
 PROJECT ENGINEER: Jordan T. Page 1 of 2

PROJECT: Jefferson East CSR Works (Contract 5)	CLIENT: City of Winnipeg WWD	TESTHOLE NO: TH19-02
LOCATION: UTM 14 - 5533973 m N, 634084 m E		PROJECT NO.: 60599385
CONTRACTOR: Maple Leaf Drilling	METHOD: Acker MP-5 - 125 mm SSA	ELEVATION (m): 231.28
SAMPLE TYPE <input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input checked="" type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE		

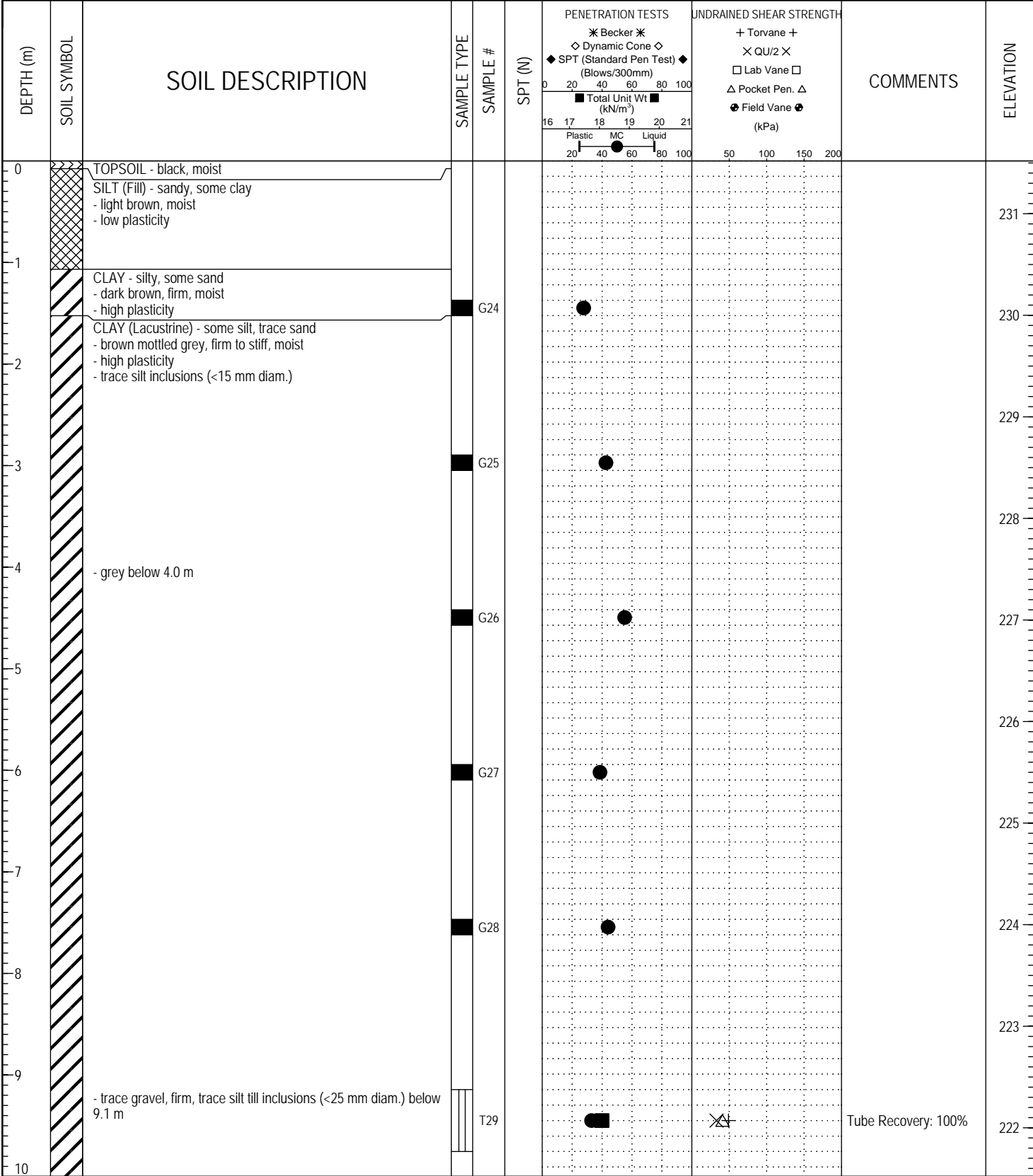
DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	SPT (N)	PENETRATION TESTS	UNDRAINED SHEAR STRENGTH	COMMENTS	ELEVATION
10		- trace silt till inclusions (<15 mm diam.) below 10.7 m		G22		●			221
11									220
12				G23		●			219
13		END OF TEST HOLE AT 12.19 m IN CLAY Notes: 1. Seepage not observed during augering. 2. Sloughing not observed during augering. 3. Test hole backfilled with auger cuttings and bentonite upon completion.							218
14									217
15									216
16									215
17									214
18									213
19									212
20									211

LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 12.19 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/25/19
PROJECT ENGINEER: Jordan T.	Page 2 of 2

PROJECT: Jefferson East CSR Works (Contract 5)	CLIENT: City of Winnipeg WWD	TESTHOLE NO: TH19-03
LOCATION: UTM 14 - 5533922 m N, 634193 m E		PROJECT NO.: 60599385
CONTRACTOR: Maple Leaf Drilling	METHOD: Acker MP-5 - 125 mm SSA	ELEVATION (m): 231.52
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input checked="" type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE	



LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 12.19 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/25/19
PROJECT ENGINEER: Jordan T.	Page 1 of 2

PROJECT: Jefferson East CSR Works (Contract 5) CLIENT: City of Winnipeg WWD TESTHOLE NO: TH19-03
 LOCATION: UTM 14 - 5533922 m N, 634193 m E PROJECT NO.: 60599385
 CONTRACTOR: Maple Leaf Drilling METHOD: Acker MP-5 - 125 mm SSA ELEVATION (m): 231.52

SAMPLE TYPE GRAB SHELBY TUBE SPLIT SPOON BULK NO RECOVERY CORE

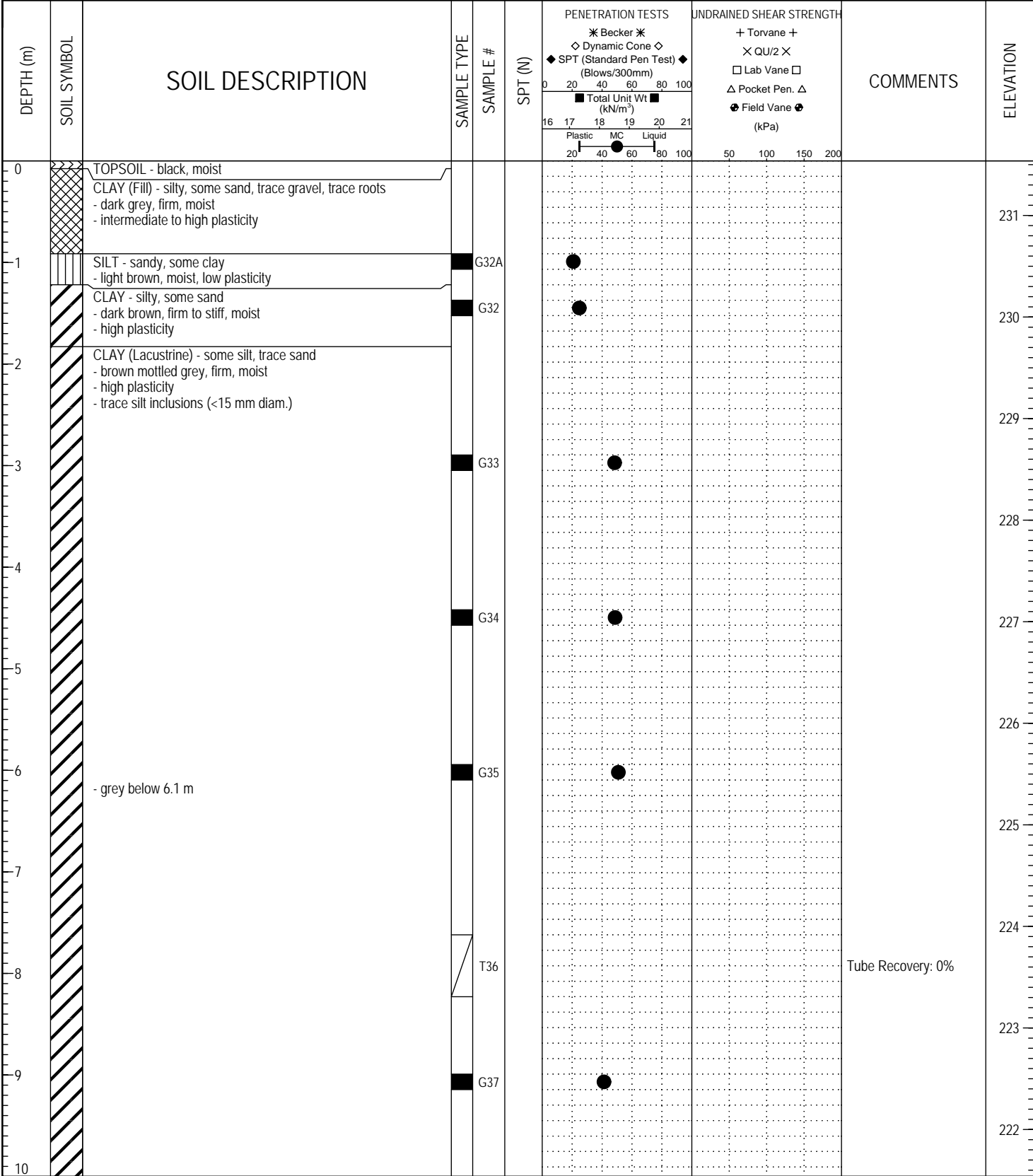
DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	SPT (N)	PENETRATION TESTS		UNDRAINED SHEAR STRENGTH (kPa)	COMMENTS	ELEVATION
						Blows/300mm	Total Unit Wt (kN/m ³)			
10										221
11										220
12				G30						219
12.19		END OF TEST HOLE AT 12.19 m IN CLAY		G31						219
13		Notes: 1. Seepage not observed during augering. 2. Sloughing not observed during augering. 3. Test hole backfilled with auger cuttings and bentonite upon completion.								218
14										217
15										216
16										215
17										214
18										213
19										212
20										212

LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras COMPLETION DEPTH: 12.19 m
 REVIEWED BY: Faris Alobaidy COMPLETION DATE: 6/25/19
 PROJECT ENGINEER: Jordan T. Page 2 of 2

PROJECT: Jefferson East CSR Works (Contract 5)	CLIENT: City of Winnipeg WWD	TESTHOLE NO: TH19-04
LOCATION: UTM 14 - 5533885 m N, 634272 m E		PROJECT NO.: 60599385
CONTRACTOR: Maple Leaf Drilling	METHOD: Acker MP-5 - 125 mm SSA	ELEVATION (m): 231.54
SAMPLE TYPE <input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input checked="" type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE		



LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 12.19 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/25/19
PROJECT ENGINEER: Jordan T.	Page 1 of 2

PROJECT: Jefferson East CSR Works (Contract 5)	CLIENT: City of Winnipeg WWD	TESTHOLE NO: TH19-04
LOCATION: UTM 14 - 5533885 m N, 634272 m E		PROJECT NO.: 60599385
CONTRACTOR: Maple Leaf Drilling	METHOD: Acker MP-5 - 125 mm SSA	ELEVATION (m): 231.54
SAMPLE TYPE <input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input checked="" type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE		

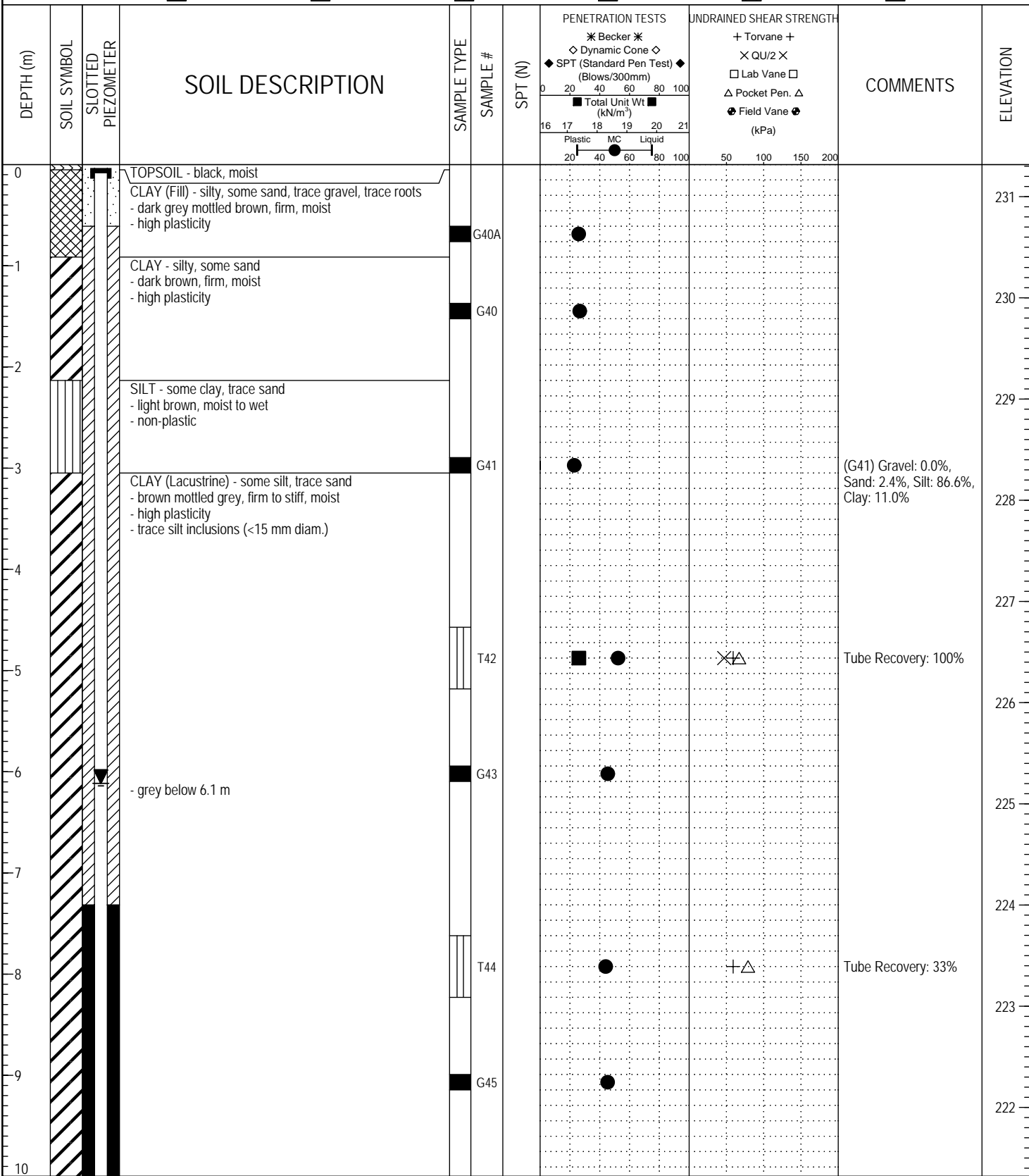
DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	PENETRATION TESTS (Blows/300mm)	UNDRAINED SHEAR STRENGTH (kPa)	COMMENTS	ELEVATION
10				G38	●			221
11								220
12				G39	●			219
13		END OF TEST HOLE AT 12.19 m IN CLAY Notes: 1. Seepage not observed during augering. 2. Sloughing not observed during augering. 3. Test hole backfilled with auger cuttings and bentonite upon completion.						218
14								217
15								216
16								215
17								214
18								213
19								212
20								211

LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 12.19 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/25/19
PROJECT ENGINEER: Jordan T.	Page 2 of 2

PROJECT: Jefferson East CSR Works (Contract 5)		CLIENT: City of Winnipeg WWD		TESTHOLE NO: TH19-05		
LOCATION: UTM 14 - 5533828 m N, 634394 m E				PROJECT NO.: 60599385		
CONTRACTOR: Maple Leaf Drilling		METHOD: Acker MP-5 - 125 mm SSA		ELEVATION (m): 231.32		
SAMPLE TYPE	GRAB	SHELBY TUBE	SPLIT SPOON	BULK	NO RECOVERY	CORE
BACKFILL TYPE	BENTONITE	GRAVEL	SLOUGH	GROUT	CUTTINGS	SAND

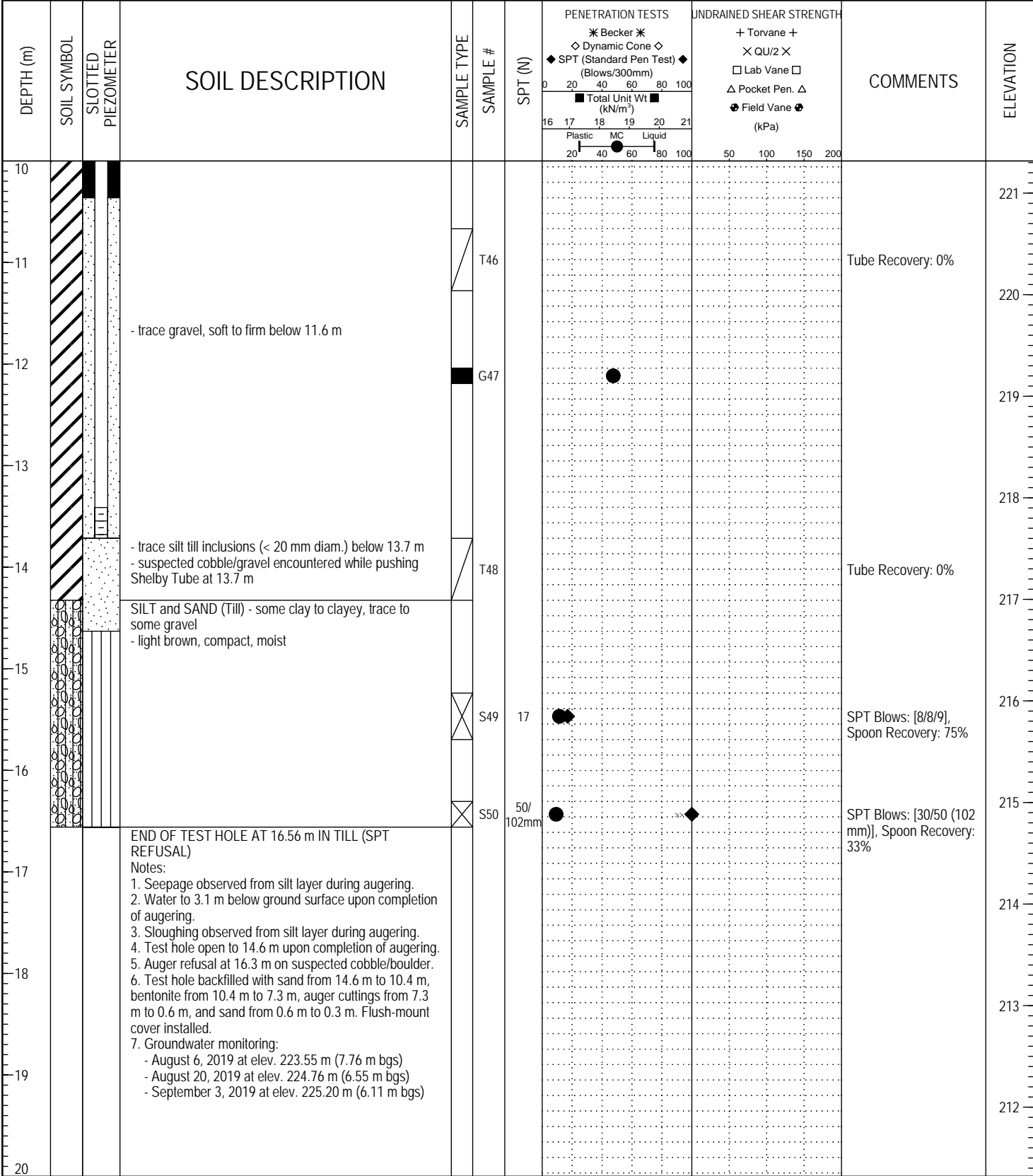


LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 16.56 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/26/19
PROJECT ENGINEER: Jordan T.	Page 1 of 2

PROJECT: Jefferson East CSR Works (Contract 5)		CLIENT: City of Winnipeg WWD		TESTHOLE NO: TH19-05		
LOCATION: UTM 14 - 5533828 m N, 634394 m E				PROJECT NO.: 60599385		
CONTRACTOR: Maple Leaf Drilling			METHOD: Acker MP-5 - 125 mm SSA		ELEVATION (m): 231.32	
SAMPLE TYPE	GRAB	SHELBY TUBE	SPLIT SPOON	BULK	NO RECOVERY	CORE
BACKFILL TYPE	BENTONITE	GRAVEL	SLOUGH	GROUT	CUTTINGS	SAND

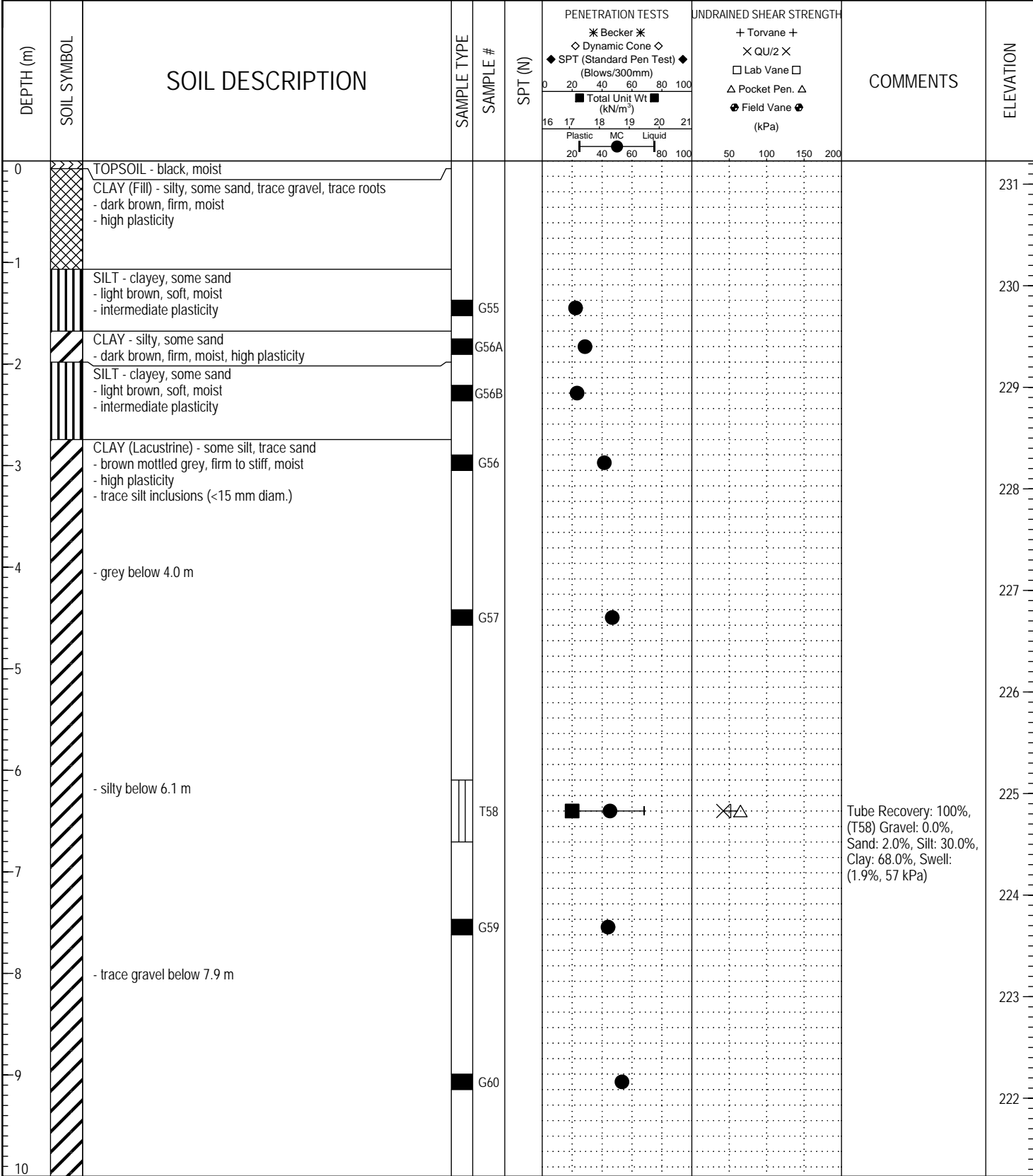


LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 16.56 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/26/19
PROJECT ENGINEER: Jordan T.	Page 2 of 2

PROJECT: Jefferson East CSR Works (Contract 5)	CLIENT: City of Winnipeg WWD	TESTHOLE NO: TH19-06
LOCATION: UTM 14 - 5533801 m N, 634449 m E		PROJECT NO.: 60599385
CONTRACTOR: Maple Leaf Drilling	METHOD: Acker MP-5 - 125 mm SSA	ELEVATION (m): 231.23
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE	



LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 12.19 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/26/19
PROJECT ENGINEER: Jordan T.	Page 1 of 2

PROJECT: Jefferson East CSR Works (Contract 5)	CLIENT: City of Winnipeg WWD	TESTHOLE NO: TH19-06
LOCATION: UTM 14 - 5533801 m N, 634449 m E		PROJECT NO.: 60599385
CONTRACTOR: Maple Leaf Drilling	METHOD: Acker MP-5 - 125 mm SSA	ELEVATION (m): 231.23
SAMPLE TYPE <input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input checked="" type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE		

DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	SPT (N)	UNDRAINED SHEAR STRENGTH (kPa)	COMMENTS	ELEVATION
10		- trace silt till inclusions (<25 mm diam.) below 10.7 m		G61	55			221
12				G62	65			219
12.19		END OF TEST HOLE AT 12.19 m IN CLAY Notes: 1. Seepage not observed during augering. 2. Sloughing not observed during augering. 3. Test hole backfilled with auger cuttings and bentonite upon completion.						

LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 12.19 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/26/19
PROJECT ENGINEER: Jordan T.	Page 2 of 2

PROJECT: Jefferson East CSR Works (Contract 5) CLIENT: City of Winnipeg WWD TESTHOLE NO: TH19-07
 LOCATION: UTM 14 - 5533750 m N, 634559 m E PROJECT NO.: 60599385
 CONTRACTOR: Maple Leaf Drilling METHOD: Acker MP-5 - 125 mm SSA ELEVATION (m): 231.13

SAMPLE TYPE GRAB SHELBY TUBE SPLIT SPOON BULK NO RECOVERY CORE

DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	SPT (N)	PENETRATION TESTS		UNDRAINED SHEAR STRENGTH		COMMENTS	ELEVATION
						◆ SPT (Standard Pen Test) (Blows/300mm)	■ Total Unit Wt (kN/m ³)	+ Torvane +	× QU/2 ×		
0		TOPSOIL - black, moist									231
0		CLAY (Fill) - silty, some sand, trace gravel, trace roots - dark grey mottled brown, firm, moist - high plasticity									
0		SAND - silty, trace clay - light brown, moist									
1											230
2		CLAY (Lacustrine) - some silt, trace sand - brown mottled grey, firm to stiff, moist - high plasticity - trace silt inclusions (<15 mm diam.)		G63	55	20					229
3				G64	65	45					228
4				G65	75	55					227
5				G66	85	65					226
6		- grey below 5.5 m									225
7		- trace gravel below 7.0 m									224
8				T67	85	65	100	100	100	Tube Recovery: 100%	223
9				G68	95	75					222

LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras COMPLETION DEPTH: 12.19 m
 REVIEWED BY: Faris Alobaidy COMPLETION DATE: 6/26/19
 PROJECT ENGINEER: Jordan T. Page 1 of 2

PROJECT: Jefferson East CSR Works (Contract 5)	CLIENT: City of Winnipeg WWD	TESTHOLE NO: TH19-07
LOCATION: UTM 14 - 5533750 m N, 634559 m E		PROJECT NO.: 60599385
CONTRACTOR: Maple Leaf Drilling	METHOD: Acker MP-5 - 125 mm SSA	ELEVATION (m): 231.13
SAMPLE TYPE <input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input checked="" type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE		

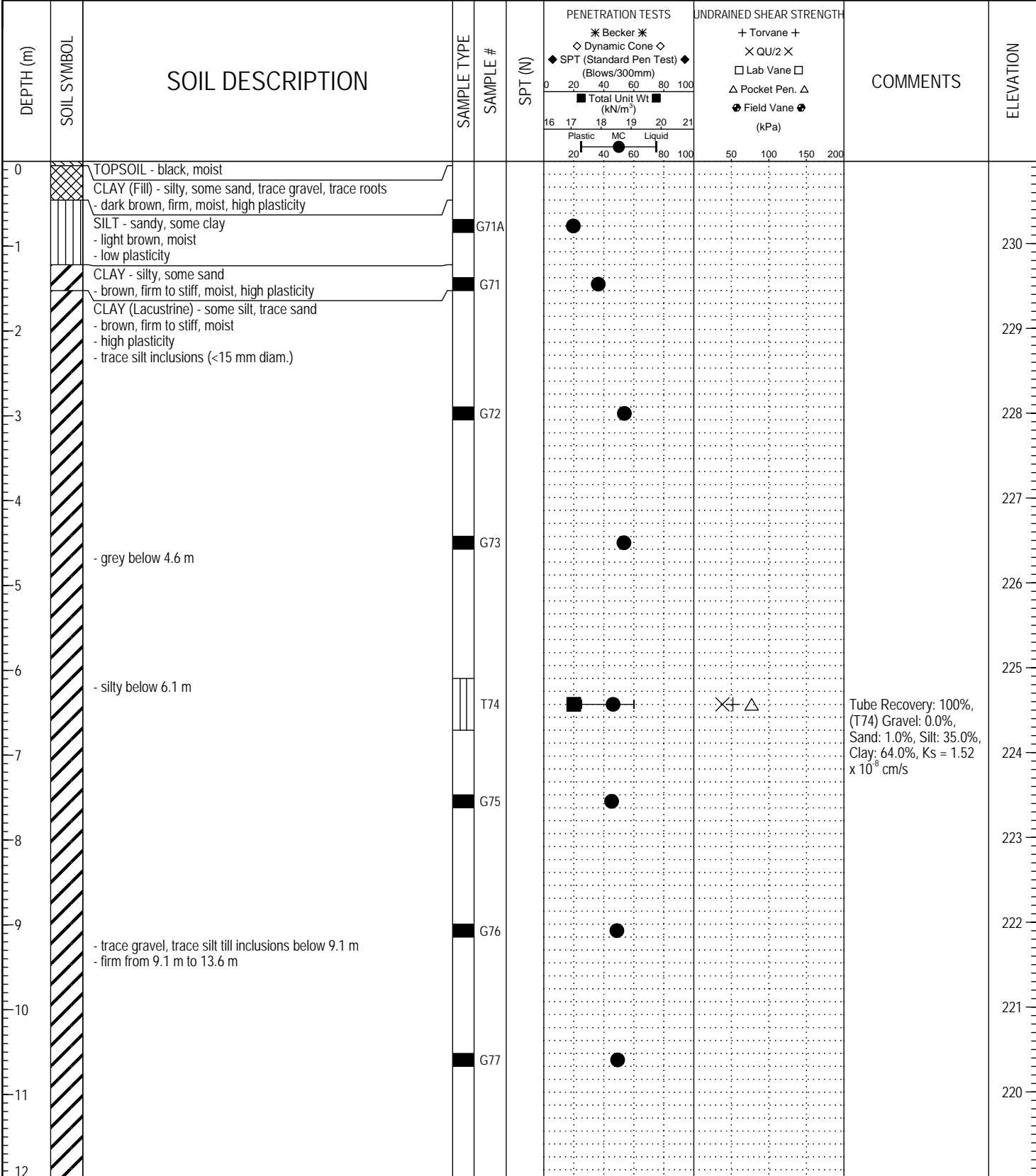
DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	SPT (N)	PENETRATION TESTS	UNDRAINED SHEAR STRENGTH	COMMENTS	ELEVATION
10				G69		●			221
11									220
12				G70		●			219
13		END OF TEST HOLE AT 12.19 m IN CLAY Notes: 1. Seepage not observed during augering. 2. Sloughing not observed during augering. 3. Test hole backfilled with auger cuttings and bentonite upon completion.							218
14									217
15									216
16									215
17									214
18									213
19									212
20									211

LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 12.19 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/26/19
PROJECT ENGINEER: Jordan T.	Page 2 of 2

PROJECT: Jefferson East CSR Works (Contract 5)	CLIENT: City of Winnipeg WWD	TESTHOLE NO: TH19-08
LOCATION: UTM 14 - 5533718 m N, 634627 m E		PROJECT NO.: 60599385
CONTRACTOR: Maple Leaf Drilling	METHOD: Acker MP-5 - 125 mm SSA	ELEVATION (m): 230.97
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input checked="" type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE	

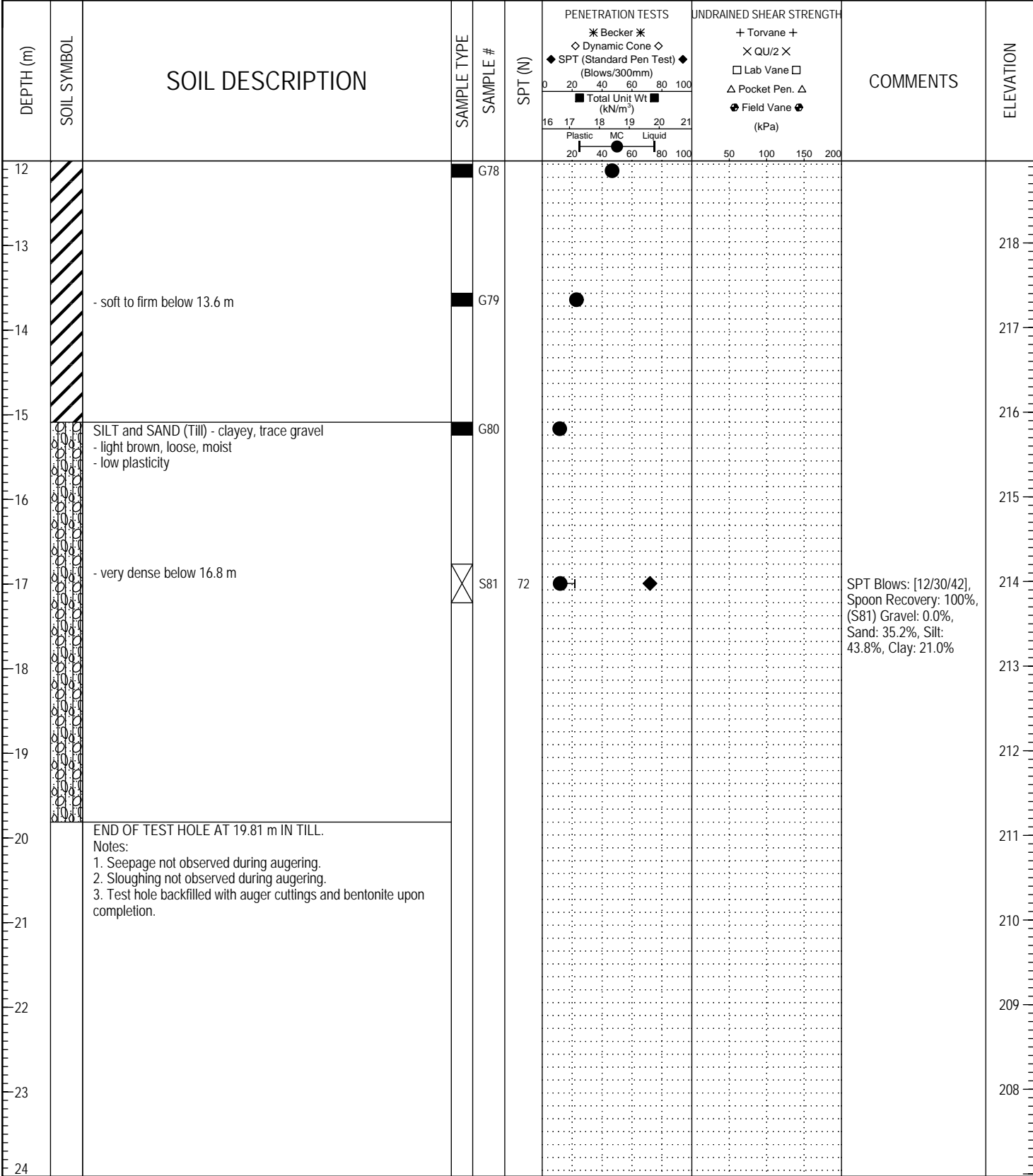


LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 19.81 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/26/19
PROJECT ENGINEER: Jordan T.	Page 1 of 2

PROJECT: Jefferson East CSR Works (Contract 5)	CLIENT: City of Winnipeg WWD	TESTHOLE NO: TH19-08
LOCATION: UTM 14 - 5533718 m N, 634627 m E		PROJECT NO.: 60599385
CONTRACTOR: Maple Leaf Drilling	METHOD: Acker MP-5 - 125 mm SSA	ELEVATION (m): 230.97
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input checked="" type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE	



LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 19.81 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/26/19
PROJECT ENGINEER: Jordan T.	Page 2 of 2

PROJECT: Jefferson East CSR Works (Contract 5) CLIENT: City of Winnipeg WWD TESTHOLE NO: TH19-10
 LOCATION: UTM 14 - 5533626 m N, 634825 m E PROJECT NO.: 60599385
 CONTRACTOR: Maple Leaf Drilling METHOD: Canterra CT-250 - 125 mm SSA ELEVATION (m): 230.73

SAMPLE TYPE GRAB SHELBY TUBE SPLIT SPOON BULK NO RECOVERY CORE

DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	SPT (N)	PENETRATION TESTS		UNDRAINED SHEAR STRENGTH		COMMENTS	ELEVATION
						Becker	Dynamic Cone	Torvane	QU/2		
0		TOPSOIL - black, moist									
0.4		CLAY (Fill) - silty, some sand, trace gravel, trace roots - dark grey, firm, moist - brown below 0.4 m - high plasticity									
1.0		CLAY - silty, some sand - dark brown, firm, moist - high plasticity		G91A	~45						230
1.5		SILT - sandy, some clay - light brown, moist - low plasticity		G91	~55						229
3.0		CLAY (Lacustrine) - some silt to silty, trace sand - brown mottled grey, firm to stiff, moist - high plasticity - trace silt inclusions (<15 mm diam.) - some silt below 3.1 m		G92	~65						228
4.5				G93	~75						227
5.5		- grey below 4.9 m		G94	~85						226
8.0				T95	~45					Tube Recovery: 100%	223
9.0				G96	~55						222
10.0											221

LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras COMPLETION DEPTH: 12.19 m
 REVIEWED BY: Faris Alobaidy COMPLETION DATE: 6/27/19
 PROJECT ENGINEER: Jordan T. Page 1 of 2

PROJECT: Jefferson East CSR Works (Contract 5)	CLIENT: City of Winnipeg WWD	TESTHOLE NO: TH19-10
LOCATION: UTM 14 - 5533626 m N, 634825 m E		PROJECT NO.: 60599385
CONTRACTOR: Maple Leaf Drilling	METHOD: Canterra CT-250 - 125 mm SSA	ELEVATION (m): 230.73

SAMPLE TYPE GRAB SHELBY TUBE SPLIT SPOON BULK NO RECOVERY CORE

DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	PENETRATION TESTS (Blows/300mm)	UNDRAINED SHEAR STRENGTH (kPa)	COMMENTS	ELEVATION
10				G97	●			220
11								219
12				G98	●			218
13		END OF TEST HOLE AT 12.19 m IN CLAY Notes: 1. Seepage not observed during augering. 2. Sloughing not observed during augering. 3. Test hole backfilled with auger cuttings and bentonite upon completion.						217
14								216
15								215
16								214
17								213
18								212
19								211
20								210

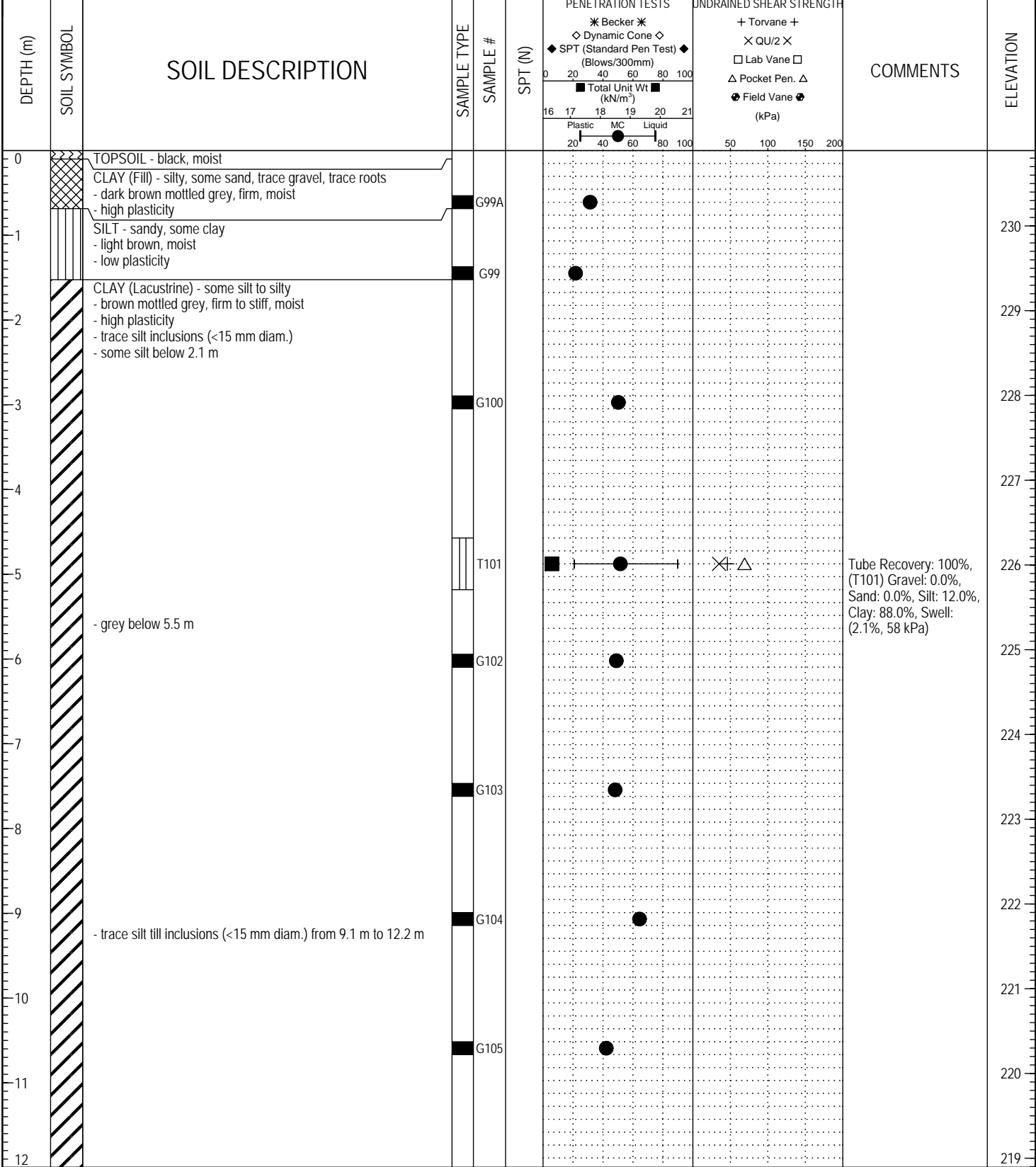
LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 12.19 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/27/19
PROJECT ENGINEER: Jordan T.	Page 2 of 2

PROJECT: Jefferson East CSR Works (Contract 5)	CLIENT: City of Winnipeg WWD	TESTHOLE NO: TH19-11
LOCATION: UTM 14 - 5533577 m N, 634929 m E		PROJECT NO.: 60599385
CONTRACTOR: Maple Leaf Drilling	METHOD: Canterra CT-250 - 125 mm SSA	ELEVATION (m): 230.89

SAMPLE TYPE GRAB SHELBY TUBE SPLIT SPOON BULK NO RECOVERY CORE



LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 19.81 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/27/19
PROJECT ENGINEER: Jordan T.	Page 1 of 2

PROJECT: Jefferson East CSR Works (Contract 5)	CLIENT: City of Winnipeg WWD	TESTHOLE NO: TH19-11
LOCATION: UTM 14 - 5533577 m N, 634929 m E		PROJECT NO.: 60599385
CONTRACTOR: Maple Leaf Drilling	METHOD: Canterra CT-250 - 125 mm SSA	ELEVATION (m): 230.89

SAMPLE TYPE GRAB SHELBY TUBE SPLIT SPOON BULK NO RECOVERY CORE

DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	SPT (N)	PENETRATION TESTS		UNDRAINED SHEAR STRENGTH (kPa)	COMMENTS	ELEVATION
						Blows/300mm	Total Unit Wt (kN/m ³)			
12		- trace gravel, trace silt till inclusions (<50 mm diam.) below 12.2 m	<input checked="" type="checkbox"/>	G106	60	200				218
13										
14				G107	45	180				217
15		- soft to firm below 15.1 m		G108	25	160				216
16										215
17		SILT and SAND (Till) - some clay to clayey, trace to some gravel - light brown, loose, moist	<input checked="" type="checkbox"/>	G109	55	190				214
18										213
19		- compact below 19.8 m		G110	55	180				212
20		END OF TEST HOLE AT 19.81 m IN TILL Notes: 1. Seepage not observed during augering. 2. Sloughing not observed during augering. 3. Test hole backfilled with auger cuttings and bentonite upon completion.								211
21										210
22										209
23										208
24										207

LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 19.81 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/27/19
PROJECT ENGINEER: Jordan T.	Page 2 of 2

PROJECT: Jefferson East CSR Works (Contract 5) CLIENT: City of Winnipeg WWD TESTHOLE NO: TH19-12
 LOCATION: UTM 14 - 5533542 m N, 635003 m E PROJECT NO.: 60599385
 CONTRACTOR: Maple Leaf Drilling METHOD: Acker MP-5 - 125 mm SSA ELEVATION (m): 230.76

SAMPLE TYPE GRAB SHELBY TUBE SPLIT SPOON BULK NO RECOVERY CORE

DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	SPT (N)	PENETRATION TESTS		UNDRAINED SHEAR STRENGTH	COMMENTS	ELEVATION
						Blows/300mm	Total Unit Wt (kN/m ³)			
0		TOPSOIL - black, moist								
0		CLAY (Fill) - silty, some sand, trace gravel, trace roots - dark grey, firm, moist - high plasticity								
1		CLAY - silty, some sand - brown, firm, moist - high plasticity		G111A						230
1		SILT - clayey, some sand - light brown, soft to firm, moist - intermediate plasticity		G111						229
3		CLAY (Lacustrine) - some silt, trace sand - brown, firm to stiff, moist - high plasticity - trace silt inclusions (<15 mm diam.) - brown mottled grey from 3.1 m to 4.3 m		G112						228
4		- grey, firm below 4.3 m		G113						227
5				G115						226
6				G116						225
6				T114					Tube Recovery: 100%	224
7				G115						223
8				G116						222
9										221

LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras COMPLETION DEPTH: 12.19 m
 REVIEWED BY: Faris Alobaidy COMPLETION DATE: 6/25/19
 PROJECT ENGINEER: Jordan T. Page 1 of 2

PROJECT: Jefferson East CSR Works (Contract 5)	CLIENT: City of Winnipeg WWD	TESTHOLE NO: TH19-12
LOCATION: UTM 14 - 5533542 m N, 635003 m E		PROJECT NO.: 60599385
CONTRACTOR: Maple Leaf Drilling	METHOD: Acker MP-5 - 125 mm SSA	ELEVATION (m): 230.76
SAMPLE TYPE <input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input checked="" type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE		

DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	SPT (N)	PENETRATION TESTS	UNDRAINED SHEAR STRENGTH	COMMENTS	ELEVATION
10									
11				G117		●			220
12				G118		●			219
13		END OF TEST HOLE AT 12.19 m IN CLAY Notes: 1. Seepage not observed during augering. 2. Sloughing not observed during augering. 3. Test hole backfilled with auger cuttings and bentonite upon completion.							218
14									217
15									216
16									215
17									214
18									213
19									212
20									211

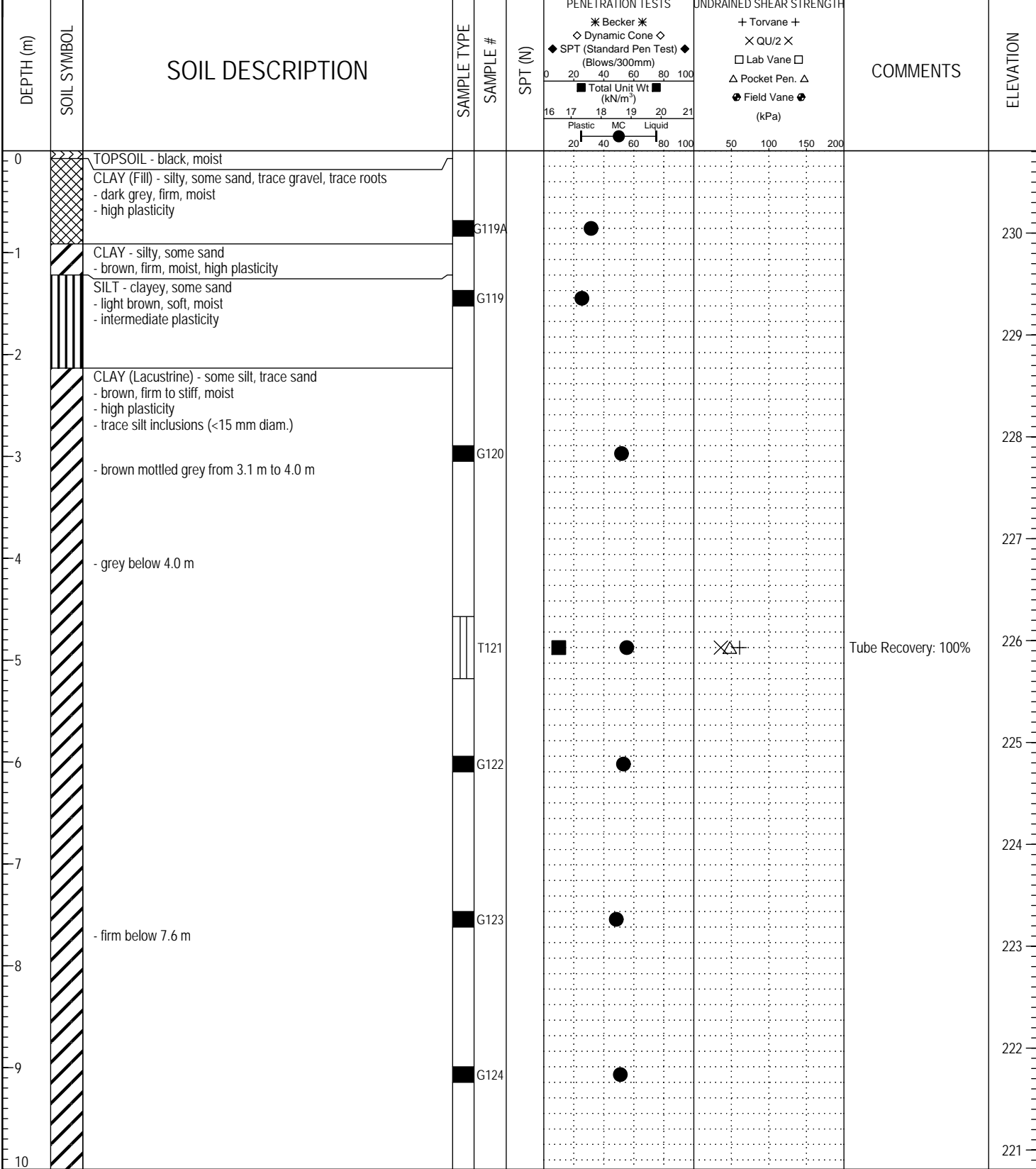
LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 12.19 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/25/19
PROJECT ENGINEER: Jordan T.	Page 2 of 2

PROJECT: Jefferson East CSR Works (Contract 5) CLIENT: City of Winnipeg WWD TESTHOLE NO: TH19-13
 LOCATION: UTM 14 - 5533487 m N, 635121 m E PROJECT NO.: 60599385
 CONTRACTOR: Maple Leaf Drilling METHOD: Acker MP-5 - 125 mm SSA ELEVATION (m): 230.81

SAMPLE TYPE GRAB SHELBY TUBE SPLIT SPOON BULK NO RECOVERY CORE



LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras COMPLETION DEPTH: 12.19 m
 REVIEWED BY: Faris Alobaidy COMPLETION DATE: 6/25/19
 PROJECT ENGINEER: Jordan T. Page 1 of 2

PROJECT: Jefferson East CSR Works (Contract 5)	CLIENT: City of Winnipeg WWD	TESTHOLE NO: TH19-13
LOCATION: UTM 14 - 5533487 m N, 635121 m E		PROJECT NO.: 60599385
CONTRACTOR: Maple Leaf Drilling	METHOD: Acker MP-5 - 125 mm SSA	ELEVATION (m): 230.81
SAMPLE TYPE <input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input checked="" type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE		

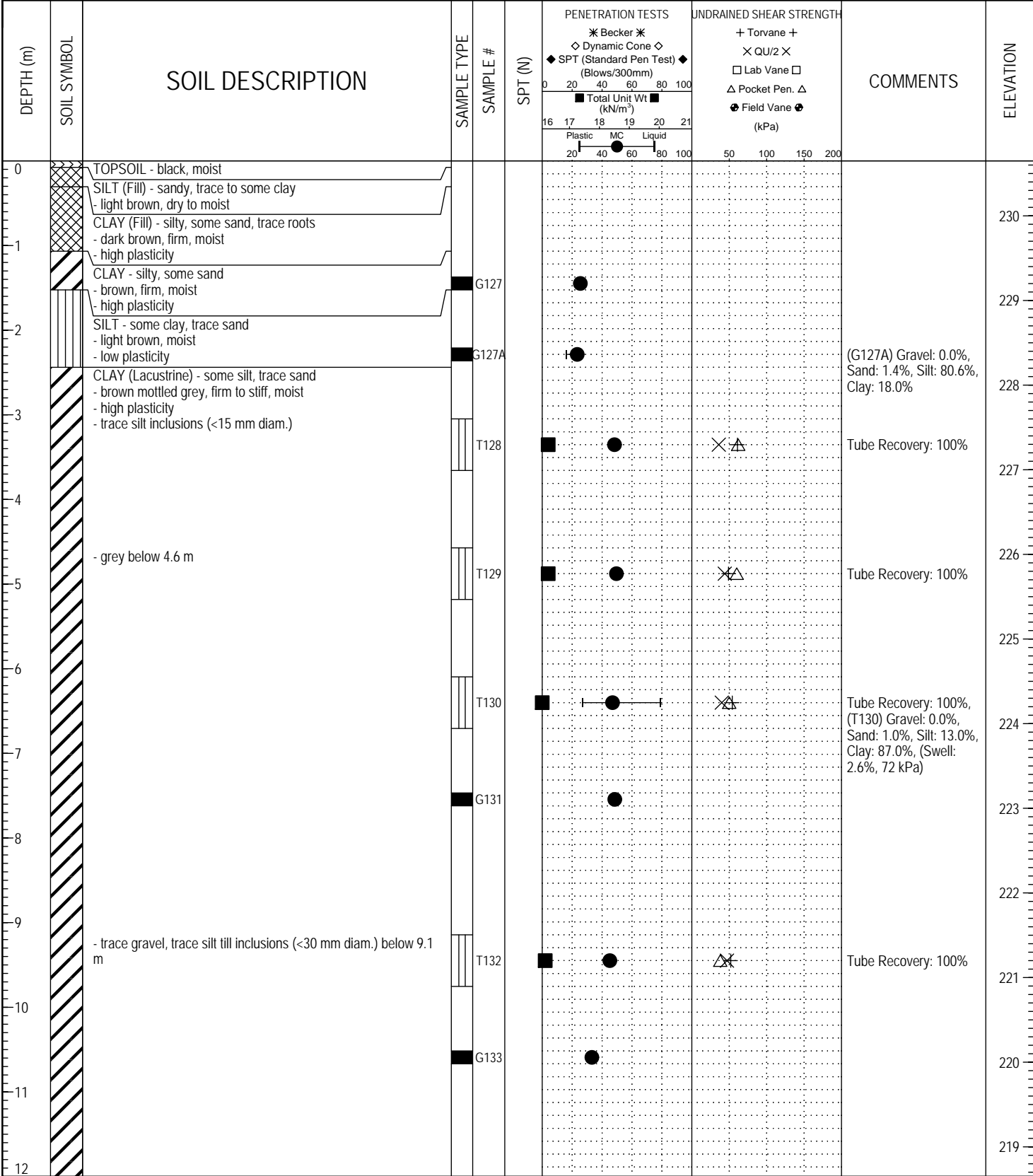
DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	PENETRATION TESTS SPT (N)	UNDRAINED SHEAR STRENGTH (kPa)	COMMENTS	ELEVATION
10								230.81
11				G125	~55			229.81
12				G126	~65			219.81
12.19		END OF TEST HOLE AT 12.19 m IN CLAY Notes: 1. Seepage not observed during augering. 2. Sloughing not observed during augering. 3. Test hole backfilled with auger cuttings and bentonite upon completion.						217.62
13								216.62
14								215.62
15								214.62
16								213.62
17								212.62
18								211.62
19								210.62
20								209.62

LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 12.19 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/25/19
PROJECT ENGINEER: Jordan T.	Page 2 of 2

PROJECT: Jefferson East CSR Works (Contract 5)	CLIENT: City of Winnipeg WWD	TESTHOLE NO: TH19-14
LOCATION: UTM 14 - 5533421 m N, 635261 m E		PROJECT NO.: 60599385
CONTRACTOR: Maple Leaf Drilling	METHOD: Acker MP-5 - 125 mm SSA	ELEVATION (m): 230.65
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input checked="" type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE	

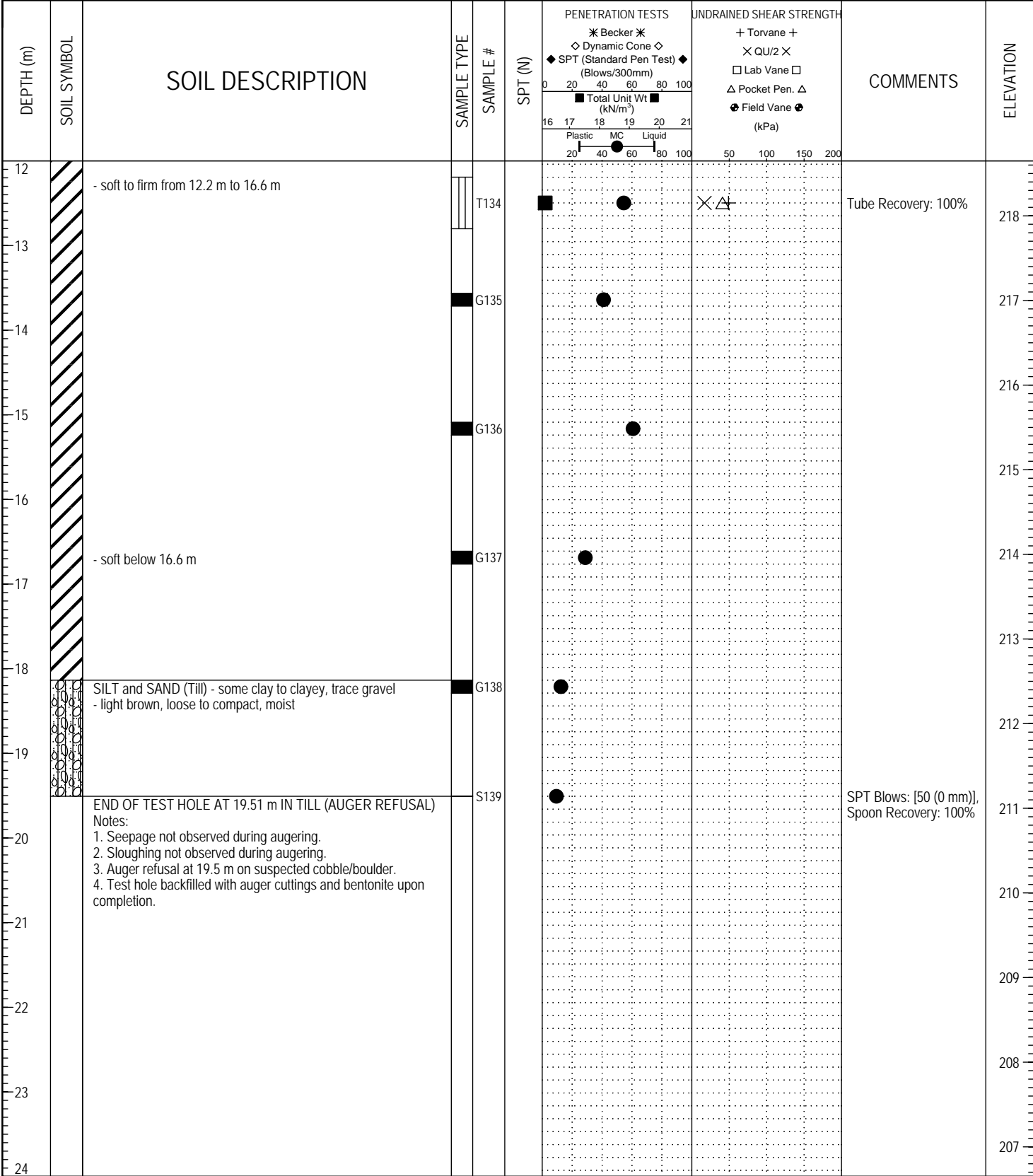


LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 19.51 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/25/19
PROJECT ENGINEER: Jordan T.	Page 1 of 2

PROJECT: Jefferson East CSR Works (Contract 5)	CLIENT: City of Winnipeg WWD	TESTHOLE NO: TH19-14
LOCATION: UTM 14 - 5533421 m N, 635261 m E		PROJECT NO.: 60599385
CONTRACTOR: Maple Leaf Drilling	METHOD: Acker MP-5 - 125 mm SSA	ELEVATION (m): 230.65
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input checked="" type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE	

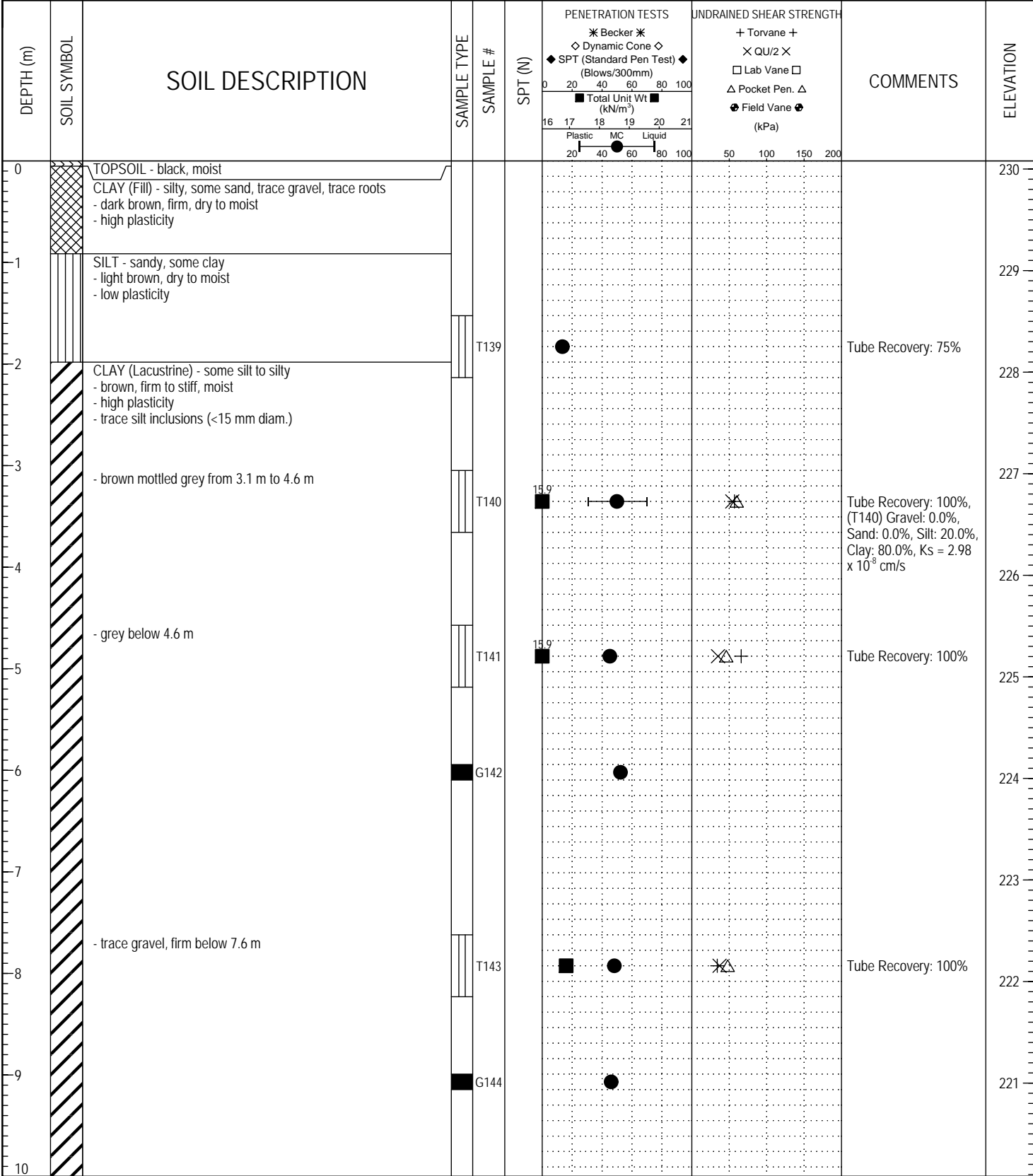


LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 19.51 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/25/19
PROJECT ENGINEER: Jordan T.	Page 2 of 2

PROJECT: Jefferson East CSR Works (Contract 5)	CLIENT: City of Winnipeg WWD	TESTHOLE NO: TH19-15
LOCATION: UTM 14 - 5533404 m N, 635298 m E		PROJECT NO.: 60599385
CONTRACTOR: Maple Leaf Drilling	METHOD: Acker MP-5 - 125 mm SSA	ELEVATION (m): 230.08
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE	



LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 11.28 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/24/19
PROJECT ENGINEER: Jordan T.	Page 1 of 2

PROJECT: Jefferson East CSR Works (Contract 5)	CLIENT: City of Winnipeg WWD	TESTHOLE NO: TH19-15
LOCATION: UTM 14 - 5533404 m N, 635298 m E		PROJECT NO.: 60599385
CONTRACTOR: Maple Leaf Drilling	METHOD: Acker MP-5 - 125 mm SSA	ELEVATION (m): 230.08
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input checked="" type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE	

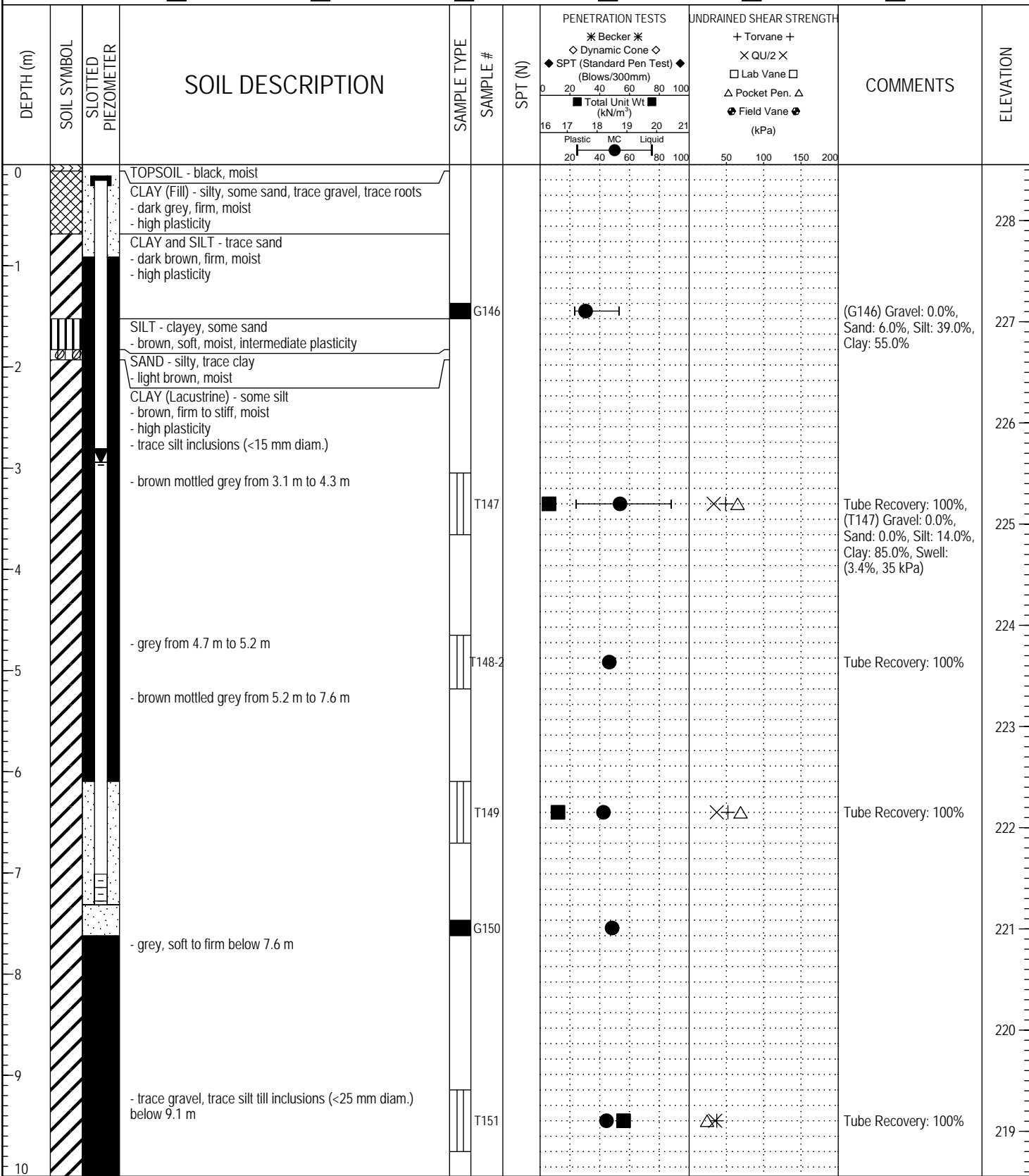
DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	SPT (N)	PENETRATION TESTS		UNDRAINED SHEAR STRENGTH	COMMENTS	ELEVATION
						Blows/300mm	Total Unit Wt (kN/m ³)			
10										220
11				T145					Tube Recovery: 54%	219
12		END OF TEST HOLE AT 11.28 m IN CLAY Notes: 1. Seepage not observed during augering. 2. Sloughing not observed during augering. 3. Test hole backfilled with auger cuttings and bentonite upon completion.								218
13										217
14										216
15										215
16										214
17										213
18										212
19										211
20										210

LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 11.28 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/24/19
PROJECT ENGINEER: Jordan T.	Page 2 of 2

PROJECT: Jefferson East CSR Works (Contract 5)		CLIENT: City of Winnipeg WWD		TESTHOLE NO: TH19-16		
LOCATION: UTM 14 - 5533376 m N, 635357 m E				PROJECT NO.: 60599385		
CONTRACTOR: Maple Leaf Drilling			METHOD: Acker MP-5 - 125 mm SSA		ELEVATION (m): 228.55	
SAMPLE TYPE	GRAB	SHELBY TUBE	SPLIT SPOON	BULK	NO RECOVERY	CORE
BACKFILL TYPE	BENTONITE	GRAVEL	SLOUGH	GROUT	CUTTINGS	SAND

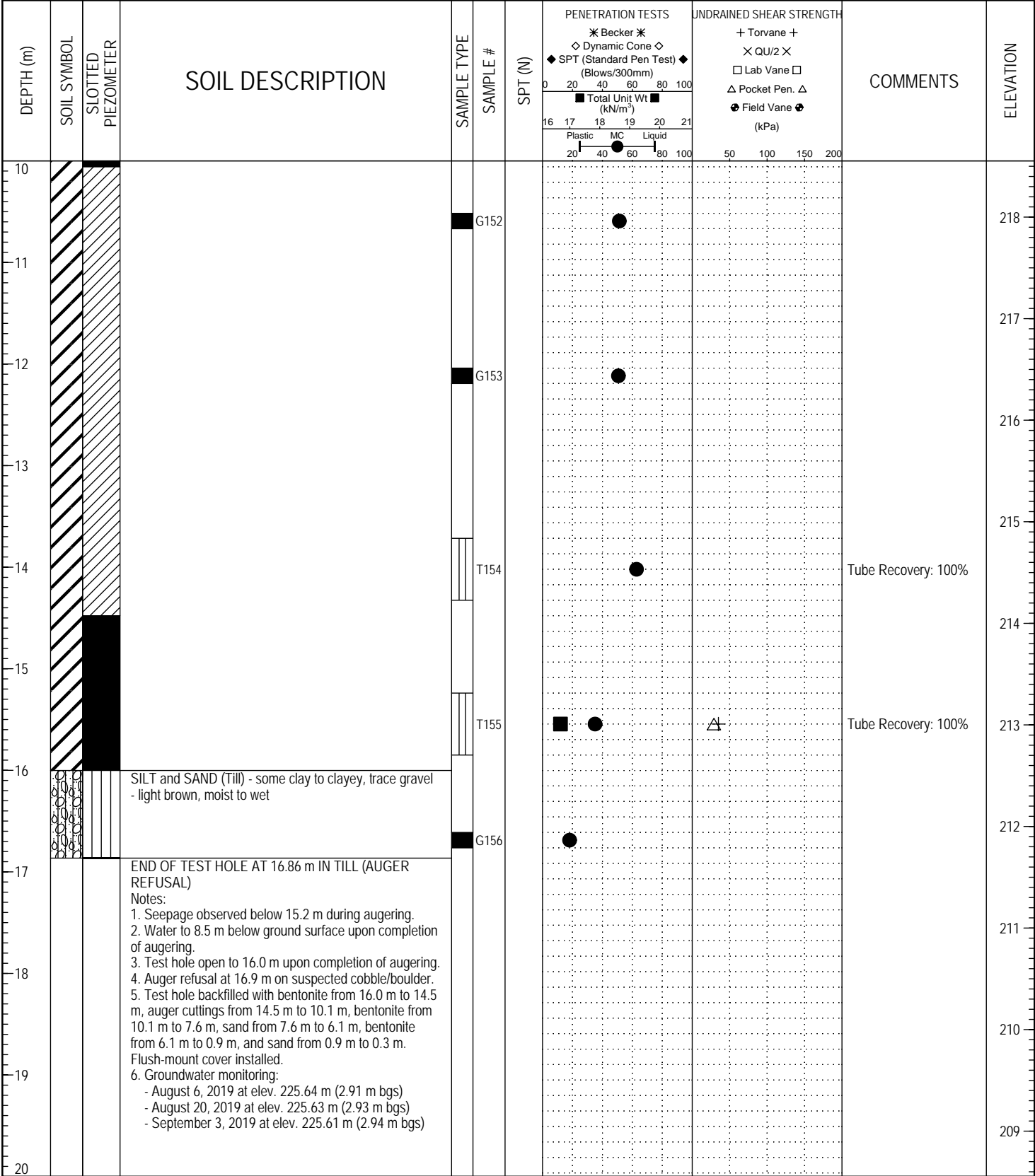


LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 16.86 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/24/19
PROJECT ENGINEER: Jordan T.	Page 1 of 2

PROJECT: Jefferson East CSR Works (Contract 5)		CLIENT: City of Winnipeg WWD		TESTHOLE NO: TH19-16		
LOCATION: UTM 14 - 5533376 m N, 635357 m E				PROJECT NO.: 60599385		
CONTRACTOR: Maple Leaf Drilling			METHOD: Acker MP-5 - 125 mm SSA		ELEVATION (m): 228.55	
SAMPLE TYPE	GRAB	SHELBY TUBE	SPLIT SPOON	BULK	NO RECOVERY	CORE
BACKFILL TYPE	BENTONITE	GRAVEL	SLOUGH	GROUT	CUTTINGS	SAND

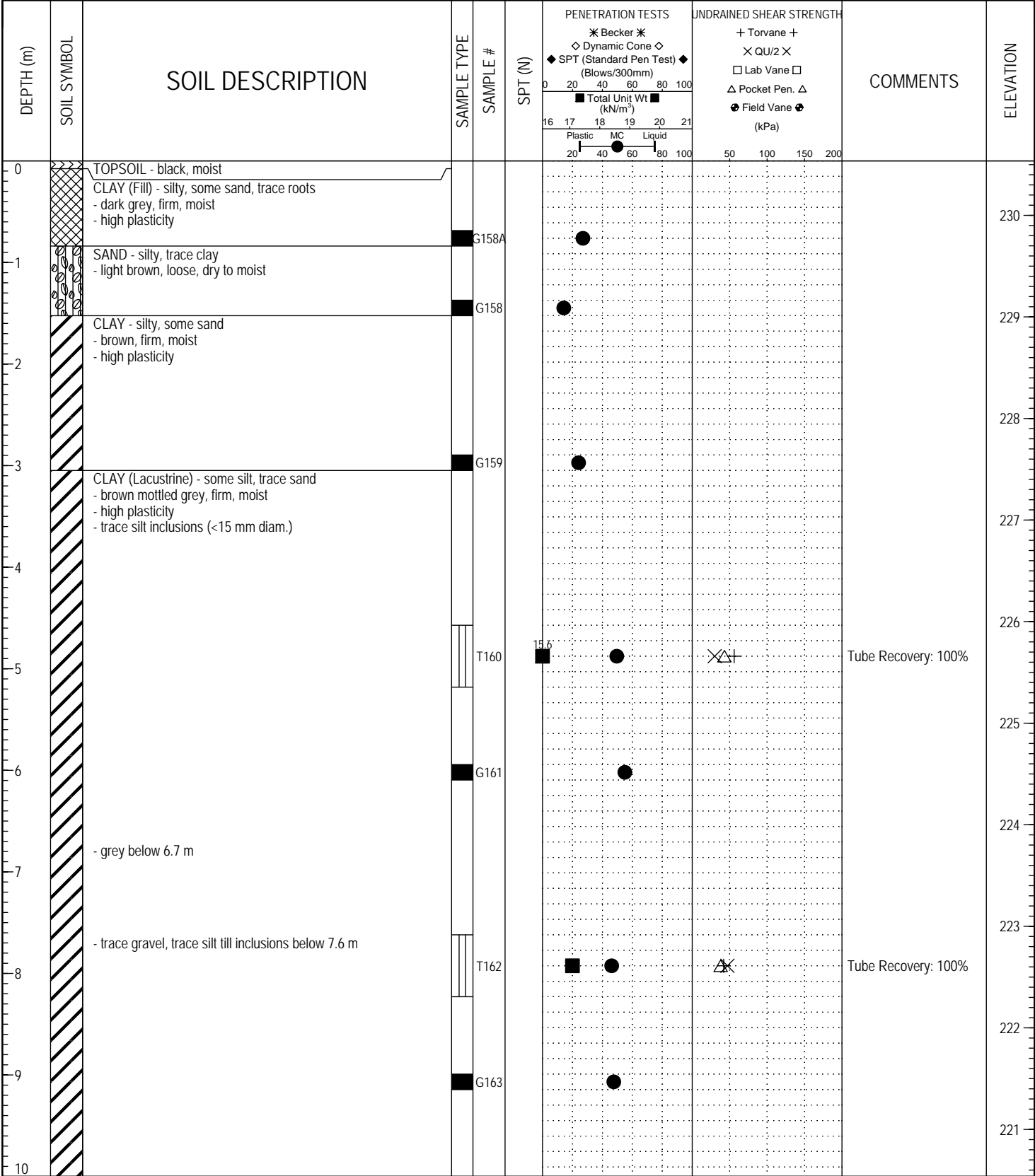


LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras		COMPLETION DEPTH: 16.86 m	
REVIEWED BY: Faris Alobaidy		COMPLETION DATE: 6/24/19	
PROJECT ENGINEER: Jordan T.		Page 2 of 2	

PROJECT: Jefferson East CSR Works (Contract 5)	CLIENT: City of Winnipeg WWD	TESTHOLE NO: TH19-17
LOCATION: UTM 14 - 5533349 m N, 635414 m E		PROJECT NO.: 60599385
CONTRACTOR: Maple Leaf Drilling	METHOD: Acker MP-5 - 125 mm SSA	ELEVATION (m): 230.54
SAMPLE TYPE	<input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE	



LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 11.28 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/24/19
PROJECT ENGINEER: Jordan T.	Page 1 of 2

PROJECT: Jefferson East CSR Works (Contract 5)	CLIENT: City of Winnipeg WWD	TESTHOLE NO: TH19-17
LOCATION: UTM 14 - 5533349 m N, 635414 m E		PROJECT NO.: 60599385
CONTRACTOR: Maple Leaf Drilling	METHOD: Acker MP-5 - 125 mm SSA	ELEVATION (m): 230.54
SAMPLE TYPE <input checked="" type="checkbox"/> GRAB <input type="checkbox"/> SHELBY TUBE <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> BULK <input checked="" type="checkbox"/> NO RECOVERY <input type="checkbox"/> CORE		

DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE #	PENETRATION TESTS	UNDRAINED SHEAR STRENGTH	COMMENTS	ELEVATION
10	▨				PENETRATION TESTS * Becker * ◇ Dynamic Cone ◇ ◆ SPT (Standard Pen Test) ◆ (Blows/300mm) ■ Total Unit Wt (kN/m ³) Plastic MC Liquid 20 40 60 80 100	UNDRAINED SHEAR STRENGTH + Torvane + X QU/2 X □ Lab Vane □ △ Pocket Pen. △ ● Field Vane ● (kPa) 50 100 150 200		220
11	▨			T164	16 17 18 19 20 21 20 40 60 80 100	50 100 150 200	Tube Recovery: 100%	219
12		END OF TEST HOLE AT 11.28 m IN CLAY Notes: 1. Seepage not observed during augering. 2. Sloughing not observed during augering. 3. Test hole backfilled with auger cuttings and bentonite upon completion.						218
13								217
14								216
15								215
16								214
17								213
18								212
19								211
20								210

LOG OF TEST HOLE 60599385 - TEST HOLE LOGS - ALL COMBINED.GPJ UMA WINN.GDT 11/28/19



LOGGED BY: Ryan Harras	COMPLETION DEPTH: 11.28 m
REVIEWED BY: Faris Alobaidy	COMPLETION DATE: 6/24/19
PROJECT ENGINEER: Jordan T.	Page 2 of 2

Appendix **D**

Laboratory Testing Reports

- D-1: AECOM (February 2012) Laboratory Testing Results
- D-2: AECOM (October 2015) Laboratory Testing Results
- D-3: AECOM (June 2019) Laboratory Testing Results

MOISTURE CONTENT

JOB No.: 60219315
 CLIENT: City of Winnipeg
 PROJECT: Jefferson East CSR

DATE: December 2011 - January 2012

HOLE NO.	TH11-01	-	-	-	-	-
SAMPLE NO.	G1	G2	G3	G4	G5	G6
DEPTH (FT)	5.0	10.0	15.0	20.0	25.0	30.0
MOISTURE CONTENT %	23.7	29.4	30.2	24.5	33.4	32.6
HOLE NO.	TH11-01	-	TH11-02	-	-	-
SAMPLE NO.	G7	G8	G9	G10	G11	G12
DEPTH (FT)	35.0	40.0	5.0	10.0	15.0	20.0
MOISTURE CONTENT %	33.7	34.6	28.2	16.7	24.9	28.6
HOLE NO.	TH11-02	-	-	-	TH11-03	-
SAMPLE NO.	G13	G14	G15	G16	G17	G18
DEPTH (FT)	25.0	30.0	35.0	40.0	5.0	10.0
MOISTURE CONTENT %	29.7	33.9	31.3	20.0	24.6	30.4
HOLE NO.	TH11-03	-	-	-	-	-
SAMPLE NO.	G19	G20	G21	G22	G23	G24
DEPTH (FT)	15.0	20.0	25.0	30.0	35.0	38.0
MOISTURE CONTENT %	32.1	37.2	34.5	35.1	30.1	17.9

NOTES:



MATERIALS LABORATORY
 AECOM
 99 Commerce Drive, Winnipeg, MB R3P 0Y7 Canada
 tel (204) 477-5381 fax (204) 284-2040

MOISTURE CONTENT

JOB No.: 60219315
 CLIENT: City of Winnipeg
 PROJECT: Jefferson East CSR

DATE: December 2011 - January 2012

HOLE NO.	TH11-03	TH11-05	-	-	-	-
SAMPLE NO.	G25	G26	G27	G28	G29	G30
DEPTH (FT)	40.0	5.0	10.0	15.0	20.0	25.0
MOISTURE CONTENT %	44.4	35.6	35.9	52.2	51.7	52.6
HOLE NO.	TH11-05	-	-	TH11-07	-	-
SAMPLE NO.	G31	G32	G33	G34	G35	G36
DEPTH (FT)	30.0	35.0	40.0	5.0	10.0	15.0
MOISTURE CONTENT %	52.9	58.4	54.7	21.5	40.0	54.6
HOLE NO.	TH11-07	-	-	-	-	TH11-06
SAMPLE NO.	G37	G38	G39	G40	G41	G42
DEPTH (FT)	20.0	25.0	30.0	35.0	40.0	5.0
MOISTURE CONTENT %	51.1	49.3	48.7	50.6	56.3	27.6
HOLE NO.	TH11-06	-	-	-	-	-
SAMPLE NO.	G43	G44	G45	G46	G47	G48
DEPTH (FT)	8.0	10.0	15.0	20.0	25.0	30.0
MOISTURE CONTENT %	22.0	33.3	58.4	58.0	53.8	50.1

NOTES:



MATERIALS LABORATORY
 AECOM
 99 Commerce Drive, Winnipeg, MB R3P 0Y7 Canada
 tel (204) 477-5381 fax (204) 284-2040

MOISTURE CONTENT

JOB No.: 60219315
 CLIENT: City of Winnipeg
 PROJECT: Jefferson East CSR

DATE: December 2011 - January 2012

HOLE NO.	TH11-06	-	TH11-04	-	-	-
SAMPLE NO.	G49	G50	G51	G52	G53	G54
DEPTH (FT)	35.0	40.0	5.0	10.0	15.0	20.0
MOISTURE CONTENT %	53.1	44.1	20.9	29.5	33.7	35.0
HOLE NO.	TH11-04	-	-	-	TH11-12	-
SAMPLE NO.	G55	G56	G57	G58	G59	G60
DEPTH (FT)	25.0	30.0	35.0	40.0	5.0	7.0
MOISTURE CONTENT %	37.6	28.7	24.6	48.9	24.9	22.0
HOLE NO.	TH11-12	-	-	-	-	-
SAMPLE NO.	G61	G62	G63	G64	G65	G66
DEPTH (FT)	10.0	15.0	20.0	25.0	30.0	35.0
MOISTURE CONTENT %	42.9	52.3	55.9	44.9	44.3	44.70
HOLE NO.	TH11-12	TH11-11	-	-	-	-
SAMPLE NO.	G67	G68	G69	G70	G71	G72
DEPTH (FT)	40.0	5.0	7.0	10.0	15.0	20.0
MOISTURE CONTENT %	35.6	31.2	24.0	39.5	56.0	57.5

NOTES:



MATERIALS LABORATORY
 AECOM

99 Commerce Drive, Winnipeg, MB R3P 0Y7 Canada
 tel (204) 477-5381 fax (204) 284-2040

MOISTURE CONTENT

JOB No.: 60219315
 CLIENT: City of Winnipeg
 PROJECT: Jefferson East CSR

DATE: December 2011 - January 2012

HOLE NO.	TH11-11	-	-	-	TH11-08	-
SAMPLE NO.	G73	G74	G75	G76	G77	G78
DEPTH (FT)	25.0	30.0	35.0	40.0	5.0	10.0
MOISTURE CONTENT %	43.7	50.9	50.7	46.0	29.0	33.6
HOLE NO.	TH11-08	-	-	-	-	-
SAMPLE NO.	G79	G80	G81	G82	G83	G84
DEPTH (FT)	15.0	20.0	25.0	30.0	35.0	40.0
MOISTURE CONTENT %	53.7	54.3	52.1	50.6	52.1	52.8
HOLE NO.	TH11-09	-	-	-	-	-
SAMPLE NO.	G85	G86	G87	G88	G89	G90
DEPTH (FT)	5.0	6.0	10.0	15.0	20.0	25.0
MOISTURE CONTENT %	31.4	25.6	52.6	52.5	47.3	43.3
HOLE NO.	TH11-09	-	-	TH11-10	-	-
SAMPLE NO.	G91	G92	G93	G94	G95	G96
DEPTH (FT)	30.0	35.0	40.0	1.0	7.0	10.0
MOISTURE CONTENT %	47.1	46.8	47.2	21.5	19.9	35.5

NOTES:



MATERIALS LABORATORY
 AECOM

99 Commerce Drive, Winnipeg, MB R3P 0Y7 Canada
 tel (204) 477-5381 fax (204) 284-2040

MOISTURE CONTENT

JOB No.: 60219315
 CLIENT: City of Winnipeg
 PROJECT: Jefferson East CSR

DATE: December 2011 - January 2012

HOLE NO.	TH11-10	-	-	-	-	-
SAMPLE NO.	G97	G98	G99	G100	G101	G102
DEPTH (FT)	15.0	20.0	25.0	30.0	35.0	40.0
MOISTURE CONTENT %	57.1	55.3	52.6	54.6	44.5	42.3
HOLE NO.	TH11-14	-	-	-	-	-
SAMPLE NO.	G103	G104	G105	G107	G108	G109
DEPTH (FT)	2.0	5.0	10.0	15.0	20.0	25.0
MOISTURE CONTENT %	8.1	44.2	43.8	38.0	38.7	31.5
HOLE NO.	TH11-14	-	-	-	-	TH11-13
SAMPLE NO.	G110	G111	G112	G113	G114	G115
DEPTH (FT)	30.0	35.0	40.0	45.0	50.0	5.0
MOISTURE CONTENT %	13.1	53.1	50.8	30.1	12.3	24.4
HOLE NO.	TH11-13	-	-	-	-	-
SAMPLE NO.	G116	G117	G119	G120	G121	G122
DEPTH (FT)	7.0	10.0	15.0	20.0	25.0	30.0
MOISTURE CONTENT %	11.7	40.4	53.7	56.1	47.0	49.8

NOTES:



MATERIALS LABORATORY

AECOM

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MOISTURE CONTENT

JOB No.: 60219315
 CLIENT: City of Winnipeg
 PROJECT: Jefferson East CSR

DATE: December 2011 - January 2012

HOLE NO.	TH11-13	-	-	-	-	-
SAMPLE NO.	G124	G125	G126	G127	G128	G129
DEPTH (FT)	35.0	40.0	45.0	50.0	55.0	58.5
MOISTURE CONTENT %	55.5	41.9	47.5	52.6	48.9	27.7
HOLE NO.	TH11-13	-				
SAMPLE NO.	G130	G131				
DEPTH (FT)	60.0	63.0				
MOISTURE CONTENT %	62.0	17.2				
HOLE NO.						
SAMPLE NO.						
DEPTH (FT)						
MOISTURE CONTENT %						
HOLE NO.						
SAMPLE NO.						
DEPTH (FT)						
MOISTURE CONTENT %						

NOTES:



MATERIALS LABORATORY
 AECOM
 99 Commerce Drive, Winnipeg, MB R3P 0Y7 Canada
 tel (204) 477-5381 fax (204) 284-2040

ATTERBERG
(ASTM D4318-98)



MATERIALS LABORATORY

AECOM

99 Commerce Dr., Winnipeg, MB R3P 0Y7 Canada

tel (204) 477-5381 fax (204) 284-2040

JOB No.: 60219315
 CLIENT: City of Winnipeg
 PROJECT: Jefferson East CSR
 LOCATION:

DATE: 4-Jan-12
 TEST HOLE: TH11-02
 SAMPLE: G12
 DEPTH: 20.0'
 TECH.: AL

Liquid Limit

WATER CONTENT

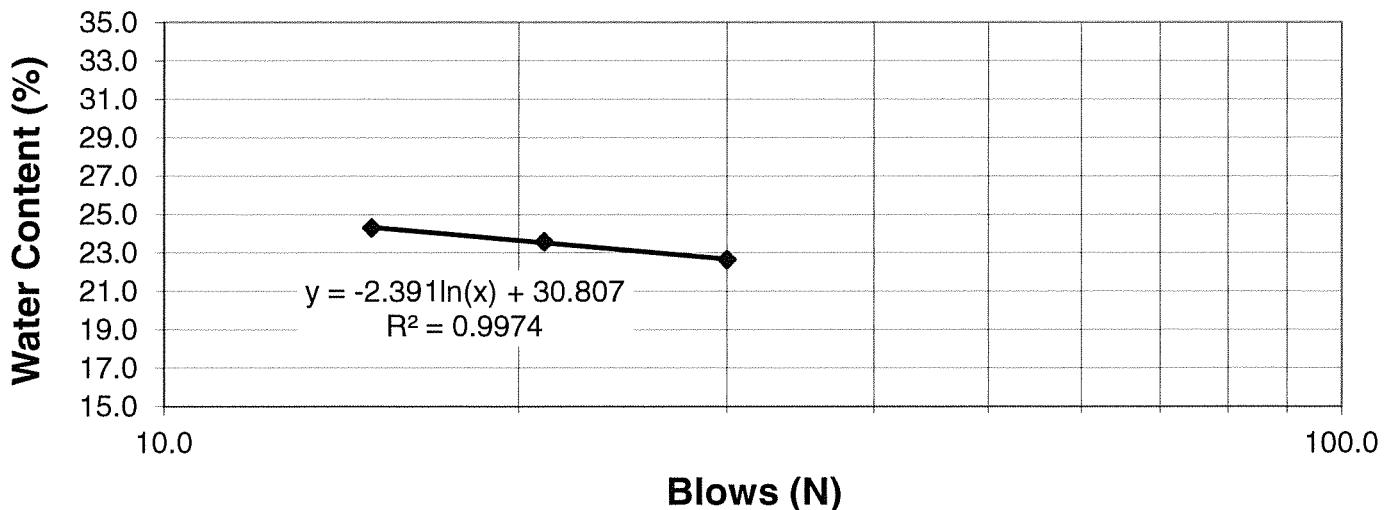
Blows	30	21	15		
WT. SAMPLE WET + TARE (gr)	104.020	105.633	108.950		
WT. SAMPLE DRY + TARE (gr)	99.491	101.590	104.305		
WT. TARE (gr)	79.496	84.441	85.195		
WT. WATER (gr)	4.529	4.043	4.645		
WT. DRY SOIL (gr)	19.995	17.149	19.110		
MOISTURE CONTENT (%)	22.651	23.576	24.307		

Plastic Limit

WATER CONTENT

WT. SAMPLE WET + TARE (gr)	92.370	86.663			
WT. SAMPLE DRY + TARE (gr)	91.580	85.797			
WT. TARE (gr)	86.387	80.119			
WT. WATER (gr)	0.790	0.866			
WT. DRY SOIL (gr)	5.193	5.678			
MOISTURE CONTENT (%)	15.213	15.252			

LIQUID LIMIT



Liquid Limit = 23.1

Plastic Limit = 15.2

Plasticity Index = 7.9

ATTERBERG
(ASTM D4318-98)



MATERIALS LABORATORY

AECOM

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tel (204) 477-5381 fax (204) 284-2040

JOB No.: 60219315
 CLIENT: City of Winnipeg
 PROJECT: Jefferson East CSR
 LOCATION:

DATE: 4-Jan-12
 TEST HOLE: TH11-07
 SAMPLE: G34
 DEPTH: 5.0'
 TECH.: AL

Liquid Limit

WATER CONTENT

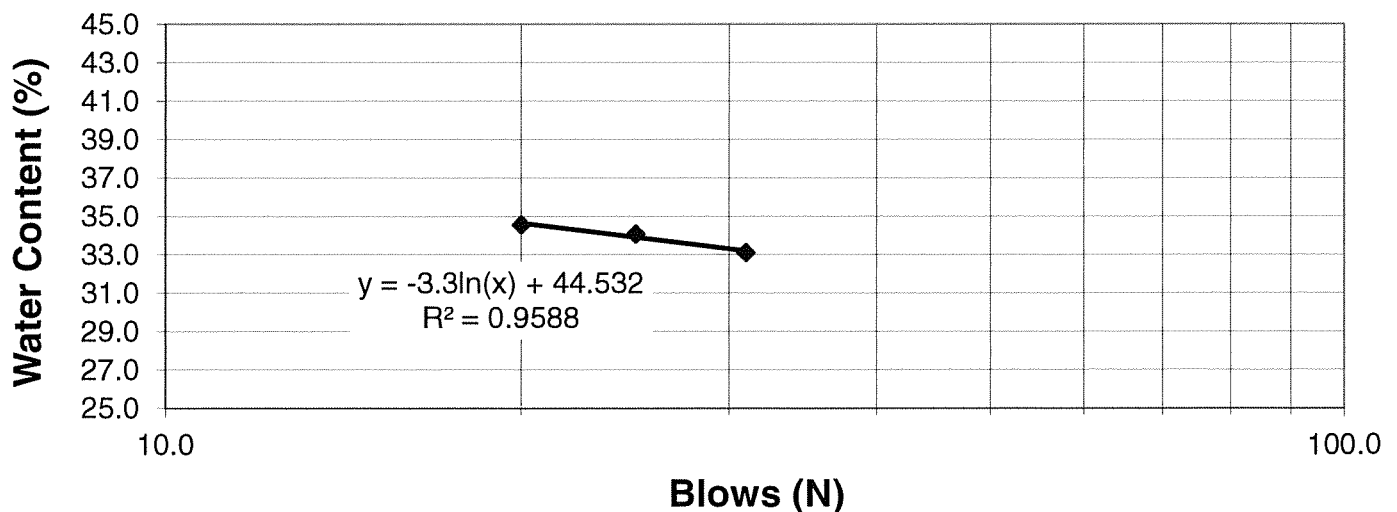
Blows	31	25	20		
WT. SAMPLE WET + TARE (gr)	99.544	98.151	92.131		
WT. SAMPLE DRY + TARE (gr)	96.267	94.541	89.127		
WT. TARE (gr)	86.370	83.949	80.435		
WT. WATER (gr)	3.277	3.610	3.004		
WT. DRY SOIL (gr)	9.897	10.592	8.692		
MOISTURE CONTENT (%)	33.111	34.082	34.561		

Plastic Limit

WATER CONTENT

WT. SAMPLE WET + TARE (gr)	92.603	92.822			
WT. SAMPLE DRY + TARE (gr)	91.608	91.874			
WT. TARE (gr)	85.821	86.365			
WT. WATER (gr)	0.995	0.948			
WT. DRY SOIL (gr)	5.787	5.509			
MOISTURE CONTENT (%)	17.194	17.208			

LIQUID LIMIT



Liquid Limit = 33.9

Plastic Limit = 17.2

Plasticity Index = 16.7

ATTERBERG
(ASTM D4318-98)



MATERIALS LABORATORY

AECOM

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tel (204) 477-5381 fax (204) 284-2040

JOB No.: 60219315
 CLIENT: City of Winnipeg
 PROJECT: Jefferson East CSR
 LOCATION:

DATE: 4-Jan-12
 TEST HOLE: TH11-13
 SAMPLE: T118
 DEPTH: 10.0 - 12.0'
 TECH.: AL

Liquid Limit

WATER CONTENT

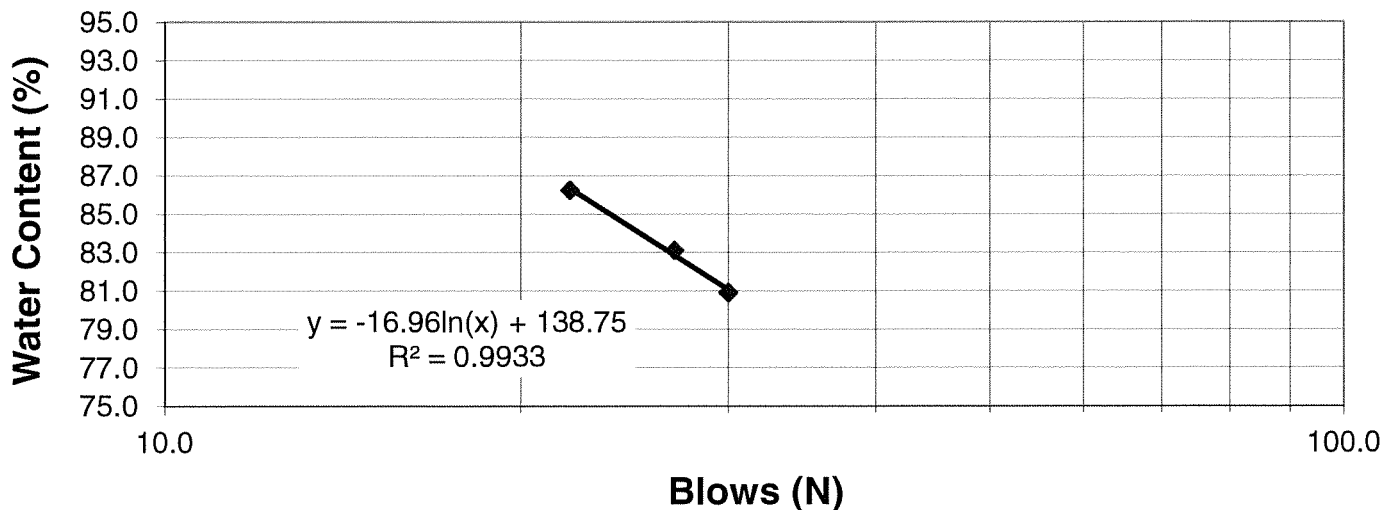
Blows	30	27	22		
WT. SAMPLE WET + TARE (gr)	93.527	92.186	93.261		
WT. SAMPLE DRY + TARE (gr)	88.267	86.724	88.061		
WT. TARE (gr)	81.766	80.152	82.032		
WT. WATER (gr)	5.260	5.462	5.200		
WT. DRY SOIL (gr)	6.501	6.572	6.029		
MOISTURE CONTENT (%)	80.911	83.110	86.250		

Plastic Limit

WATER CONTENT

WT. SAMPLE WET + TARE (gr)	84.944	83.483		
WT. SAMPLE DRY + TARE (gr)	83.829	82.010		
WT. TARE (gr)	80.237	77.286		
WT. WATER (gr)	1.115	1.473		
WT. DRY SOIL (gr)	3.592	4.724		
MOISTURE CONTENT (%)	31.041	31.181		

LIQUID LIMIT



Liquid Limit = 84.2

Plastic Limit = 31.1

Plasticity Index = 53.0

ATTERBERG
(ASTM D4318-98)



MATERIALS LABORATORY

AECOM

99 Commerce Dr., Winnipeg, MB R3P 0Y7 Canada

tel (204) 477-5381 fax (204) 284-2040

JOB No.: 60219315
 CLIENT: City of Winnipeg
 PROJECT: Jefferson East CSR
 LOCATION:

DATE: 4-Jan-12
 TEST HOLE: TH11-14
 SAMPLE: T106
 DEPTH: 10.0 - 12.0'
 TECH.: AL

Liquid Limit

WATER CONTENT

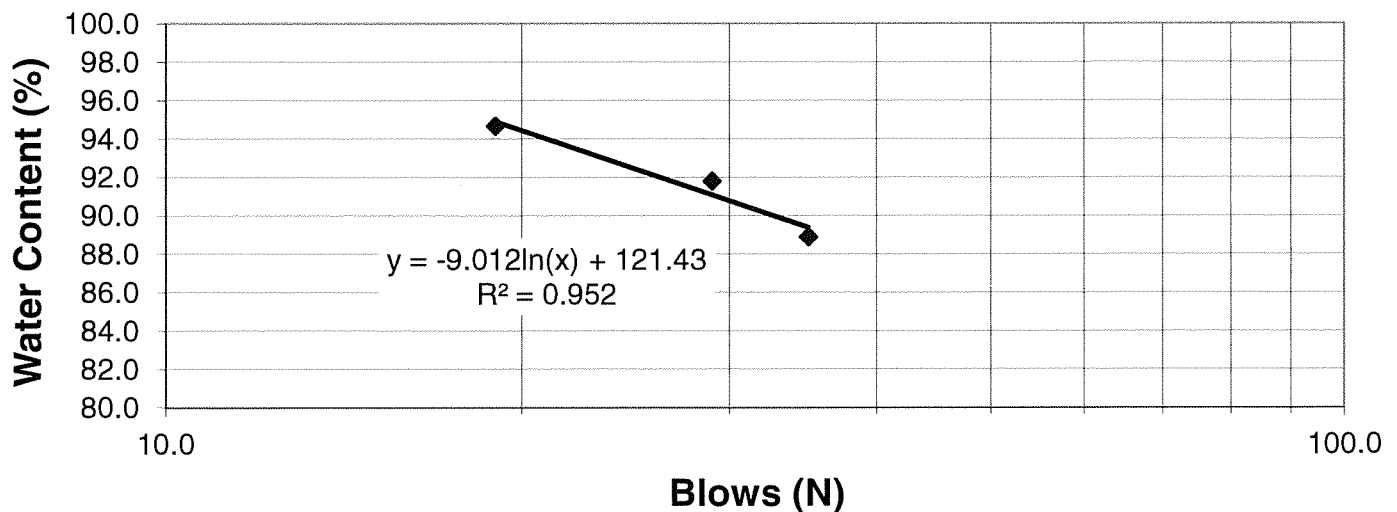
Blows	35	29	19		
WT. SAMPLE WET + TARE (gr)	91.139	91.936	92.988		
WT. SAMPLE DRY + TARE (gr)	86.440	86.284	87.441		
WT. TARE (gr)	81.154	80.127	81.582		
WT. WATER (gr)	4.699	5.652	5.547		
WT. DRY SOIL (gr)	5.286	6.157	5.859		
MOISTURE CONTENT (%)	88.895	91.798	94.675		

Plastic Limit

WATER CONTENT

WT. SAMPLE WET + TARE (gr)	88.930	91.154			
WT. SAMPLE DRY + TARE (gr)	87.957	89.995			
WT. TARE (gr)	84.775	86.208			
WT. WATER (gr)	0.973	1.159			
WT. DRY SOIL (gr)	3.182	3.787			
MOISTURE CONTENT (%)	30.578	30.605			

LIQUID LIMIT



Liquid Limit = 92.4

Plastic Limit = 30.6

Plasticity Index = 61.8

GRAIN SIZE DISTRIBUTION



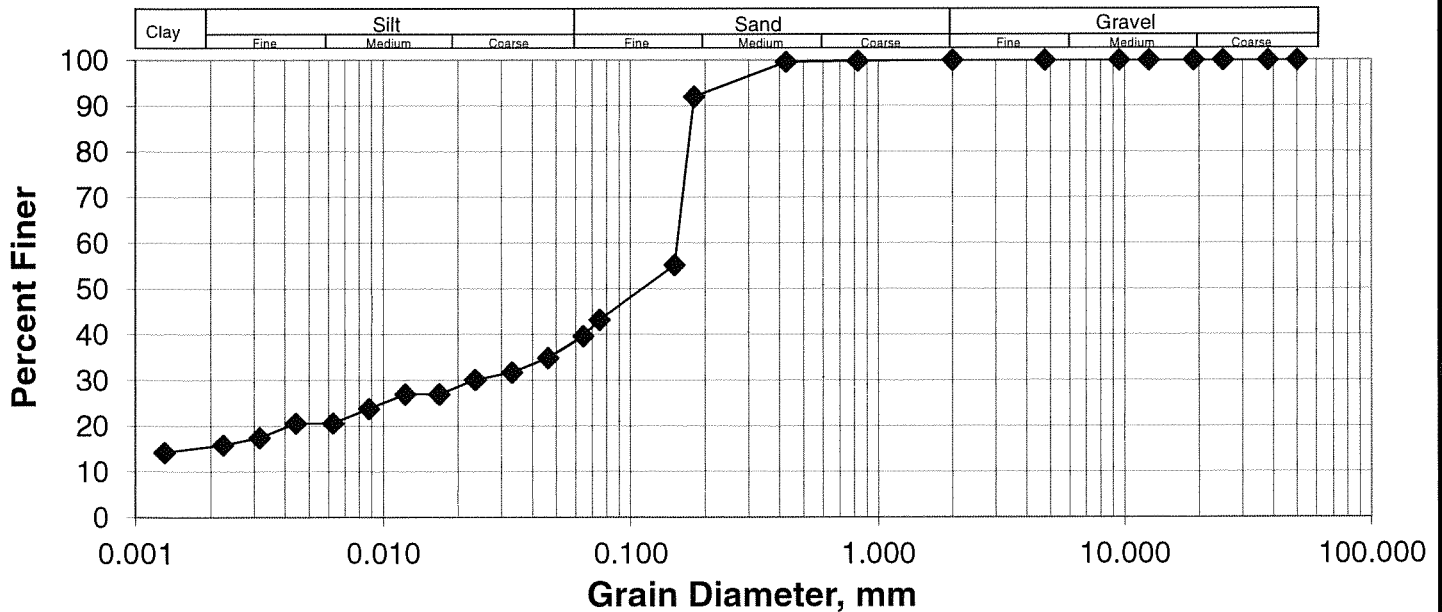
MATERIALS LABORATORY
 AECOM
 99 Commerce Dr., Winnipeg, MB R3P 0Y7 Canada
 tel (204) 477-5381 fax (204) 284-2040

Job No.: 60219315
 Client: City of Winnipeg
 Project: Jefferson East CSR
 Date Tested: 4-Jan-12
 Tested By: _____

Hole No.: TH11-02
 Sample No.: G12
 Depth: 20.0'
 Date Sampled: _____
 Sampled By: _____

GRAVEL SIZES		SAND SIZES		FINES	
Grain Size (mm.)	Total Percent Passing	Grain Size (mm.)	Total Percent Passing	Grain Size (mm.)	Total Percent Passing
50.0	100.0	2.00	100.0	0.0750	43.2
38.0	100.0	0.83	99.8	0.0644	39.6
25.0	100.0	0.43	99.6	0.0463	34.9
19.0	100.0	0.18	92.0	0.0331	31.7
12.5	100.0	0.15	55.2	0.0235	30.1
9.5	100.0	0.075	43.2	0.0168	26.9
4.75	100.0			0.0123	26.9
2.00	100.0			0.0088	23.8
				0.0063	20.6
				0.0044	20.6
				0.0032	17.4
				0.0022	15.8
				0.0013	14.2

GRAIN SIZE DISTRIBUTION CURVE



Gravel	0.0%	Silt	23.1%
Sand	61.5%	Clay	15.4%

** Note: Soil Classification based on Grain Size from Canadian Foundation Engineering Manual, 3rd edition (1992).

GRAIN SIZE DISTRIBUTION



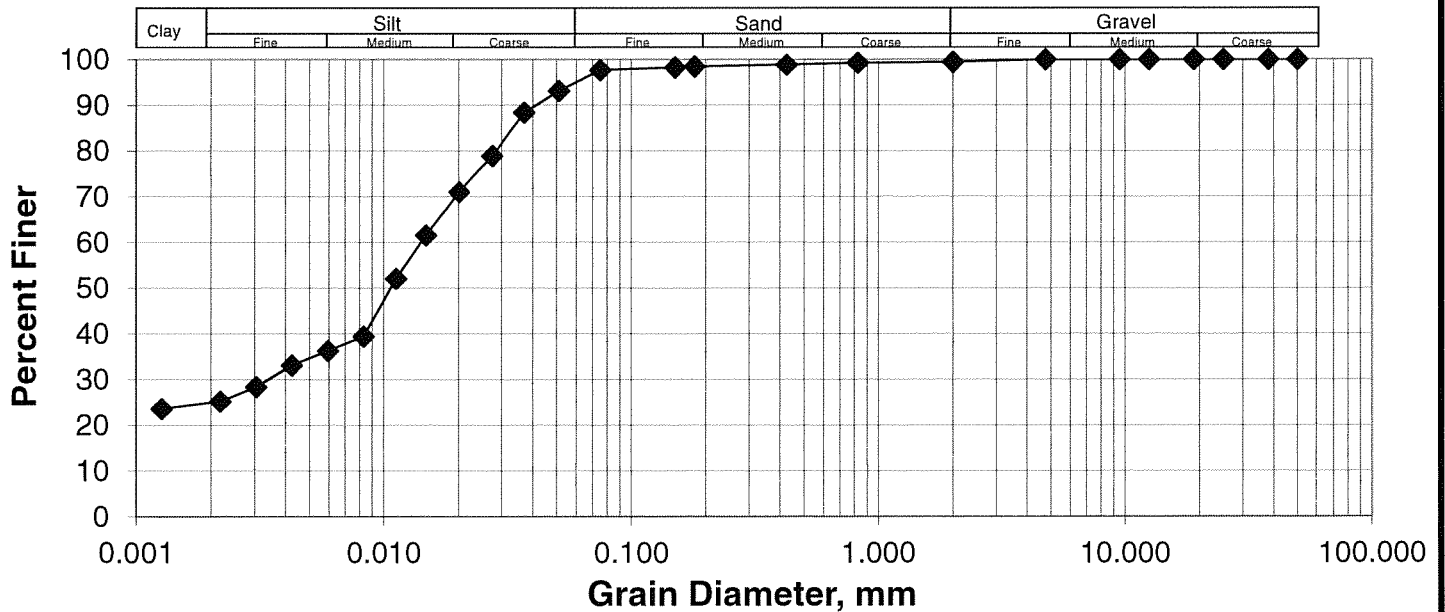
MATERIALS LABORATORY
 AECOM
 99 Commerce Dr., Winnipeg, MB R3P 0Y7 Canada
 tel (204) 477-5381 fax (204) 284-2040

Job No.: 60219315
 Client: City of Winnipeg
 Project: Jefferson East CSR
 Date Tested: 4-Jan-12
 Tested By: _____

Hole No.: TH11-07
 Sample No.: G34
 Depth: 5.0'
 Date Sampled: _____
 Sampled By: _____

GRAVEL SIZES		SAND SIZES		FINES	
Grain Size (mm.)	Total Percent Passing	Grain Size (mm.)	Total Percent Passing	Grain Size (mm.)	Total Percent Passing
50.0	100.0	2.00	99.4	0.0750	97.7
38.0	100.0	0.83	99.2	0.0510	93.1
25.0	100.0	0.43	98.8	0.0370	88.4
19.0	100.0	0.18	98.4	0.0274	78.9
12.5	100.0	0.15	98.2	0.0201	71.0
9.5	100.0	0.075	97.7	0.0148	61.5
4.75	100.0			0.0112	52.1
2.00	99.4			0.0083	39.4
				0.0059	36.3
				0.0042	33.1
				0.0030	28.4
				0.0022	25.2
				0.0013	23.6

GRAIN SIZE DISTRIBUTION CURVE



Gravel	0.6%	Silt	69.9%
Sand	4.6%	Clay	24.9%

** Note: Soil Classification based on Grain Size from Canadian Foundation Engineering Manual, 3rd edition (1992).

GRAIN SIZE DISTRIBUTION



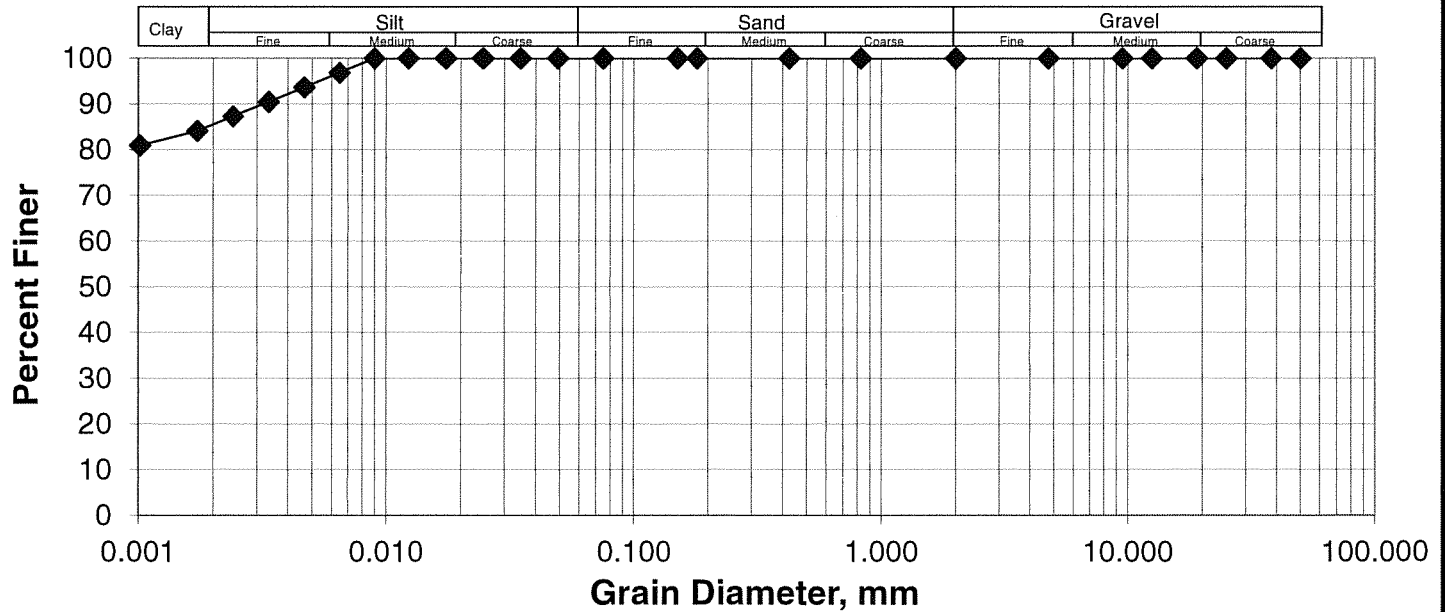
MATERIALS LABORATORY
 AECOM
 99 Commerce Dr., Winnipeg, MB R3P 0Y7 Canada
 tel (204) 477-5381 fax (204) 284-2040

Job No.: 60219315
 Client: City of Winnipeg
 Project: Jefferson East CSR
 Date Tested: 4-Jan-12
 Tested By: _____

Hole No.: TH11-13
 Sample No.: T118
 Depth: 10.0 - 12.0'
 Date Sampled: _____
 Sampled By: _____

GRAVEL SIZES		SAND SIZES		FINES	
Grain Size (mm.)	Total Percent Passing	Grain Size (mm.)	Total Percent Passing	Grain Size (mm.)	Total Percent Passing
50.0	100.0	2.00	100.0	0.0750	100.0
38.0	100.0	0.83	100.0	0.0491	100.0
25.0	100.0	0.43	100.0	0.0347	100.0
19.0	100.0	0.18	100.0	0.0246	100.0
12.5	100.0	0.15	100.0	0.0174	100.0
9.5	100.0	0.075	100.0	0.0123	100.0
4.75	100.0			0.0090	100.0
2.00	100.0			0.0065	96.8
				0.0047	93.6
				0.0033	90.5
				0.0024	87.3
				0.0017	84.1
				0.0010	80.9

GRAIN SIZE DISTRIBUTION CURVE



Gravel	0.0%	Silt	14.6%
Sand	0.0%	Clay	85.4%

** Note: Soil Classification based on Grain Size from Canadian Foundation Engineering Manual, 3rd edition (1992).

GRAIN SIZE DISTRIBUTION



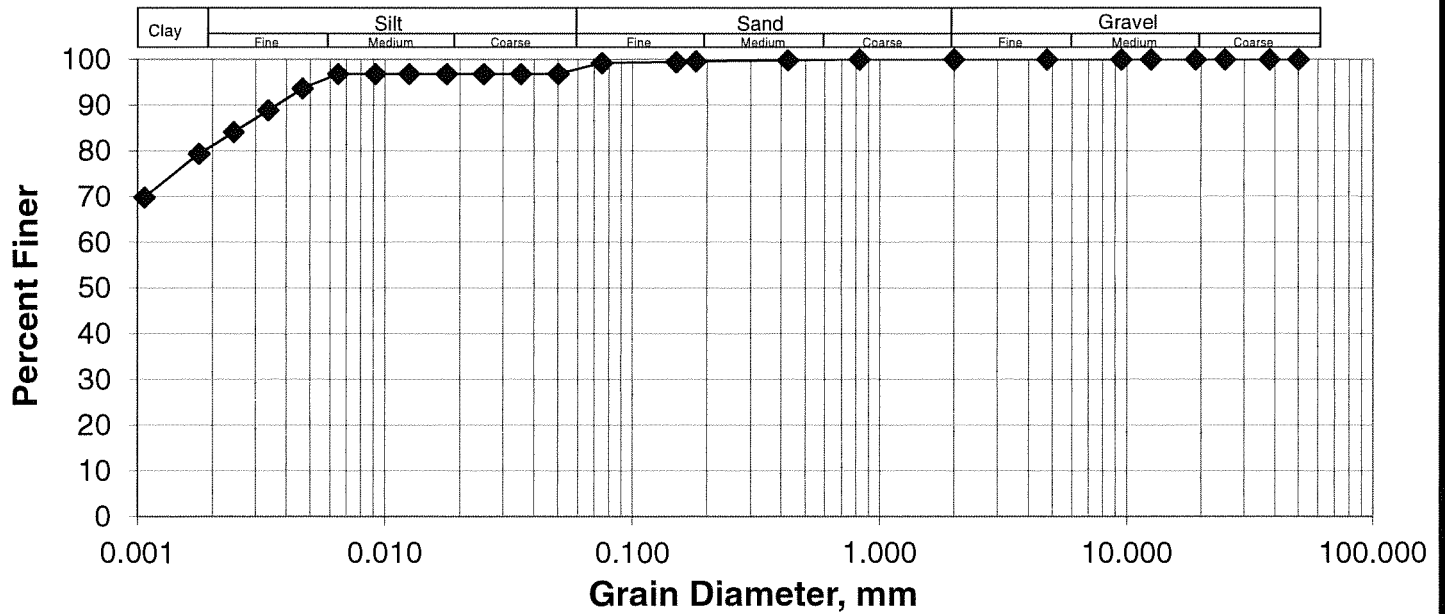
MATERIALS LABORATORY
 AECOM
 99 Commerce Dr., Winnipeg, MB R3P 0Y7 Canada
 tel (204) 477-5381 fax (204) 284-2040

Job No.: 60219315
 Client: City of Winnipeg
 Project: Jefferson East CSR
 Date Tested: 4-Jan-12
 Tested By: _____

Hole No.: TH11-14
 Sample No.: T106
 Depth: 10.0 - 12.0'
 Date Sampled: _____
 Sampled By: _____

GRAVEL SIZES		SAND SIZES		FINES	
Grain Size (mm.)	Total Percent Passing	Grain Size (mm.)	Total Percent Passing	Grain Size (mm.)	Total Percent Passing
50.0	100.0	2.00	100.0	0.0750	99.2
38.0	100.0	0.83	100.0	0.0500	96.8
25.0	100.0	0.43	99.8	0.0354	96.8
19.0	100.0	0.18	99.6	0.0250	96.8
12.5	100.0	0.15	99.4	0.0177	96.8
9.5	100.0	0.075	99.2	0.0125	96.8
4.75	100.0			0.0091	96.8
2.00	100.0			0.0065	96.8
				0.0047	93.6
				0.0034	88.9
				0.0024	84.1
				0.0018	79.3
				0.0011	69.8

GRAIN SIZE DISTRIBUTION CURVE



Gravel	0.0%	Silt	16.8%
Sand	2.2%	Clay	81.0%

** Note: Soil Classification based on Grain Size from Canadian Foundation Engineering Manual, 3rd edition (1992).

AECOM - SOILS LABORATORY
SHEAR STRENGTH, MOISTURE CONTENT & DENSITY CALCULATIONS



CLIENT: City of Winnipeg
PROJECT: Jefferson East CSR
JOB NO.: 60219315

TEST HOLE NO.:	TH11-13		
SAMPLE NO.:	T118		
SAMPLE DEPTH:	10.0 - 12.0'		
DATE TESTED:	2-Jan-12		
SHEAR STRENGTH TESTS			
LAB VANE			
Reading	Su 1		
Spring Number	4		
Undrained Shear Strength (kPa) =	0.0		
Undrained Shear Strength (ksf) =	0.00		
TORVANE			
Reading	0.70		
Vane Size (S, M, L)	m		
Undrained Shear Strength (kPa) =	68.7		
Undrained Shear Strength (ksf) =	1.43		
POCKET PENETROMETER			
Reading - Qu (tsf)	2.25		
Undrained Shear Strength (kPa) =	107.7		
Reading - Qu (tsf)	2.25		
Undrained Shear Strength (kPa) =	107.7		
Reading - Qu (tsf)	2.50		
Undrained Shear Strength (kPa) =	119.7		
UNCONFINED COMPRESSIVE STRENGTH TEST			
Unconfined compressive strength (kPa) =	114.8		
Unconfined compressive strength (ksf) =	2.4		
Undrained Shear Strength (kPa) =	57.4		
Undrained Shear Strength (ksf) =	1.199		
Density - Su1			
MOISTURE CONTENT			
Tare Number	A19		
Wt. Sample wet + tare (g)	375.0		
Wt. Sample dry + tare (g)	262.3		
Wt. Tare (g)	8.2		
Moisture Content, w% =	44.4		
BULK DENSITY			
Sample Wt. (g)	1040.6		
Diameter 1 (cm)	7.25		
Diameter 2 (cm)	7.18		
Diameter 3 (cm)	7.21		
Avg. Diameter (cm)	7.21		
Length 1 (cm)	14.48		
Length 2 (cm)	14.45		
Length 3 (cm)	14.42		
Avg. Length (cm)	14.45		
Volume (cm ³)	590.5		
Moisture content (%)	44.4		
Bulk Density (g/cm ³)	1.762		
Bulk Density (kN/m³)	17.3		
Bulk Density (pcf)	110.0		
Drv Density (kN/m³)	11.97		

AECOM - SOILS LABORATORY
UNCONFINED COMPRESSIVE STRENGTH OF COHESIVE SOILS (ASTM D2166)



CLIENT:	City of Winnipeg
PROJECT:	Jefferson East CSR
JOB NO.:	60219315

TEST HOLE NO.:	TH11-13
SAMPLE NO.:	T118
SAMPLE DEPTH:	10.0 - 12.0'
SAMPLE DATE:	14-Dec-11
TEST DATE:	2-Jan-12

SOIL DESCRIPTION:	
CLAY; trace silt, light brown, moist, firm, high plasticity, blocky	
MOISTURE CONTENT: 44.4	



FAILURE SKETCH

SAMPLE DIAM.(Do):	72.13	(mm)	INITIAL AREA, A _o :	4086.6	(mm ²)
SAMPLE LENGTH, (L _o):	144.50	(mm)	PISTON RATE:	0.051	(inches / minute)
L / D RATIO:	2.0	(2 < L/D < 2.5)	AXIAL STRAIN RATE, R:	0.90	(0.5 < R < 2 % / minute)

TEST DATA - DIAL READINGS		TOTAL AXIAL STRAIN, E _t	AVERAGE CROSS-SECTIONAL AREA, A	APPLIED AXIAL LOAD, P	COMPRESSIVE STRESS, σ _c		
AXIAL COMPRESSION	PROVING RING				(psi)	(ksf)	(kPa)
(inches)	(inches)	(%)	(inches ²)	(lbs)			
0.01	0.0009	0.00	6.33	8.43	1.33	0.192	9.2
0.02	0.0014	0.15	6.34	13.12	2.07	0.298	14.3
0.03	0.0019	0.30	6.35	17.80	2.80	0.404	19.3
0.03	0.0026	0.45	6.36	24.36	3.83	0.551	26.4
0.04	0.0033	0.60	6.37	30.92	4.85	0.699	33.5
0.05	0.0041	0.75	6.38	38.42	6.02	0.867	41.5
0.06	0.0048	0.90	6.39	44.98	7.04	1.013	48.5
0.07	0.0055	1.05	6.40	51.54	8.05	1.159	55.5
0.08	0.0060	1.20	6.41	56.22	8.77	1.263	60.5
0.09	0.0065	1.34	6.42	60.91	9.49	1.366	65.4
0.09	0.0070	1.49	6.43	65.59	10.20	1.469	70.3
0.10	0.0074	1.64	6.44	69.34	10.77	1.550	74.2
0.11	0.0079	1.79	6.45	74.02	11.48	1.653	79.1
0.12	0.0083	1.94	6.46	77.77	12.04	1.734	83.0
0.13	0.0087	2.09	6.47	81.52	12.60	1.814	86.9
0.14	0.0090	2.24	6.48	84.33	13.01	1.874	89.7
0.14	0.0094	2.39	6.49	88.08	13.57	1.954	93.6
0.15	0.0096	2.54	6.50	89.95	13.84	1.993	95.4
0.16	0.0099	2.69	6.51	92.76	14.25	2.052	98.3
0.17	0.0102	2.84	6.52	95.57	14.66	2.111	101.1
0.18	0.0104	2.99	6.53	97.45	14.92	2.149	102.9
0.19	0.0106	3.14	6.54	99.32	15.19	2.187	104.7
0.20	0.0108	3.29	6.55	101.20	15.45	2.225	106.5
0.20	0.0110	3.44	6.56	103.07	15.71	2.263	108.3
0.21	0.0112	3.59	6.57	104.94	15.97	2.300	110.1
0.22	0.0113	3.74	6.58	105.88	16.09	2.317	110.9
0.23	0.0114	3.88	6.59	106.82	16.21	2.334	111.8
0.24	0.0115	4.03	6.60	107.76	16.33	2.351	112.6
0.25	0.0116	4.18	6.61	108.69	16.44	2.368	113.4
0.26	0.0117	4.33	6.62	109.63	16.56	2.384	114.2
0.26	0.0117	4.48	6.63	109.63	16.53	2.381	114.0
0.27	0.0118	4.63	6.64	110.57	16.65	2.397	114.8
0.28	0.0118	4.78	6.65	110.57	16.62	2.393	114.6
0.29	0.0118	4.93	6.66	110.57	16.59	2.390	114.4
0.30	0.0118	5.08	6.67	110.57	16.57	2.386	114.2
0.31	0.0117	5.23	6.68	109.63	16.40	2.362	113.1
0.31	0.0117	5.38	6.69	109.63	16.38	2.358	112.9
0.32	0.0116	5.53	6.70	108.69	16.21	2.334	111.8

UNCONFINED COMPRESSIVE STRENGTH, q _u : (based on maximum q _u value)	114.78	kPa
	2.397	ksf
UNDRAINED SHEAR STRENGTH, S _u : (based on maximum q _u value)	57.39	kPa
	1.199	ksf

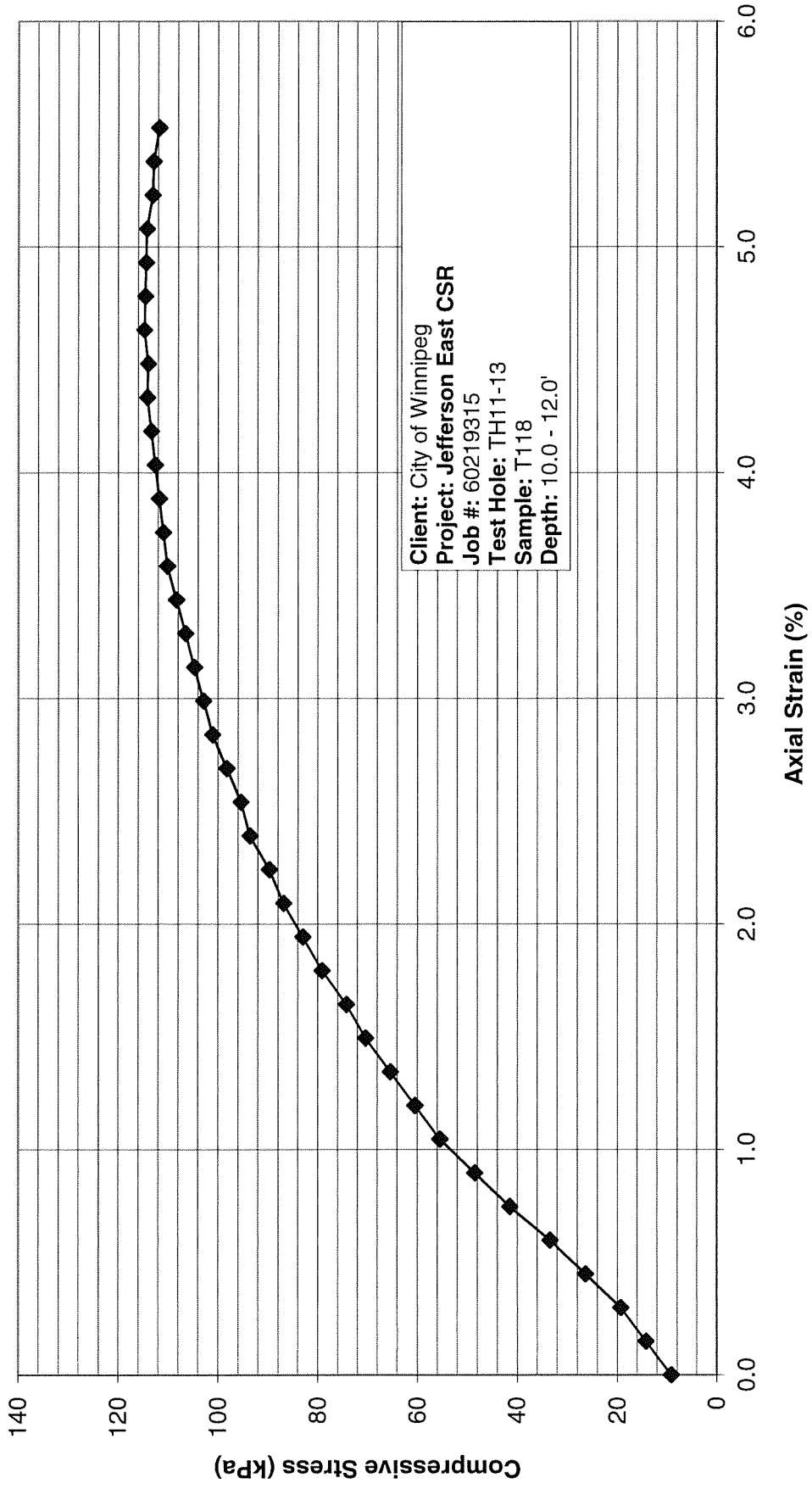
NOTES:

REMARKS:

AECOM

AECOM

UNCONFINED COMPRESSIVE STRENGTH OF COHESIVE SOILS
(ASTM D2166)



AECOM - SOILS LABORATORY
SHEAR STRENGTH, MOISTURE CONTENT & DENSITY CALCULATIONS



CLIENT: City of Winnipeg
 PROJECT: Jefferson East CSR
 JOB NO.: 60219315

TEST HOLE NO.:	TH11-13
SAMPLE NO.:	T123
SAMPLE DEPTH:	30.0 32.0'
DATE TESTED:	2-Jan-12
SHEAR STRENGTH TESTS	
LAB VANE	Su 1
Reading	
Spring Number	4
Undrained Shear Strength (kPa) =	0.0
Undrained Shear Strength (ksf) =	0.00
TORVANE	
Reading	0.45
Vane Size (S, M, L)	m
Undrained Shear Strength (kPa) =	44.1
Undrained Shear Strength (ksf) =	0.92
POCKET PENETROMETER	
Reading - Qu (tsf)	0.75
Undrained Shear Strength (kPa) =	35.9
Reading - Qu (tsf)	0.75
Undrained Shear Strength (kPa) =	35.9
Reading - Qu (tsf)	0.75
Undrained Shear Strength (kPa) =	35.9
UNCONFINED COMPRESSIVE STRENGTH TEST	
Unconfined compressive strength (kPa) =	106.3
Unconfined compressive strength (ksf) =	2.2
Undrained Shear Strength (kPa) =	53.1
Undrained Shear Strength (ksf) =	1.110
MOISTURE CONTENT	
Tare Number	75
Wt. Sample wet + tare (g)	327.4
Wt. Sample dry + tare (g)	225.1
Wt. Tare (g)	8.3
Moisture Content, w% =	47.2
BULK DENSITY	
Sample Wt. (g)	1095.0
Diameter 1 (cm)	7.22
Diameter 2 (cm)	7.22
Diameter 3 (cm)	7.24
Avg. Diameter (cm)	7.23
Length 1 (cm)	15.32
Length 2 (cm)	15.35
Length 3 (cm)	15.32
Avg. Length (cm)	15.33
Volume (cm ³)	628.8
Moisture content (%)	47.2
Bulk Density (g/cm ³)	1.741
Bulk Density (kN/m³)	17.1
Bulk Density (pcf)	108.7
Dry Density (kN/m³)	11.60

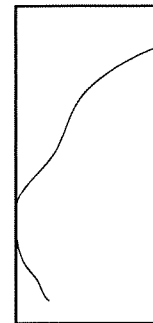
AECOM - SOILS LABORATORY
UNCONFINED COMPRESSIVE STRENGTH OF COHESIVE SOILS (ASTM D2166)



CLIENT:	City of Winnipeg
PROJECT:	Jefferson East CSR
JOB NO.:	60219315

TEST HOLE NO.:	TH11-13
SAMPLE NO.:	T123
SAMPLE DEPTH:	30.0 32.0'
SAMPLE DATE:	14-Dec-11
TEST DATE:	2-Jan-12

SOIL DESCRIPTION:	
CLAY: some silt inclusions (< 5mm), grey, moist, firm, high plasticity, slickensided	
MOISTURE CONTENT:	47.2



FAILURE SKETCH

SAMPLE DIAM.(D _o):	72.27	(mm)	INITIAL AREA, A _o :	4101.7	(mm ²)
SAMPLE LENGTH, (L _o):	153.30	(mm)	PISTON RATE:	0.051	(inches / minute)
L / D RATIO:	2.1	(2 < L/D < 2.5)	AXIAL STRAIN RATE, R:	0.85	(0.5 < R < 2 % / minute)

TEST DATA - DIAL READINGS		TOTAL AXIAL STRAIN, E _t	AVERAGE CROSS-SECTIONAL AREA, A	APPLIED AXIAL LOAD, P	COMPRESSIVE STRESS, σ _c		
AXIAL COMPRESSION	PROVING RING				(psi)	(ksf)	(kPa)
(inches)	(inches)	(%)	(inches ²)	(lbs)			
0.01	0.0008	0.00	6.36	7.50	1.18	0.170	8.1
0.02	0.0021	0.14	6.37	19.68	3.09	0.445	21.3
0.03	0.0034	0.28	6.38	31.86	5.00	0.720	34.5
0.03	0.0044	0.42	6.38	41.23	6.46	0.930	44.5
0.04	0.0053	0.56	6.39	49.66	7.77	1.118	53.6
0.05	0.0061	0.70	6.40	57.16	8.93	1.285	61.5
0.06	0.0069	0.85	6.41	64.65	10.08	1.452	69.5
0.07	0.0077	0.99	6.42	72.15	11.24	1.618	77.5
0.08	0.0083	1.13	6.43	77.77	12.09	1.742	83.4
0.09	0.0090	1.27	6.44	84.33	13.10	1.886	90.3
0.09	0.0096	1.41	6.45	89.95	13.95	2.009	96.2
0.10	0.0099	1.55	6.46	92.76	14.36	2.069	99.0
0.11	0.0102	1.69	6.47	95.57	14.78	2.128	101.9
0.12	0.0104	1.83	6.48	97.45	15.05	2.167	103.7
0.13	0.0105	1.97	6.49	98.39	15.17	2.184	104.6
0.14	0.0106	2.11	6.49	99.32	15.29	2.202	105.4
0.14	0.0107	2.25	6.50	100.25	15.41	2.220	106.3
0.15	0.0107	2.39	6.51	100.26	15.39	2.216	106.1
0.16	0.0107	2.54	6.52	100.26	15.37	2.213	106.0
0.17	0.0105	2.68	6.53	98.39	15.06	2.169	103.8
0.18	0.0101	2.82	6.54	94.64	14.47	2.083	99.7

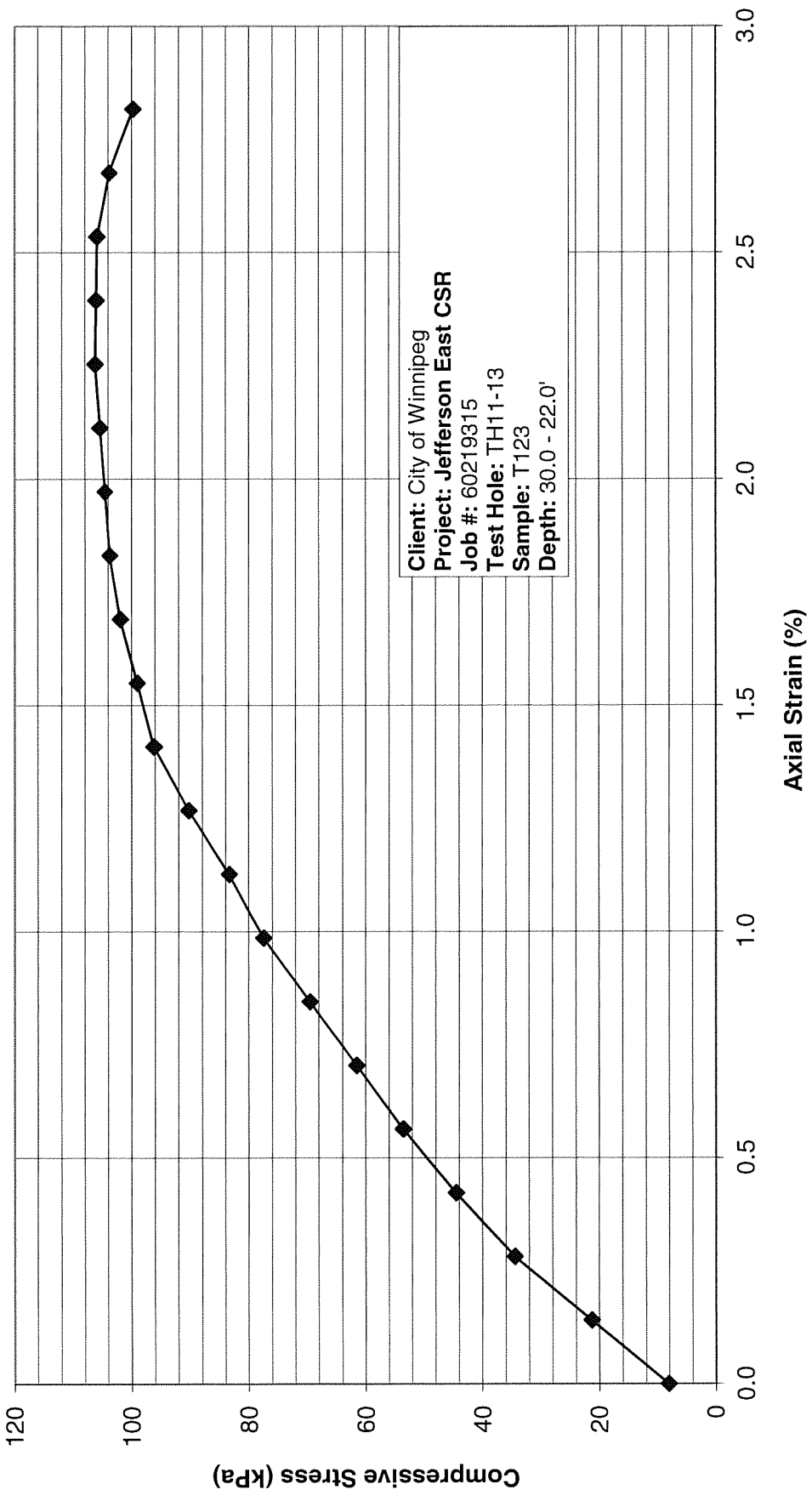
UNCONFINED COMPRESSIVE STRENGTH, q _u :	106.28	kPa
(based on maximum q _u value)	2.220	ksf
UNDRAINED SHEAR STRENGTH, S _u :	53.14	kPa
(based on maximum q _u value)	1.110	ksf

NOTES:

REMARKS:

AECOM
UNCONFINED COMPRESSIVE STRENGTH OF COHESIVE SOILS
(ASTM D2166)

AECOM



Client: City of Winnipeg
Project: Jefferson East CSR
Job #: 60219315
Test Hole: TH11-13
Sample: T123
Depth: 30.0 - 22.0'

AECOM - SOILS LABORATORY
SHEAR STRENGTH, MOISTURE CONTENT & DENSITY CALCULATIONS



CLIENT: City of Winnipeg
 PROJECT: Jefferson East CSR
 JOB NO.: 60219315

TEST HOLE NO.:	TH11-14
SAMPLE NO.:	T106
SAMPLE DEPTH:	10.0 - 12.0'
DATE TESTED:	2-Jan-12
SHEAR STRENGTH TESTS	
LAB VANE	Su 1
Reading	
Spring Number	4
Undrained Shear Strength (kPa) =	0.0
Undrained Shear Strength (ksf) =	0.00
TORVANE	
Reading	0.95
Vane Size (S, M, L)	m
Undrained Shear Strength (kPa) =	93.2
Undrained Shear Strength (ksf) =	1.95
POCKET PENETROMETER	
Reading - Qu (tsf)	2.00
Undrained Shear Strength (kPa) =	95.8
Reading - Qu (tsf)	2.00
Undrained Shear Strength (kPa) =	95.8
Reading - Qu (tsf)	2.00
Undrained Shear Strength (kPa) =	95.8
UNCONFINED COMPRESSIVE STRENGTH TEST	
Unconfined compressive strength (kPa) =	185.6
Unconfined compressive strength (ksf) =	3.9
Undrained Shear Strength (kPa) =	92.8
Undrained Shear Strength (ksf) =	1.938
MOISTURE CONTENT	Density - Su1
Tare Number	14
Wt. Sample wet + tare (g)	375.3
Wt. Sample dry + tare (g)	264.1
Wt. Tare (g)	8.1
Moisture Content, w% =	43.4
BULK DENSITY	
Sample Wt. (g)	1099
Diameter 1 (cm)	7.24
Diameter 2 (cm)	7.20
Diameter 3 (cm)	7.23
Avg. Diameter (cm)	7.22
Length 1 (cm)	15.36
Length 2 (cm)	15.36
Length 3 (cm)	15.30
Avg. Length (cm)	15.34
Volume (cm ³)	628.6
Moisture content (%)	43.4
Bulk Density (g/cm ³)	1.748
Bulk Density (kN/m³)	17.1
Bulk Density (pcf)	109.1
Dry Density (kN/m³)	11.95

AECOM - SOILS LABORATORY
UNCONFINED COMPRESSIVE STRENGTH OF COHESIVE SOILS (ASTM D2166)



CLIENT:	City of Winnipeg
PROJECT:	Jefferson East CSR
JOB NO.:	60219315

TEST HOLE NO.:	TH11-14
SAMPLE NO.:	T106
SAMPLE DEPTH:	10.0 - 12.0'
SAMPLE DATE:	14-Dec-11
TEST DATE:	2-Jan-12

SOIL DESCRIPTION:	
CLAY: trace silt inclusions (<5mm), light brown, moist, firm, intermediate to high plasticity, slickensided	
MOISTURE CONTENT:	43.4



FAILURE SKETCH

SAMPLE DIAM.(Do):	72.23	(mm)	INITIAL AREA, A _o :	4097.9	(mm ²)
SAMPLE LENGTH, (Lo):	153.40	(mm)	PISTON RATE:	0.051	(inches / minute)
L / D RATIO:	2.1	(2 < L/D < 2.5)	AXIAL STRAIN RATE, R:	0.84	(0.5<R<2 % / minute)

TEST DATA - DIAL READINGS		TOTAL AXIAL STRAIN, E _t	AVERAGE CROSS-SECTIONAL AREA, A	APPLIED AXIAL LOAD, P	COMPRESSIVE STRESS, σ _c		
AXIAL COMPRESSION	PROVING RING				(psi)	(ksf)	(kPa)
(inches)	(inches)	(%)	(inches ²)	(lbs)			
0.01	0.0009	0.00	6.35	8.43	1.33	0.191	9.2
0.02	0.0019	0.14	6.36	17.80	2.80	0.403	19.3
0.03	0.0028	0.28	6.37	26.24	4.12	0.593	28.4
0.03	0.0038	0.42	6.38	35.61	5.58	0.804	38.5
0.04	0.0049	0.56	6.39	45.91	7.19	1.035	49.6
0.05	0.0060	0.70	6.40	56.22	8.79	1.266	60.6
0.06	0.0071	0.84	6.41	66.53	10.39	1.495	71.6
0.07	0.0083	0.99	6.42	77.77	12.12	1.746	83.6
0.08	0.0093	1.13	6.42	87.14	13.56	1.953	93.5
0.09	0.0102	1.27	6.43	95.57	14.86	2.139	102.4
0.09	0.0113	1.41	6.44	105.88	16.43	2.367	113.3
0.10	0.0122	1.55	6.45	114.31	17.72	2.551	122.2
0.11	0.0131	1.69	6.46	122.75	19.00	2.736	131.0
0.12	0.0140	1.83	6.47	131.18	20.27	2.920	139.8
0.13	0.0148	1.97	6.48	138.68	21.40	3.082	147.6
0.14	0.0157	2.11	6.49	147.11	22.67	3.265	156.3
0.14	0.0164	2.25	6.50	153.67	23.65	3.405	163.0
0.15	0.0172	2.39	6.51	161.16	24.77	3.566	170.8
0.16	0.0177	2.53	6.52	165.85	25.45	3.665	175.5
0.17	0.0182	2.67	6.53	170.53	26.13	3.763	180.2
0.18	0.0187	2.81	6.54	175.22	26.81	3.861	184.8
0.19	0.0188	2.96	6.55	176.16	26.91	3.876	185.6
0.20	0.0188	3.10	6.55	176.16	26.87	3.870	185.3
0.20	0.0183	3.24	6.56	171.47	26.12	3.762	180.1

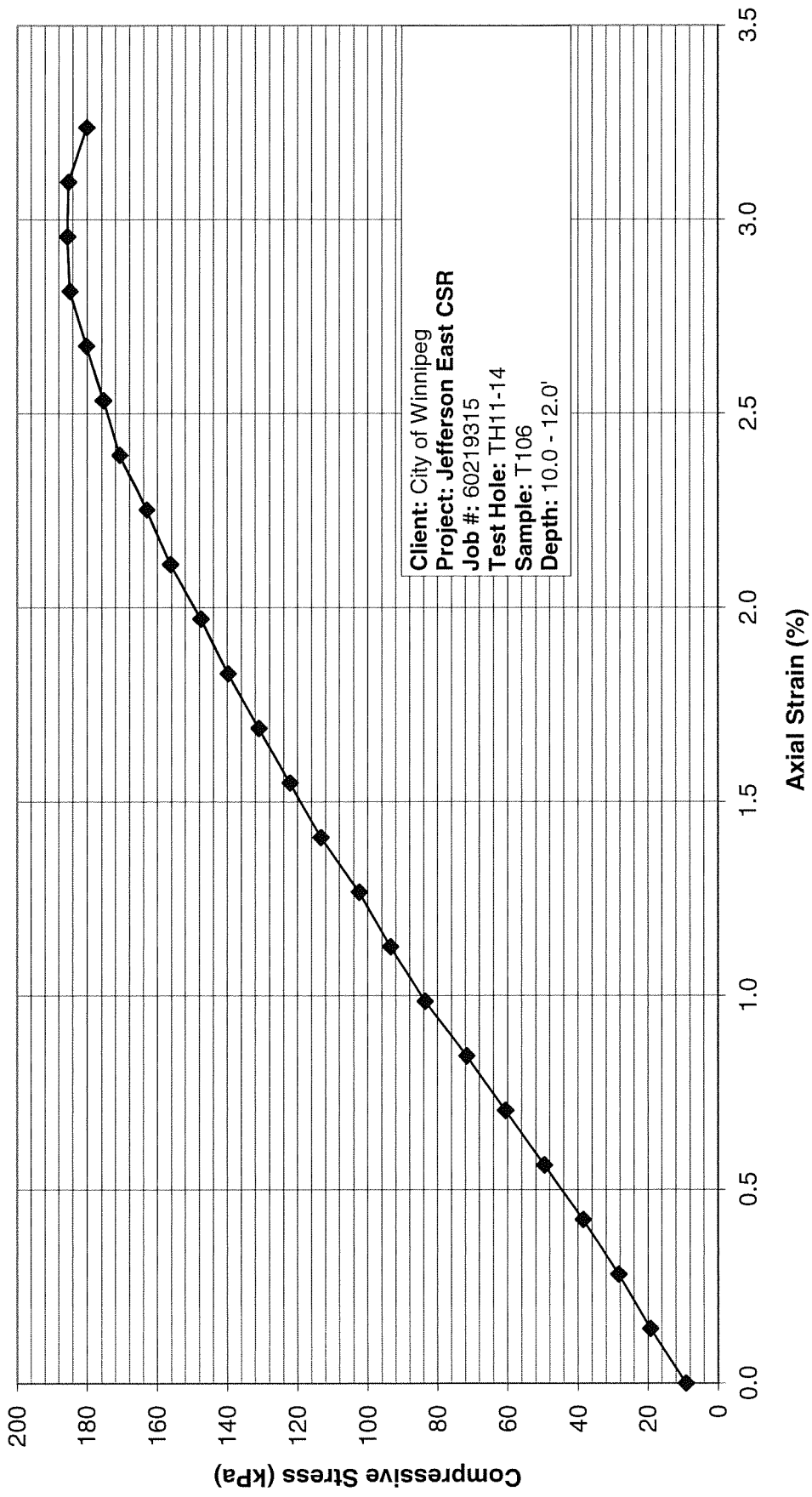
UNCONFINED COMPRESSIVE STRENGTH, q _u :	185.56	kPa
(based on maximum q _u value)	3.876	ksf
UNDRAINED SHEAR STRENGTH, S _u :	92.78	kPa
(based on maximum q _u value)	1.938	ksf

NOTES:

REMARKS:

AECOM
UNCONFINED COMPRESSIVE STRENGTH OF COHESIVE SOILS
(ASTM D2166)

AECOM





AECOM Canada Ltd.
 Winnipeg Geotechnical Laboratory
 99 Commerce Drive
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 Phone: 204 477 5381 Fax: 204 284 2040

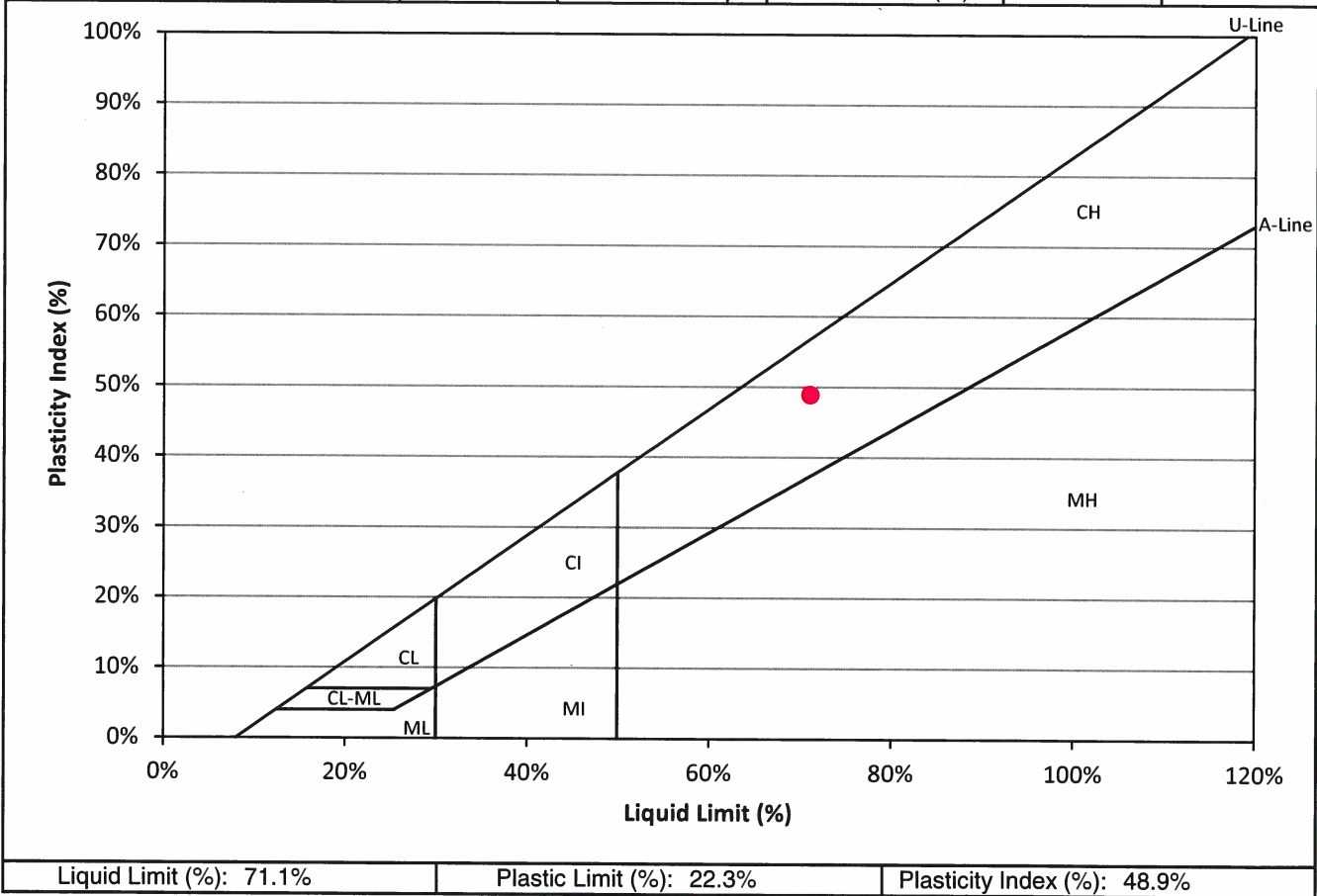
Project Name: Jefferson East CSR
 Project Number: 60219315
 Client: City of Winnipeg
 Sample Location: TH15-01
 Sample Depth: 0.30 - 0.46 m
 Sample Number: G1

Supplier: AECOM
 Specification: N/A
 Field Technician: MAlkiki
 Sample Date: February 24, 2015
 Lab Technician: EManimbao
 Date Tested: April 1, 2015

Atterberg Limits

ASTM D4318: Standard Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils

Liquid Limit				Plastic Limit		
Blows	35	23	18	Trial	1	2
Wet Sample (g)	7.5	7.9	9.5	Wet Sample (g)	8.1	9.4
Dry Sample (g)	4.5	4.6	5.5	Dry Sample (g)	6.7	7.7
Water Content (%)	69.1%	71.8%	72.8%	Water Content (%)	21.8%	22.7%





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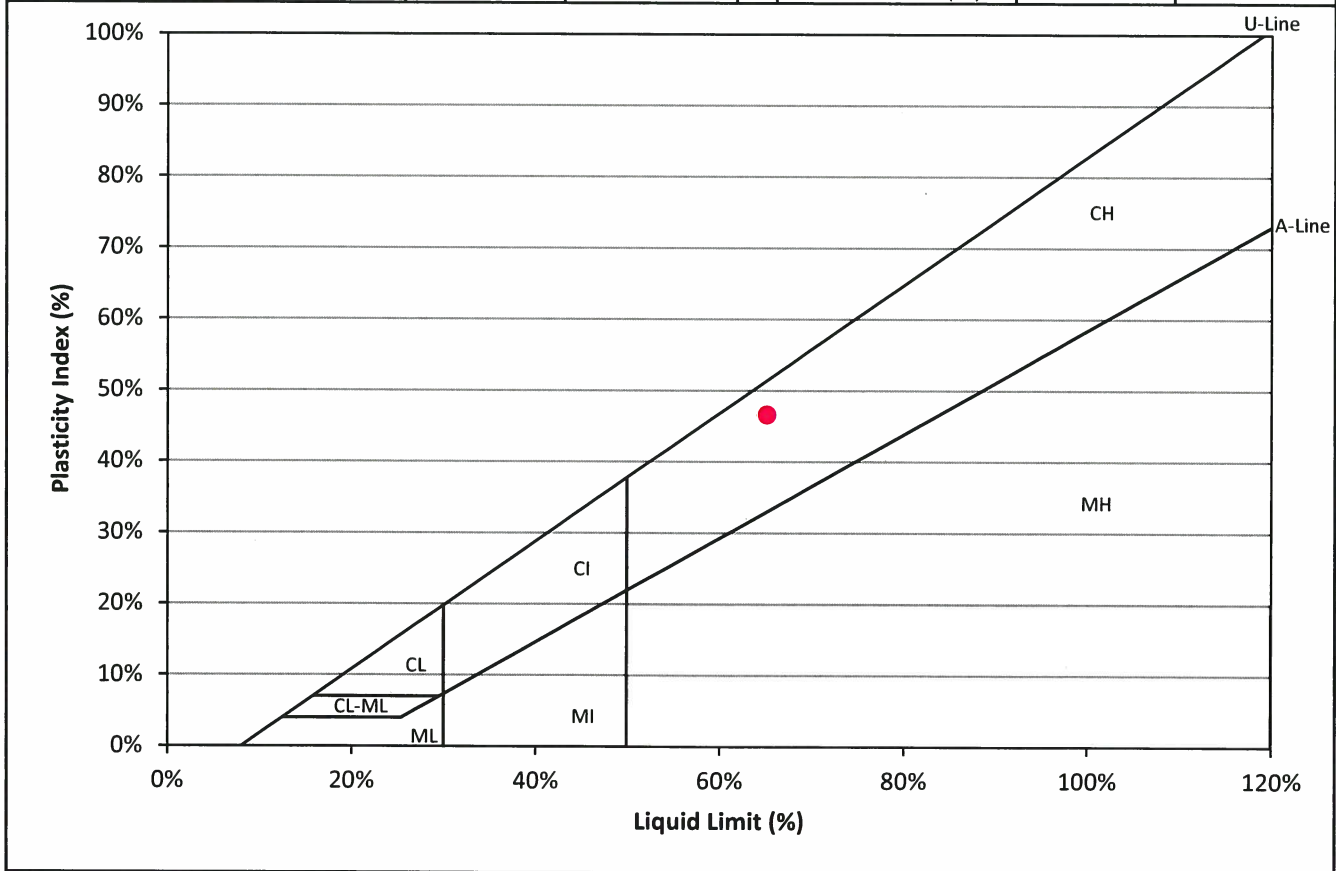
Project Name: Jefferson East CSR
 Project Number: 60219315
 Client: City of Winnipeg
 Sample Location: TH15-01
 Sample Depth: 1.22 - 1.37 m
 Sample Number: G3

Supplier: AECOM
 Specification: N/A
 Field Technician: MAlkiki
 Sample Date: February 24, 2015
 Lab Technician: EManimbao
 Date Tested: April 1, 2015

Atterberg Limits

ASTM D4318: Standard Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils

Liquid Limit				Plastic Limit		
Blows	35	26	21	Trial	1	2
Wet Sample (g)	8.4	8.0	7.5	Wet Sample (g)	9.9	9.7
Dry Sample (g)	5.2	4.8	4.5	Dry Sample (g)	8.4	8.1
Water Content (%)	62.0%	65.3%	66.5%	Water Content (%)	18.5%	18.7%



Liquid Limit (%): 65.1%	Plastic Limit (%): 18.6%	Plasticity Index (%): 46.6%
-------------------------	--------------------------	-----------------------------



AECOM Canada Ltd.
 Winnipeg Geotechnical Laboratory
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Project Name: Jefferson East CSR
 Project Number: 60219315
 Client: City of Winnipeg
 Sample Location: TH15-01
 Sample Depth: 4.57 - 5.18 m
 Sample Number: T11

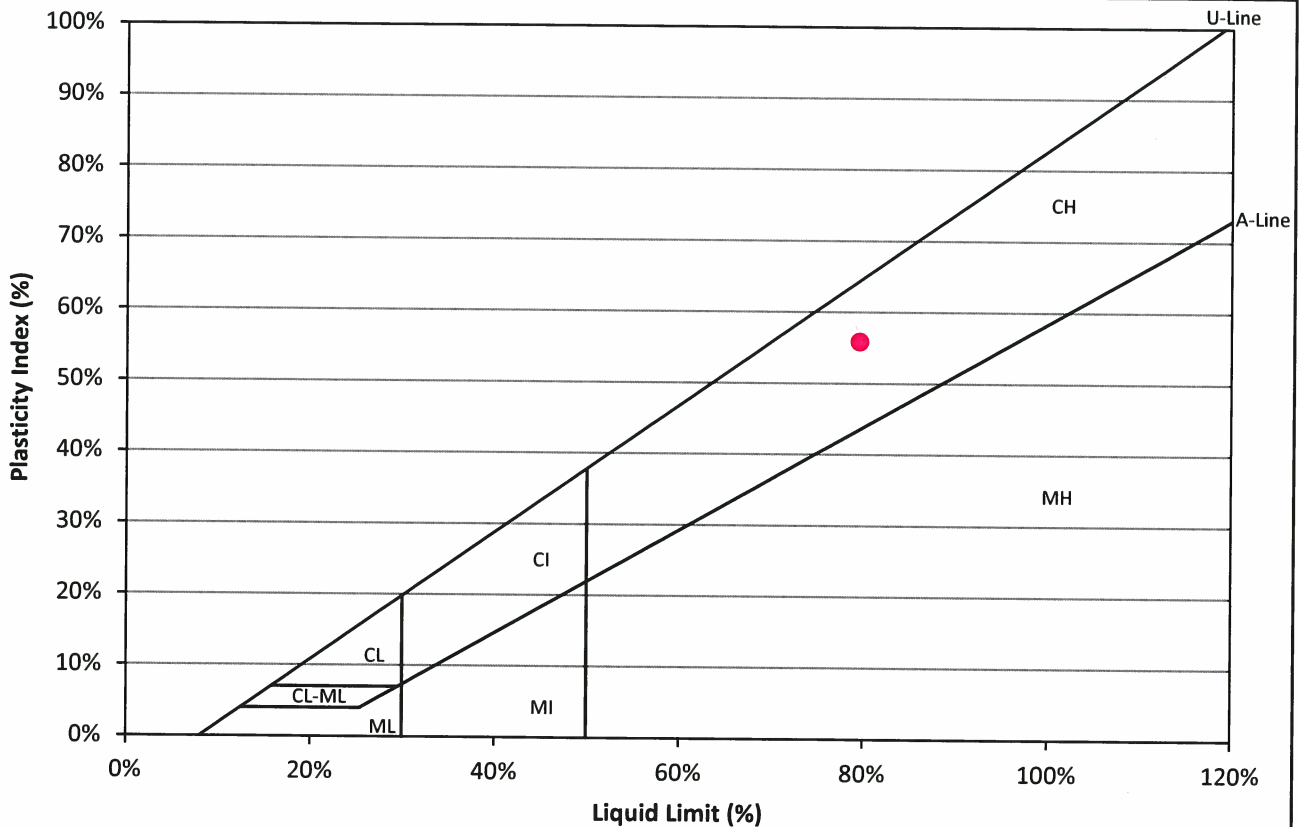
Supplier: AECOM
 Specification: N/A
 Field Technician: MAIkiki
 Sample Date: February 24, 2015
 Lab Technician: EManimbao
 Date Tested: April 2, 2015

Atterberg Limits

ASTM D4318: Standard Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils

Liquid Limit			
Blows	17	26	35
Wet Sample (g)	11.6	11.7	9.9
Dry Sample (g)	6.4	6.5	5.6
Water Content (%)	82.1%	79.4%	77.0%

Plastic Limit		
Trial	1	2
Wet Sample (g)	6.9	6.8
Dry Sample (g)	5.6	5.5
Water Content (%)	23.8%	23.7%



Liquid Limit (%): 79.6%

Plastic Limit (%): 23.8%

Plasticity Index (%): 55.9%

GRAIN SIZE DISTRIBUTION

(ASTM D422-63)



MATERIALS LABORATORY

AECOM

99 Commerce Dr., Winnipeg, MB R3P 0Y7 Canada

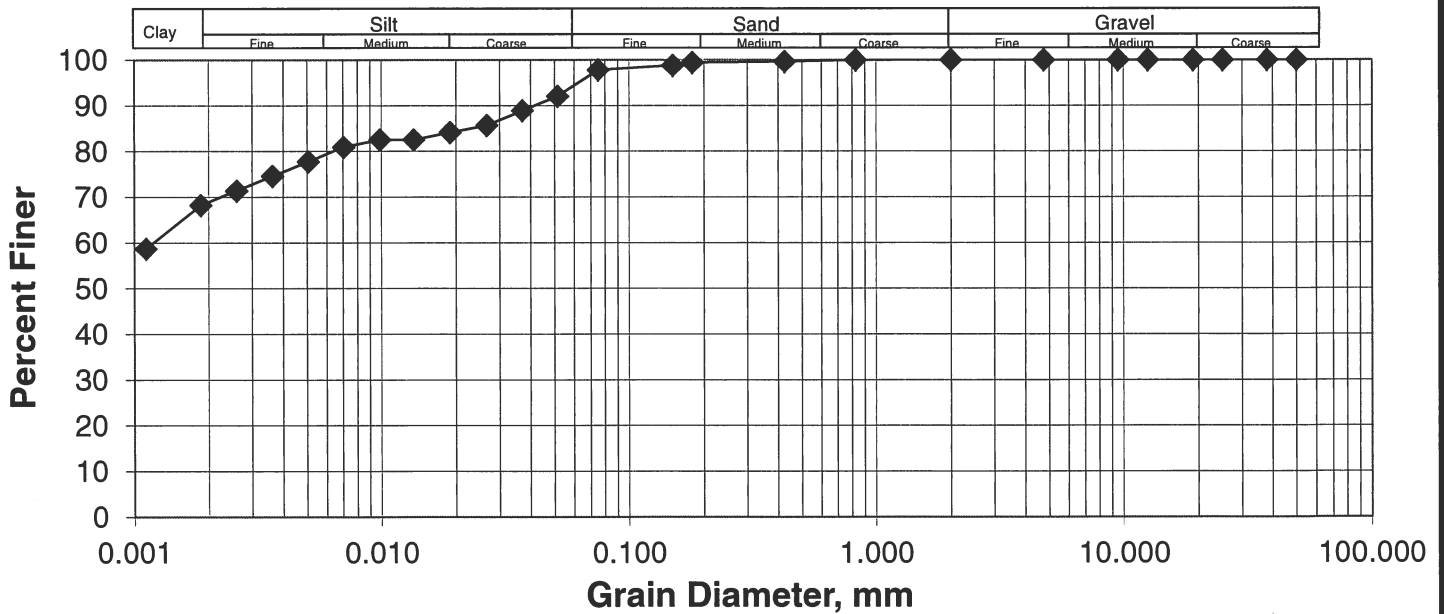
tel (204) 477-5381 fax (204) 284-2040

Job No.: 60219315
 Client: City of Winnipeg
 Project: Jefferson East CSR
 Date Tested: 2-Apr-15
 Tested By: MLotecki

Hole No.: TH15-01
 Sample No.: G1
 Depth: 0.30 - 0.46 m
 Date Sampled: 24-Feb-15
 Sampled By: AECOM

GRAVEL SIZES		SAND SIZES		FINES	
Grain Size (mm.)	Total Percent Passing	Grain Size (mm.)	Total Percent Passing	Grain Size (mm.)	Total Percent Passing
50.0	100.0	2.00	100.0	0.0750	97.8
38.0	100.0	0.83	100.0	0.0514	92.1
25.0	100.0	0.43	99.6	0.0370	88.9
19.0	100.0	0.18	99.4	0.0266	85.7
12.5	100.0	0.15	98.8	0.0189	84.1
9.5	100.0	0.075	97.8	0.0135	82.5
4.75	100.0			0.0099	82.5
2.00	100.0			0.0070	80.9
				0.0050	77.8
				0.0036	74.6
				0.0026	71.4
				0.0019	68.2
				0.0011	58.7

GRAIN SIZE DISTRIBUTION CURVE



Gravel	0.0%	Silt	25.3%
Sand	5.9%	Clay	68.8%

** Note: Soil Classification based on Grain Size from Canadian Foundation Engineering Manual, 3rd edition (1992).

GRAIN SIZE DISTRIBUTION

(ASTM D422-63)



MATERIALS LABORATORY

AECOM

99 Commerce Dr., Winnipeg, MB R3P 0Y7 Canada

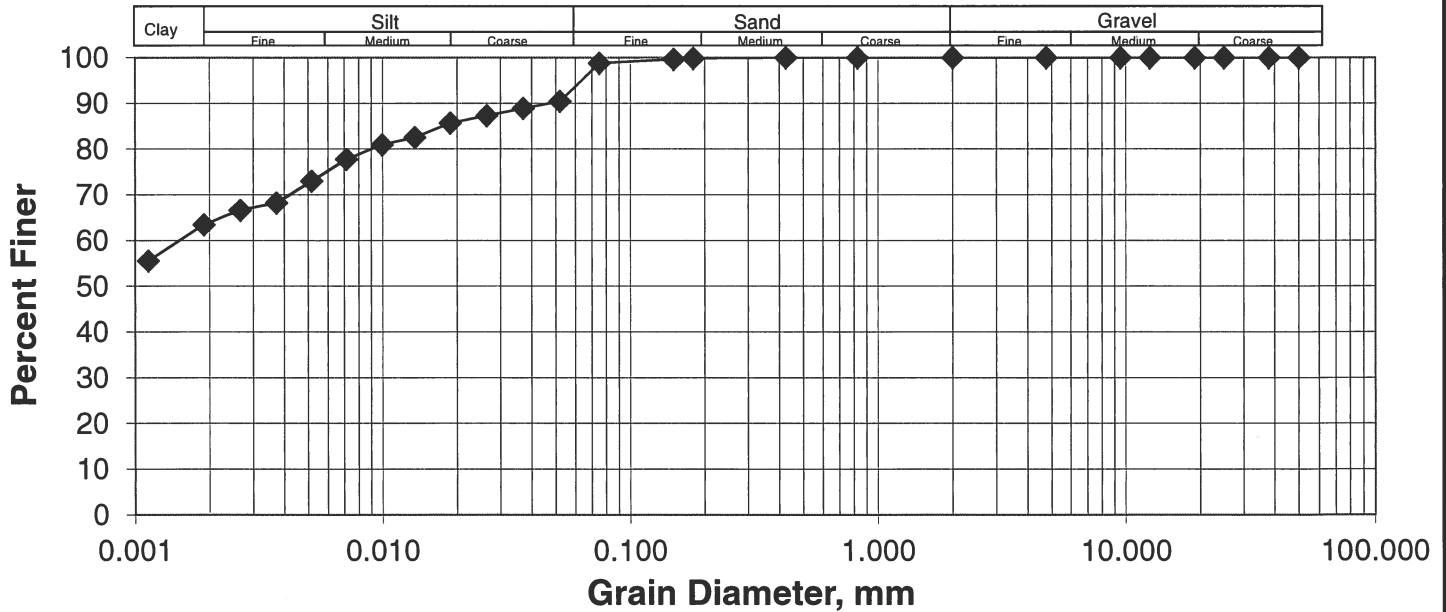
tel (204) 477-5381 fax (204) 284-2040

Job No.: 60219315
 Client: City of Winnipeg
 Project: Jefferson East CSR
 Date Tested: 2-Apr-15
 Tested By: MLotecki

Hole No.: TH15-01
 Sample No.: G3
 Depth: 1.22 - 1.37 m
 Date Sampled: 24-Feb-15
 Sampled By: AECOM

GRAVEL SIZES		SAND SIZES		FINES	
Grain Size (mm.)	Total Percent Passing	Grain Size (mm.)	Total Percent Passing	Grain Size (mm.)	Total Percent Passing
50.0	100.0	2.00	100.0	0.0750	98.8
38.0	100.0	0.83	100.0	0.0518	90.5
25.0	100.0	0.43	100.0	0.0370	88.9
19.0	100.0	0.18	99.8	0.0264	87.3
12.5	100.0	0.15	99.6	0.0188	85.7
9.5	100.0	0.075	98.8	0.0135	82.5
4.75	100.0			0.0099	80.9
2.00	100.0			0.0071	77.8
				0.0052	73.0
				0.0037	68.2
				0.0026	66.6
				0.0019	63.5
				0.0011	55.5

GRAIN SIZE DISTRIBUTION CURVE



Gravel	0.0%	Silt	29.5%
Sand	6.6%	Clay	63.9%

** Note: Soil Classification based on Grain Size from Canadian Foundation Engineering Manual, 3rd edition (1992).



AECOM Canada Ltd.
 Winnipeg Geotechnical Laboratory
 99 Commerce Drive
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 Phone: 204 477 5381 Fax: 204 284 2040

Project Name: Jefferson East CSR (Phase 2)
 Project Number: 60599385
 Client: City of Winnipeg
 Sample Location: Varies
 Sample Depth: Varies
 Sample Number: Varies

Supplier: AECOM
 Specification: N/A
 Field Technician: RHarras
 Sample Date: Varies
 Lab Technician: RHarras
 Date Tested: June 24 - 27, 2019

Moisture Content (ASTM D2216-10)

Standard Test Method for Laboratory Determination of Water (Moisture) Content of Soil and Rock by Mass

Location	Sample	Depth (m)	Moisture Content (%)
TH19-01	G1A	1.07 - 1.22 m	20.5%
	G1	1.37 - 1.52 m	28.6%
	G3	4.42 - 4.57 m	47.3%
	G4	5.94 - 6.10 m	46.4%
	G6	8.99 - 9.14 m	43.9%
	G7	10.52 - 10.67 m	39.9%
	S9	13.72 - 14.17 m	10.1%
	S10	14.94 - 15.01 m	16.4%
TH19-02	G16A	0.69 - 0.84 m	19.4%
	G16	1.37 - 1.52 m	27.1%
	G17	2.90 - 3.05 m	48.9%
	G18	4.42 - 4.57 m	48.3%
	G20	7.47 - 7.62 m	43.0%
	G21	8.99 - 9.14 m	41.3%
	G22	10.52 - 10.67 m	37.3%
	G23	12.04 - 12.19 m	38.3%
TH19-03	G24	1.37 - 1.52 m	27.7%
	G25	2.90 - 3.05 m	42.6%
	G26	4.42 - 4.57 m	55.1%
	G27	5.94 - 6.10 m	38.6%
	G28	7.47 - 7.62 m	44.0%
	G30	10.52 - 10.67 m	33.8%
	G31	12.04 - 12.19 m	26.3%
TH19-04	G32A	0.91 - 1.07 m	20.8%
	G32	1.37 - 1.52 m	24.8%
	G33	2.90 - 3.05 m	48.4%
	G34	4.42 - 4.57 m	48.7%
	G35	5.94 - 6.10 m	50.9%
	G37	8.99 - 9.14 m	41.3%
	G38	10.52 - 10.67 m	47.3%
	G39	12.04 - 12.19 m	41.4%
TH19-05	G40A	0.61 - 0.76 m	25.9%
	G40	1.37 - 1.52 m	26.6%
	G41	2.90 - 3.05 m	22.9%
	G43	5.94 - 6.10 m	45.4%
	G45	8.99 - 9.14 m	45.3%
	G47	12.04 - 12.19 m	47.5%
	S49	15.24 - 15.70 m	11.5%

Location	Sample	Depth (m)	Moisture Content (%)
	S50	16.31 - 16.56 m	9.3%
TH19-06	G55	1.37 - 1.52 m	22.3%
	G56A	1.75 - 1.91 m	28.6%
	G56B	2.21 - 2.36 m	23.3%
	G56	2.90 - 3.05 m	41.5%
	G57	4.42 - 4.57 m	46.8%
	G59	7.47 - 7.62 m	44.0%
	G60	8.99 - 9.14 m	53.3%
	G61	10.52 - 10.67 m	50.8%
	G62	12.04 - 12.19 m	59.1%
TH19-07	G63	1.37 - 1.52 m	12.3%
	G64	2.90 - 3.05 m	47.1%
	G65	4.42 - 4.57 m	50.5%
	G66	5.94 - 6.10 m	53.6%
	G68	8.99 - 9.14 m	52.2%
	G69	10.52 - 10.67 m	51.4%
	G70	12.04 - 12.19 m	54.6%
TH19-08	G71A	0.69 - 0.84 m	19.7%
	G71	1.37 - 1.52 m	36.3%
	G72	2.90 - 3.05 m	53.7%
	G73	4.42 - 4.57 m	53.4%
	G75	7.47 - 7.62 m	45.2%
	G76	8.99 - 9.14 m	48.7%
	G77	10.52 - 10.67 m	49.2%
	G78	12.04 - 12.19 m	46.7%
	G79	13.56 - 13.72 m	22.9%
	G80	15.09 - 15.24 m	11.7%
	S81	16.76 - 17.22 m	12.0%
TH19-10	G91A	0.84 - 0.99 m	25.4%
	G91	1.37 - 1.52 m	22.6%
	G92	2.90 - 3.05 m	33.9%
	G93	4.42 - 4.57 m	52.3%
	G94	5.94 - 6.10 m	51.5%
	G96	8.99 - 9.14 m	41.1%
	G97	10.52 - 10.67 m	47.3%
	G98	12.04 - 12.19 m	48.3%
TH19-11	G99A	0.53 - 0.69 m	31.4%
	G99	1.37 - 1.52 m	21.7%

MOISTURE CONTENT OF SOIL (ASTM D2216)

CLIENT: AECOM	TEST NO: 19- 001	PROJECT NO: 112-1909
PROJECT: Jefferson East CSR (Phase 2)	DATE SAMPLED: 26-Jul-2019	SAMPLED BY: Client
PROJECT CONTACT: Ryan Harras	DATE TESTED: 29-Jul-2019	TESTED BY: Navpreet Singh
TEST LOCATION: Winnipeg Manitoba		

Description	TH 19 - 16	TH 19 - 16		
Sample	T148-2	T154		
Wt Wet Sample + Tare	127.40	201.70		
Wt Dry Sample + Tare	88.40	125.50		
Wt Water	39.00	76.20		
Wt Tare	4.30	4.20		
Wt Dry Sample	84.10	121.30		
Moisture Content (%)	46.4	62.8		

Description				
Sample				
Wt Wet Sample + Tare				
Wt Dry Sample + Tare				
Wt Water				
Wt Tare				
Wt Dry Sample				
Moisture Content (%)				

Description				
Sample				
Wt Wet Sample + Tare				
Wt Dry Sample + Tare				
Wt Water				
Wt Tare				
Wt Dry Sample				
Moisture Content (%)				

Description				
Sample				
Wt Wet Sample + Tare				
Wt Dry Sample + Tare				
Wt Water				
Wt Tare				
Wt Dry Sample				
Moisture Content (%)				

CLIENT: AECOM
 99 Commerce Drive,
 Winnipeg, MB R3P 0Y7
 ATTENTION: Ryan Harras
 PROJECT: Jefferson East CSR (Phase 2)

PROJECT NO.: 112-1909

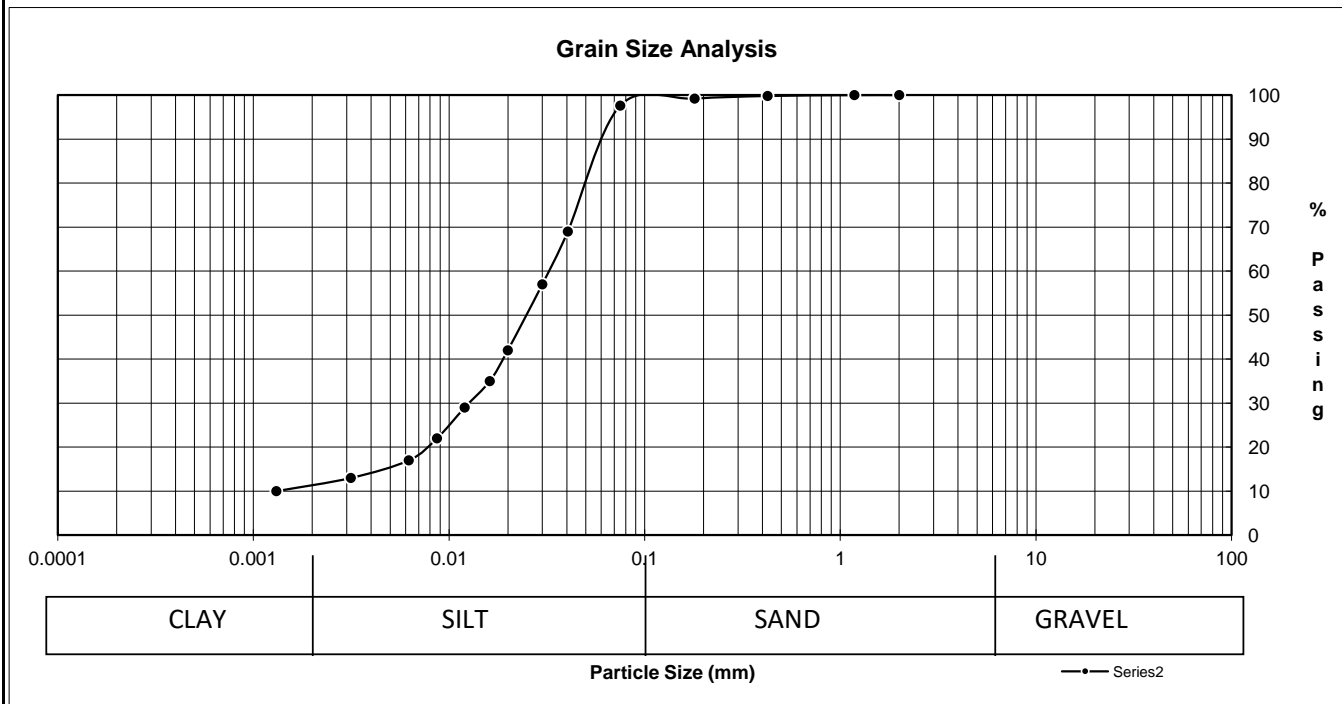
Summary of Particle Size Analysis and Atterberg Limits

Hole No	Sample No.	% Clay	% Silt	% Sand	Liquid Limit	Plastic Limit	Plasticity Index
TH 19-05	G41	11	86.6	2.4	Non plastic		
TH 19-08	T74	64	35	1.0	60	25	35
TH 19-08	S81	21	43.8	35.2	22	10	12
TH 19-14	G127A	18	80.6	1.4	24	16	8
TH 19-15	T140	80	20	0	70	31	39
TH 19-16	G146	55	39	6.0	53	23	30

PARTICLE SIZE ANALYSIS OF SOILS TEST REPORT

CLIENT:	AECOM	PROJECT NO.	112-1909
	99 Commerce Drive, Winnipeg, MB R3P 0Y7	Test No:	1
ATTENTION:	Ryan Harras	Lab No:	HM 314
PROJECT:	Jefferson East CSR (Phase 2) Winnipeg, Manitoba		

Date Sampled:	6/24-27/2019	Date Received:	27-Jul-19	Sieve Analysis		Hydrometer Analysis	
Sampled By:	Client	Date Tested:	1-Aug-19	Sieve (mm)	% Passing	Diameter	% Finer
Material Identification B.H./T.H. No. TH 19-05 Sample No. G41 Sample depth 10' Specific Gravity of Material: 2.65				50.00	100.0		
				37.50	100.0		
				25.00	100.0		
				19.00	100.0		
				16.00	100.0		
				12.50	100.0	0.0405	69.0
				9.50	100.0	0.0300	57.0
				4.75	100.0	0.0200	42.0
				2.00	100.0	0.0162	35.0
				1.18	100.0	0.0120	29.0
0.425	99.8	0.0087	22.0				
0.180	99.2	0.0062	17.0				
0.075	97.6	0.0031	13.0				



SOIL DESCRIPTION	% Composition		D10	0.00130
	2.4	Gravel	D30	0.01205
	86.6	Sand	D60	0.03245
	11.0	Silt	Cu	24.96
		Clay	Cc	3.44

Remarks: Test Method: ASTM D422, D2216, D4318
Technician: Navi

P. Bevel

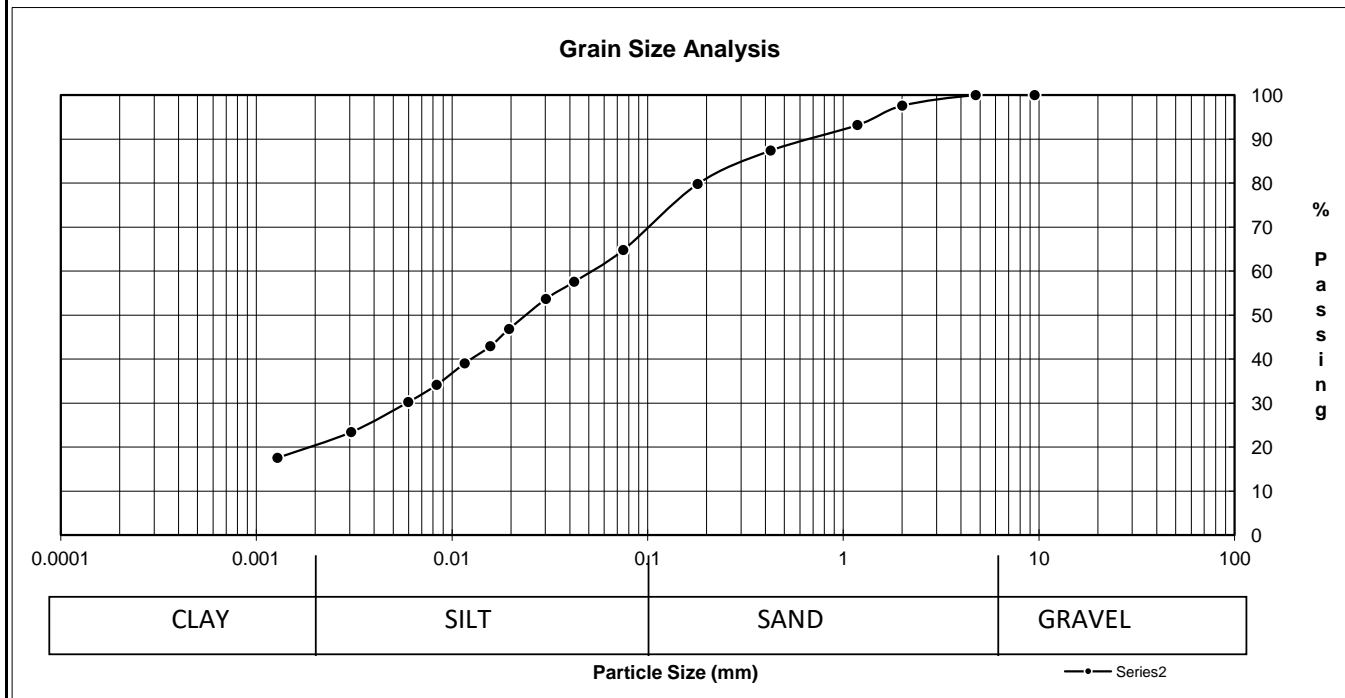
Reviewed by: Paul Bevel

PARTICLE SIZE ANALYSIS OF SOILS TEST REPORT

CLIENT:	AECOM 99 Commerce Drive, Winnipeg, MB R3P 0Y7	PROJECT NO.	112-1909
ATTENTION:	Ryan Harras	Test No:	2
PROJECT:	Jefferson East CSR (Phase 2) Winnipeg, Manitoba	Lab No:	HM 314

Date Sampled:	Date Received:	Sieve Analysis		Hydrometer Analysis	
Sampled By:	Date Tested:	Sieve (mm)	% Passing	Diameter	% Finer
6/24-27/2019	27-Jul-19	50.00	100.0		
Client	1-Aug-19	37.50	100.0		
		25.00	100.0		
		19.00	100.0		
		16.00	100.0		
		12.50	100.0	0.0421	57.6
		9.50	100.0	0.0302	53.7
		4.75	100.0	0.0196	46.8
		2.00	97.6	0.0157	42.9
		1.18	93.2	0.0116	39.0
		0.425	87.4	0.0083	34.2
		0.180	79.8	0.0060	30.3
		0.075	64.8	0.0030	23.4

Material Identification
 B.H./T.H. No. TH 19-08
 Sample No. S81
 Sample depth 55'
 Specific Gravity of Material: 2.65



SOIL DESCRIPTION	% Composition		D10	
		35.2	Gravel	D30
	43.8	Sand	D60	0.00184
	21.0	Silt	Cu	#DIV/0!
		Clay	Cc	#DIV/0!

Remarks: Test Method: ASTM D422, D2216, D4318
 Technician: Navi

P. Bevel

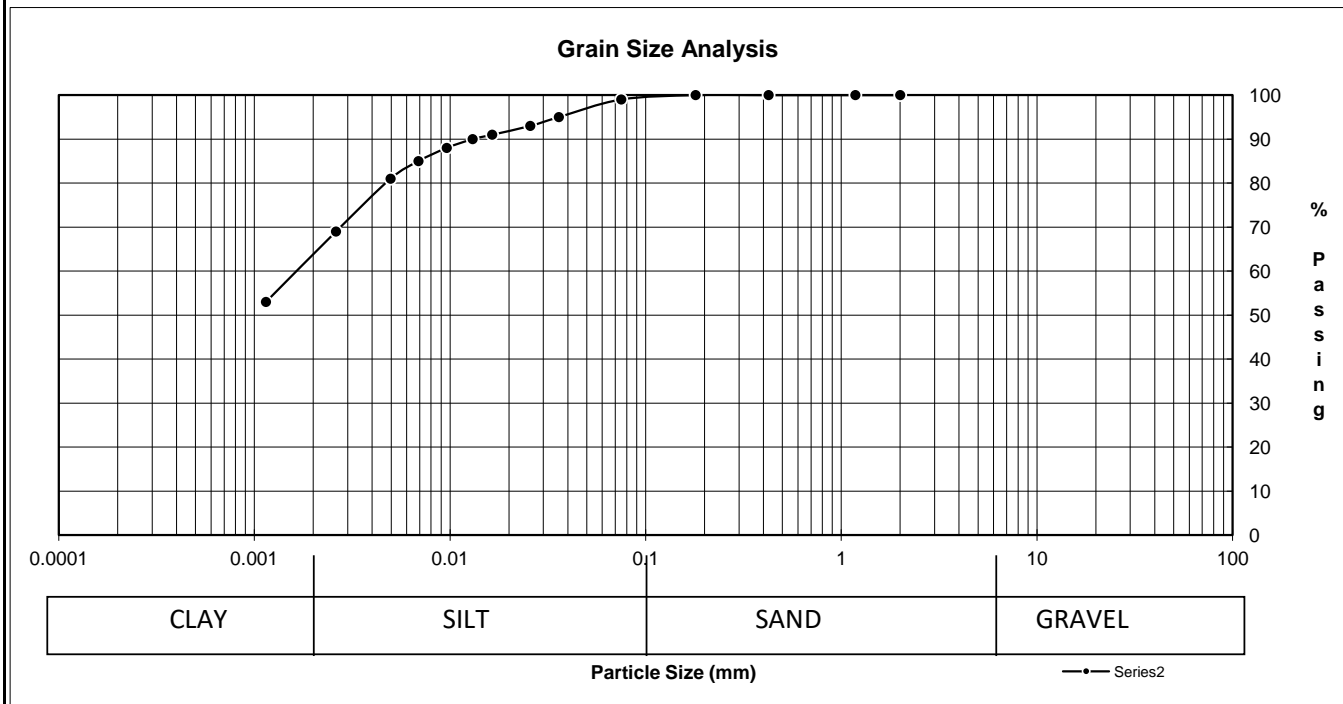
Reviewed by: Paul Bevel

PARTICLE SIZE ANALYSIS OF SOILS TEST REPORT

CLIENT:	AECOM	PROJECT NO.	112-1909
	99 Commerce Drive, Winnipeg, MB R3P 0Y7	Test No:	3
ATTENTION:	Ryan Harras	Lab No:	HM 314
PROJECT:	Jefferson East CSR (Phase 2) Winnipeg, Manitoba		

Date Sampled:	6/24-27/2019	Date Received:	27-Jul-19	Sieve Analysis	Hydrometer Analysis
Sampled By:	Client	Date Tested:	1-Aug-19	Sieve (mm) % Passing	Diameter % Finer

Material Identification B.H./T.H. No. TH 19-08 Sample No. T74 Sample depth 20' Specific Gravity of Material: 2.65	50.00	100.0		
	37.50	100.0		
	25.00	100.0		
	19.00	100.0		
	16.00	100.0		
	12.50	100.0	0.0360	95.0
	9.50	100.0	0.0257	93.0
	4.75	100.0	0.0164	91.0
	2.00	100.0	0.0130	90.0
	1.18	100.0	0.0096	88.0
	0.425	100.0	0.0069	85.0
	0.180	100.0	0.0050	81.0
0.075	99.0	0.0026	69.0	



SOIL DESCRIPTION	% Composition		D10	
	1.0	Gravel	D30	
	35.0	Sand	D60	0.00184
	64.0	Silt	Cu	#DIV/0!
		Clay	Cc	#DIV/0!

Remarks: Test Method: ASTM D422, D2216, D4318
Technician: Navi

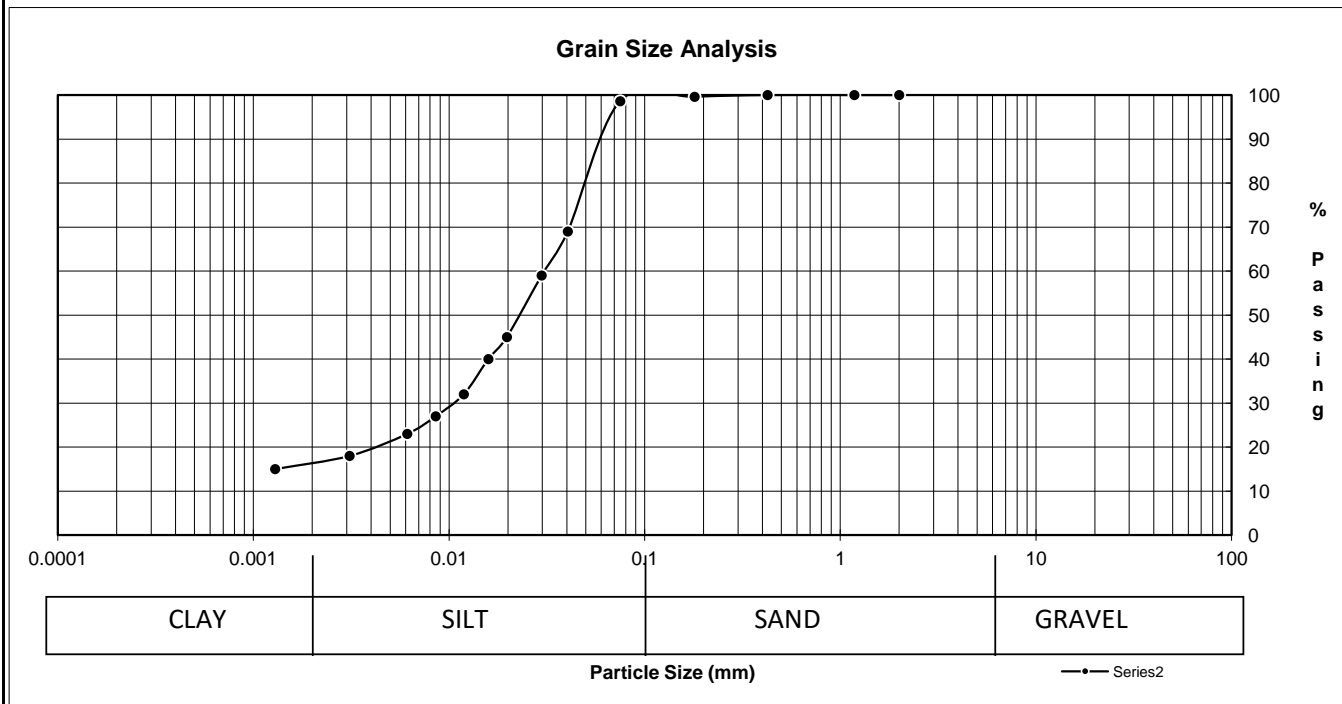
P. Bevel

Reviewed by: Paul Bevel

PARTICLE SIZE ANALYSIS OF SOILS TEST REPORT

CLIENT:	AECOM	PROJECT NO.	112-1909
	99 Commerce Drive, Winnipeg, MB R3P 0Y7	Test No:	4
ATTENTION:	Ryan Harras	Lab No:	HM 314
PROJECT:	Jefferson East CSR (Phase 2) Winnipeg, Manitoba		

Date Sampled:	6/24-27/2019	Date Received:	27-Jul-19	Sieve Analysis		Hydrometer Analysis	
Sampled By:	Client	Date Tested:	1-Aug-19	Sieve (mm)	% Passing	Diameter	% Finer
Material Identification B.H./T.H. No. TH 19-14 Sample No. G127A Sample depth 7.5' Specific Gravity of Material: 2.65				50.00	100.0		
				37.50	100.0		
				25.00	100.0		
				19.00	100.0		
				16.00	100.0		
				12.50	100.0	0.0405	69.0
				9.50	100.0	0.0297	59.0
				4.75	100.0	0.0198	45.0
				2.00	100.0	0.0159	40.0
				1.18	100.0	0.0119	32.0
0.425	100.0	0.0085	27.0				
0.180	99.6	0.0061	23.0				
0.075	98.6	0.0031	18.0				



SOIL DESCRIPTION	% Composition		D10	
	Gravel		D30	0.01268
	1.4	Sand	D60	0.02970
	80.6	Silt	Cu	#DIV/0!
	18.0	Clay	Cc	#DIV/0!

Remarks: Test Method: ASTM D422, D2216, D4318
Technician: Navi

P. Bevel

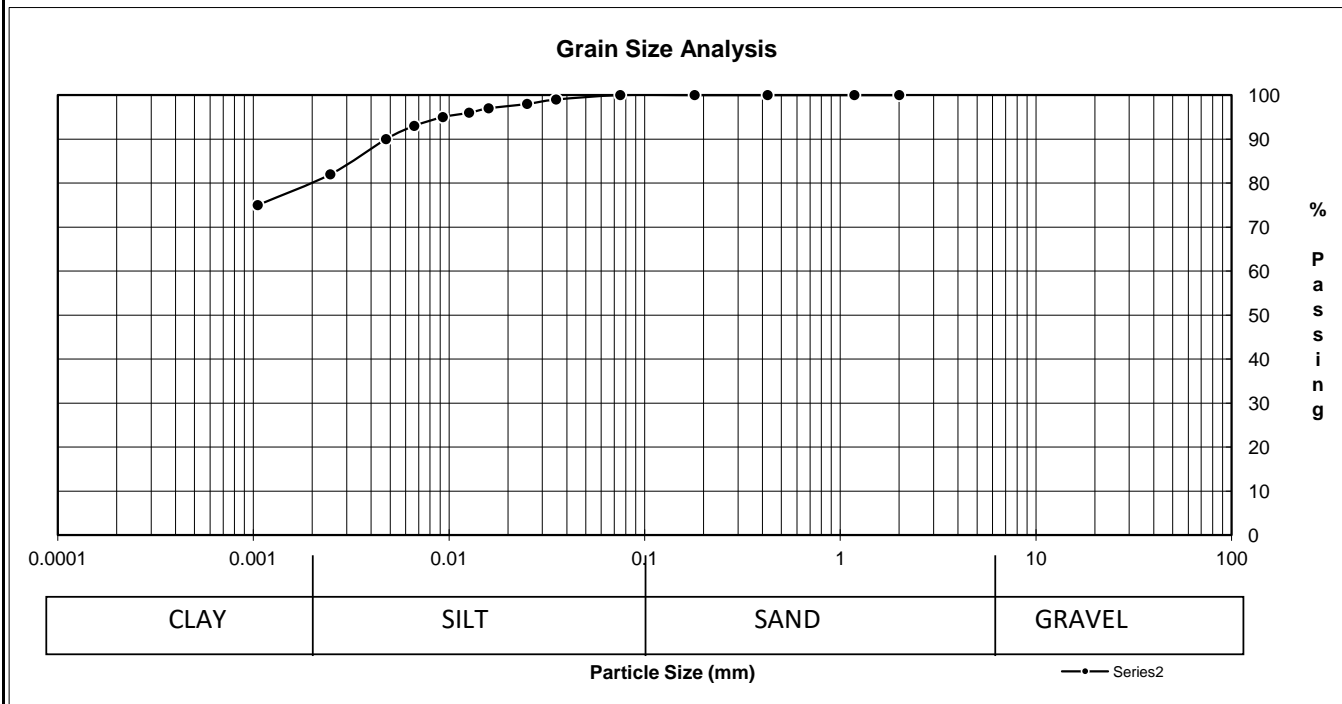
Reviewed by: Paul Bevel

PARTICLE SIZE ANALYSIS OF SOILS TEST REPORT

CLIENT:	AECOM	PROJECT NO.	112-1909
	99 Commerce Drive, Winnipeg, MB R3P 0Y7	Test No:	5
ATTENTION:	Ryan Harras	Lab No:	HM 314
PROJECT:	Jefferson East CSR (Phase 2) Winnipeg, Manitoba		

Date Sampled:	6/24-27/2019	Date Received:	27-Jul-19	Sieve Analysis	Hydrometer Analysis
Sampled By:	Client	Date Tested:	1-Aug-19	Sieve (mm) % Passing	Diameter % Finer

Material Identification B.H./T.H. No. TH 19-15 Sample No. T140 Sample depth 10' Specific Gravity of Material: 2.65	50.00	100.0		
	37.50	100.0		
	25.00	100.0		
	19.00	100.0		
	16.00	100.0		
	12.50	100.0	0.0353	99.0
	9.50	100.0	0.0251	98.0
	4.75	100.0	0.0159	97.0
	2.00	100.0	0.0127	96.0
	1.18	100.0	0.0093	95.0
	0.425	100.0	0.0066	93.0
	0.180	100.0	0.0048	90.0
0.075	100.0	0.0025	82.0	



SOIL DESCRIPTION	% Composition		D10	
	Gravel		D30	
	Sand		D60	
	20.0	Silt	Cu	#DIV/0!
	80.0	Clay	Cc	#DIV/0!

Remarks: Test Method: ASTM D422, D2216, D4318
Technician: Navi

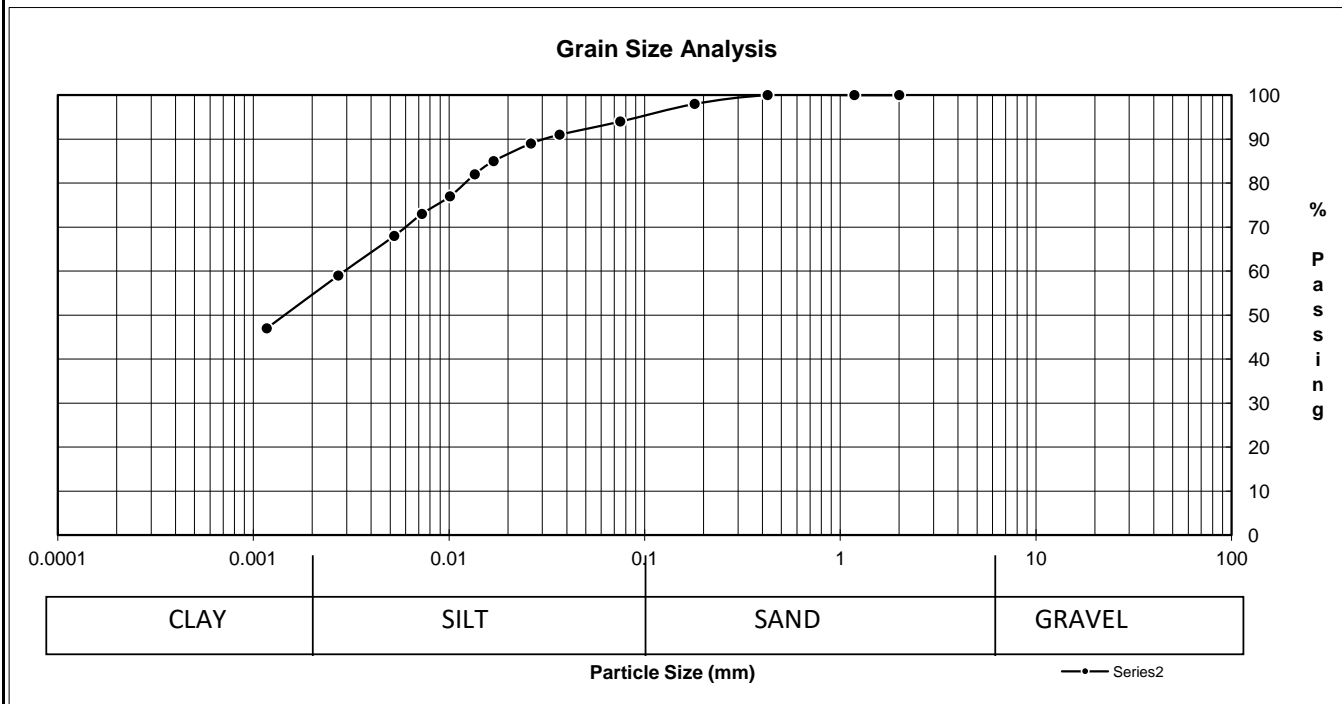
P. Bevel

Reviewed by: Paul Bevel

PARTICLE SIZE ANALYSIS OF SOILS TEST REPORT

CLIENT:	AECOM	PROJECT NO.	112-1909
	99 Commerce Drive, Winnipeg, MB R3P 0Y7	Test No:	6
ATTENTION:	Ryan Harras	Lab No:	HM 314
PROJECT:	Jefferson East CSR (Phase 2) Winnipeg, Manitoba		

Date Sampled:	6/24-27/2019	Date Received:	27-Jul-19	Sieve Analysis		Hydrometer Analysis	
Sampled By:	Client	Date Tested:	1-Aug-19	Sieve (mm)	% Passing	Diameter	% Finer
Material Identification B.H./T.H. No. TH 19-15 Sample No. T146 Sample depth 5' Specific Gravity of Material: 2.65				50.00	100.0		
				37.50	100.0		
				25.00	100.0		
				19.00	100.0		
				16.00	100.0		
				12.50	100.0	0.0367	91.0
				9.50	100.0	0.0262	89.0
				4.75	100.0	0.0169	85.0
				2.00	100.0	0.0135	82.0
				1.18	100.0	0.0101	77.0
0.425	100.0	0.0073	73.0				
0.180	98.0	0.0052	68.0				
0.075	94.0	0.0027	59.0				



SOIL DESCRIPTION	% Composition		D10	
	6.0	Gravel	D30	
	39.0	Sand	D60	0.00272
	55.0	Silt	Cu	#DIV/0!
		Clay	Cc	#DIV/0!

Remarks: Test Method: ASTM D422, D2216, D4318
Technician: Navi

P. Bevel

Reviewed by: Paul Bevel

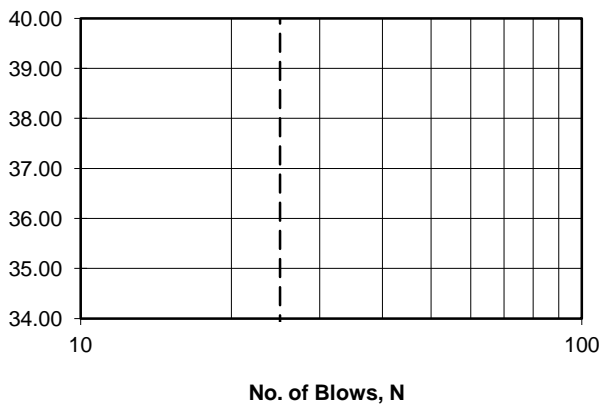
Atterberg Limits (ASTM D4318)

Client: AECOM 99 Commerce Drive Winnipeg, MB R3P 0Y7 Attention: Ryan Harras Project: Jefferson East CSR (Phase 2)	Project No.: 112-1909 Test No.: 1 Lab No.: HM 314 Date Received: 27-Jul-19 Date Tested / By: 2-Aug-19 / NS
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Liquid Limit Determination

Dish No.:	1	2	3		Liquid Limit 25 Blows
Wet Soil + Dish:					
Dry Soil + Dish:					
Moisture:					
Dish:					
Dry Soil:	Liquid Limit could not be determined (See Remarks)				
% Moisture:					
No. of Blows:					
Liquid Limits:					

Liquid Limit



Material Identification:

T.H./B.H. No. TH 19-05, G41

Depth: 10 ft

Liquid Limit, %: n/a
Plastic Limit, %: n/a
Plasticity Index: n/a
 (LL-PL)

Plastic Limit Determination

Dish No.:				
Wet Soil + Dish:				
Dry Soil + Dish:				
Moisture:				
Dish:				
Dry Soil:	Non-Plastic			
% Moisture:				
Average:				

Test Method : ASTM: D4318, D2216

Remarks: ASTM D4318 - Section 12.5: When successive trials have been made where the number of drops required to close the groove is always less than 25, record that the Liquid Limit could not be determined and report the soil as Non-Plastic.

Reviewed by: 
 Gladys Paciente, P.Eng

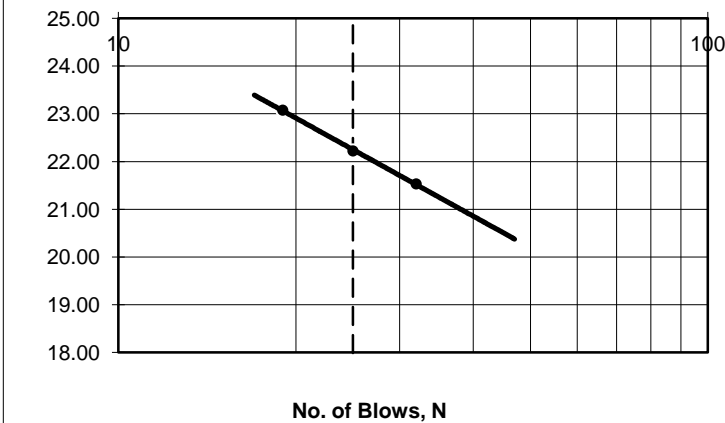
Atterberg Limits (ASTM D4318)

Client: AECOM 99 Commerce Drive Winnipeg, MB R3P 0Y7 Attention.: Ryan Harras Project: Jefferson East CSR (Phase 2)	Project No.: 112-1909 Test No.: 2 Lab No.: HM 314 Date Received: 27-Jul-19 Date Tested / By: 2-Aug-19 / NS
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Liquid Limit Determination

Dish No.:	1	2	3		Liquid Limit 25 Blows
Wet Soil + Dish:	13	15.4	13.9		
Dry Soil + Dish:	12.1	13.40	12.1		
Moisture:	0.9	2	1.8		
Dish:	4.4	4.4	4.3		
Dry Soil:	4.18	9	7.8		
% Moisture:	21.53	22.22	23.08		
No. of Blows:	32	25	19		
Liquid Limits:	22.18	22.22	22.32		

Liquid Limit



Material Identification:

T.H./B.H. No. **TH 19-08, S81**

Depth: **55ft**

Liquid Limit, %: **22**
 Plastic Limit, %: **10**
 Plasticity Index: **12**
 (LL-PL)

Plastic Limit Determination

Dish No.:	1	2	3		
Wet Soil + Dish:	12.5	12.35	12.14		
Dry Soil + Dish:	11.8	11.62	11.49		
Moisture:	0.7	0.73	0.65		
Dish:	4.53	4.28	4.43		
Dry Soil:	7.27	7.34	7.06		
% Moisture:	9.63	9.95	9.21		
Average:					10

Test Method : ASTM: D4318, D2216

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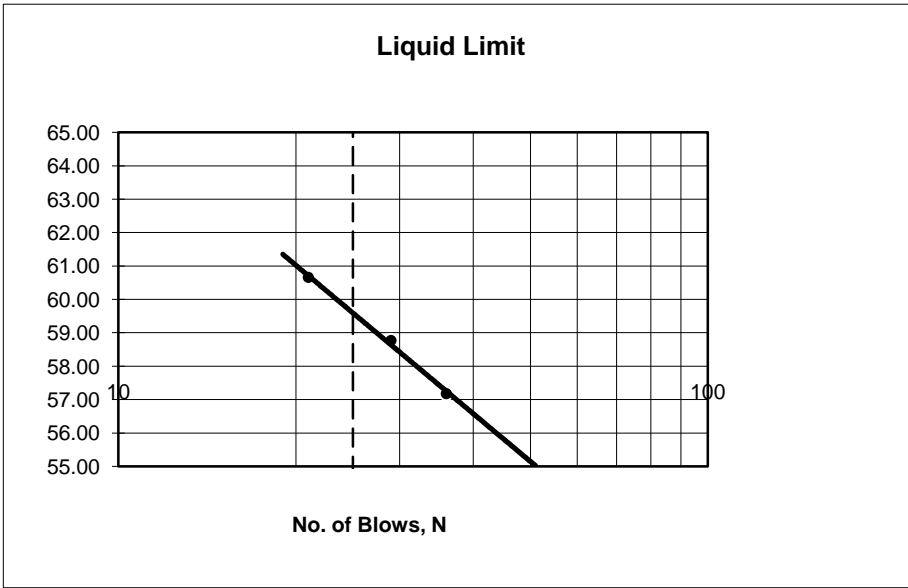
Reviewed by: Paul Bevel

Atterberg Limits (ASTM D4318)

Client: AECOM 99 Commerce Drive Winnipeg, MB R3P 0Y7	Project No.: 112-1909
Attention.: Ryan Harras	Test No.: 3
Project: Jefferson East CSR (Phase 2)	Lab No.: HM 314
	Date Received: 27-Jul-19
	Date Tested / By: 2-Aug-19 / NS

Liquid Limit Determination

Dish No.:	1	2	3		Liquid Limit 25 Blows
Wet Soil + Dish:	11.51	12.73	11.54		
Dry Soil + Dish:	9.12	9.58	8.78		
Moisture:	2.39	3.15	2.76		
Dish:	4.45	4.22	4.23		
Dry Soil:	4.18	5.36	4.55		
% Moisture:	57.18	58.77	60.66		
No. of Blows:	36	29	21		
Liquid Limits:	59.76	59.83	59.39		60



Material Identification:

T.H./B.H. No. **TH 19-08, T74**

Depth: **20ft**

Liquid Limit, %: **60**
 Plastic Limit, %: **25**
 Plasticity Index: **35**
 (LL-PL)

Plastic Limit Determination

Dish No.:	1	2	3		
Wet Soil + Dish:	9.8	9.69	9.42		
Dry Soil + Dish:	8.68	8.6	8.38		
Moisture:	1.12	1.09	1.04		
Dish:	4.25	4.28	4.29		
Dry Soil:	4.43	4.32	4.09		
% Moisture:	25.28	25.23	25.43		
Average:					25

Test Method : ASTM: D4318, D2216

P. Bevel

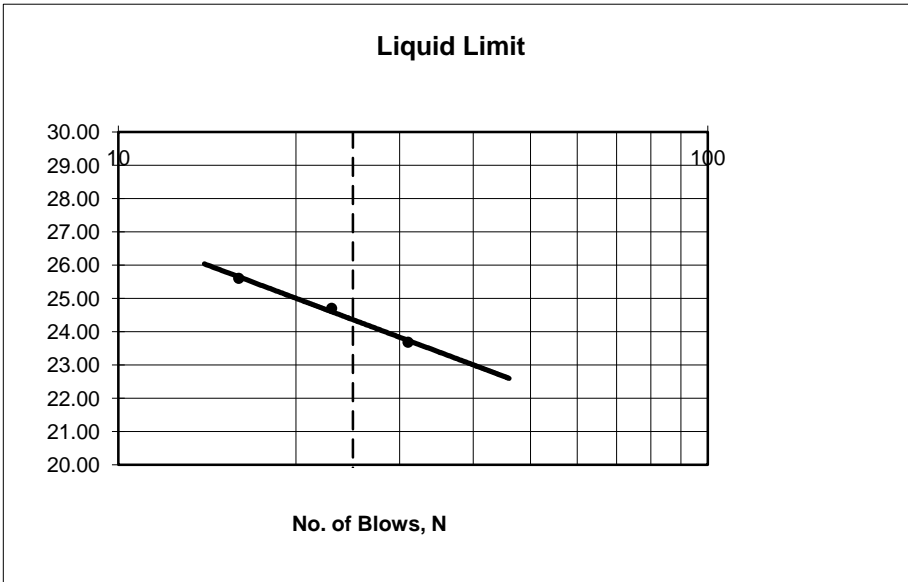
Reviewed by: Paul Bevel

Atterberg Limits (ASTM D4318)

Client: AECOM 99 Commerce Drive Winnipeg, MB R3P 0Y7 Attention.: Ryan Harras Project: Jefferson East CSR (Phase 2)	Project No.: 112-1909 Test No. 4 Lab No.: HM 314 Date Received: 27-Jul-19 Date Tested / By: 2-Aug-19 / NS
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Liquid Limit Determination

Dish No.:	1	2	3		Liquid Limit 25 Blows
Wet Soil + Dish:	12.4	14.7	14.8		
Dry Soil + Dish:	11.41	12.62	12.68		
Moisture:	0.99	2.08	2.12		
Dish:	4.2	4.2	4.4		
Dry Soil:	4.18	8.42	8.28		
% Moisture:	23.68	24.70	25.60		
No. of Blows:	31	23	16		
Liquid Limits:	24.31	24.46	24.26		



Material Identification:

T.H No **TH 19-14, G127 A**

Depth: **7.5ft**

Liquid Limit, %: **24**

Plastic Limit, %: **16**

Plasticity Index: **8**
(LL-PL)

Plastic Limit Determination

Dish No.:	1	2	3		
Wet Soil + Dish:	12.1	11.8	12.2		
Dry Soil + Dish:	11	10.72	11.1		
Moisture:	1.1	1.08	1.1		
Dish:	4.2	4.1	4.3		
Dry Soil:	6.8	6.62	6.8		
% Moisture:	16.18	16.31	16.18		
Average:					16

Test Method : ASTM: D4318, D2216

P. Bevel

Reviewed by: Paul Bevel

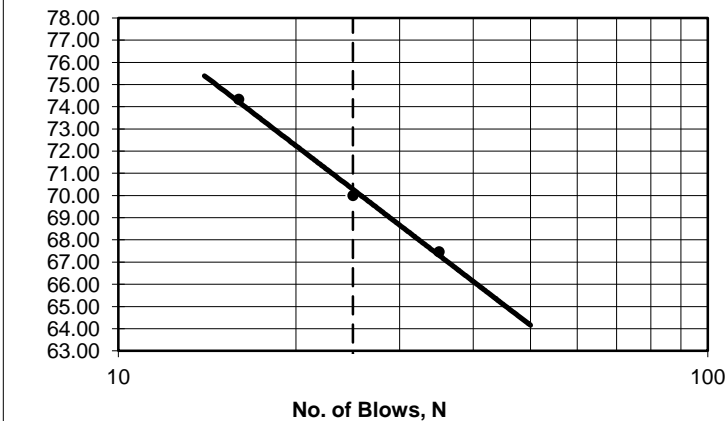
Atterberg Limits (ASTM D4318)

Client: AECOM 99 Commerce Drive Winnipeg, MB R3P 0Y7 Attention.: Ryan Harras Project: Jefferson East CSR (Phase 2)	Project No.: 112-1909 Test No.: 5 Lab No.: HM 314 Date Received: 27-Jul-19 Date Tested / By: 2-Aug-19 / Navi
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Liquid Limit Determination

Dish No.:	1	2	3		Liquid Limit 25 Blows
Wet Soil + Dish:	12.75	13.02	12.07		
Dry Soil + Dish:	9.93	9.38	8.71		
Moisture:	2.82	3.64	3.36		
Dish:	4.17	4.18	4.19		
Dry Soil:	4.18	5.2	4.52		
% Moisture:	67.46	70.00	74.34		
No. of Blows:	35	25	16		
Liquid Limits:	70.27	70.00	70.43		70

Liquid Limit



Material Identification:

T.H No **TH 19-14, G127 A**

Depth: **7.5ft**

Liquid Limit, %: **70**
 Plastic Limit, %: **31**
 Plasticity Index: **39**
 (LL-PL)

Plastic Limit Determination

Dish No.:	1	2	3		
Wet Soil + Dish:	10.04	9.9	10.62		
Dry Soil + Dish:	8.65	8.58	9.1		
Moisture:	1.39	1.32	1.52		
Dish:	4.22	4.21	4.28		
Dry Soil:	4.43	4.37	4.82		
% Moisture:	31.38	30.21	31.54		
Average:					31

Test Method : ASTM: D4318, D2216

P. Bevel

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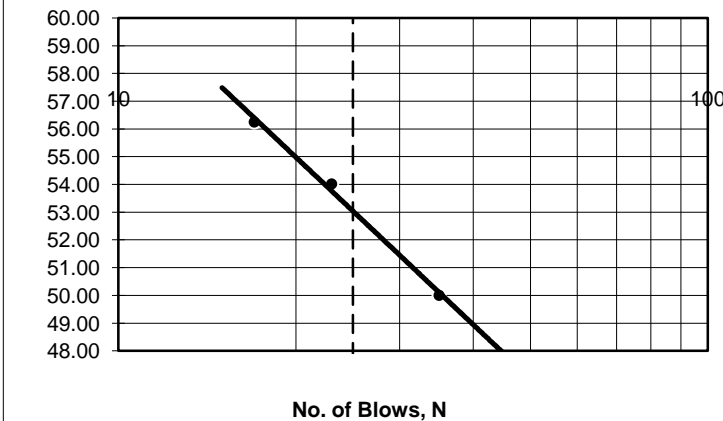
Atterberg Limits (ASTM D4318)

Client: AECOM 99 Commerce Drive Winnipeg, MB R3P 0Y7	Project No.: 112-1909
Attention.: Ryan Harras	Test No.: 6
Project: Jefferson East CSR (Phase 2)	Lab No.: HM 314
	Date Received: 27-Jul-19
	Date Tested / By: 2-Aug-19 / NS

Liquid Limit Determination

Dish No.:	1	2	3		Liquid Limit 25 Blows
Wet Soil + Dish:	13.6	11.12	13.24		
Dry Soil + Dish:	11.51	8.70	10		
Moisture:	2.09	2.42	3.24		
Dish:	4.19	4.22	4.24		
Dry Soil:	4.18	4.48	5.76		
% Moisture:	50.00	54.02	56.25		
No. of Blows:	35	23	17		
Liquid Limits:	52.08	53.48	53.69		53

Liquid Limit



Material Identification:

T.H No **TH 19-16, G126**

Depth: **5ft**

Liquid Limit, %: **53**

Plastic Limit, %: **23**

Plasticity Index: **30**

(LL-PL)

Plastic Limit Determination

Dish No.:	1	2	3		
Wet Soil + Dish:	10.26	10.13	10.03		
Dry Soil + Dish:	9.15	9	8.95		
Moisture:	1.11	1.13	1.08		
Dish:	4.43	4.18	4.31		
Dry Soil:	4.72	4.82	4.64		
% Moisture:	23.52	23.44	23.28		
Average:					

Test Method : ASTM: D4318, D2216

P. Bevel

Reviewed by: Paul Bevel



PARTICLE SIZE ANALYSIS

Report Date: 20 September 2019

Client

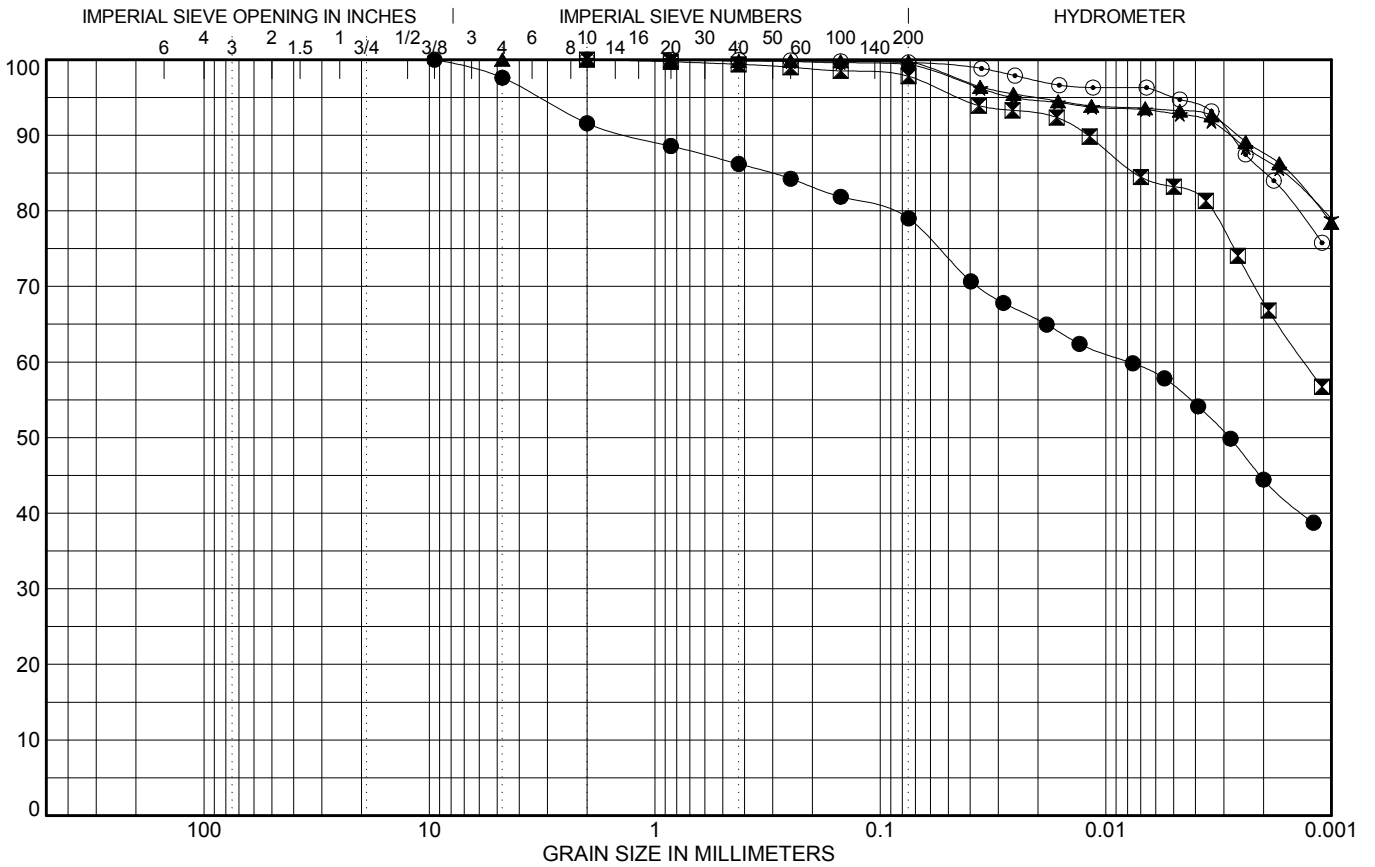
Name: AECOM C/O Dyregrov Robinson Inc.
Address: 1692 Dublin Avenue, Winnipeg, MB
Attention: Gil Robinson
PO Number:

Project

Name: Jefferson East CSR (Phase 2)
Address: Jefferson Avenue, Winnipeg MB
Project No.: WX11735
Manager: JW

Gradation Specification:

WX11735 - DYREGROV ROBINSON - JEFFERSON EAST GEOTECHNICAL TESTING.GPJ - 19/09/20 03:22 PM (WOOD - PSA MULTI RESULT WITH ATTERBERG)



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Sample ID	mUSCS	MC	D100	D60	D30	D10	LL	PL	% Gravel	% Sand	% Fines
● TH19-02, 6.1 m	CH		9.5	0			50	14	2	19	35 (Silt) : 44 (Clay)
⊠ TH19-06, 6.1 m	CH		2	0			68	20	0	2	30 (Silt) : 68 (Clay)
▲ TH19-11, 4.6 m	CH		4.8				90	21	0	0	12 (Silt) : 88 (Clay)
★ TH19-14, 6.1 m	CH		2				79	27	0	1	13 (Silt) : 87 (Clay)
⊙ TH19-16, 3 m	CH		2				88	24	0	0	14 (Silt) : 85 (Clay)

PROJECT: Jefferson East CSR
AECOM PROJECT No.: 60599385

DATE: July 2019

Test Hole	19-01	Depth	10 feet
		Sample No.	T2
Wet + Tare Wt.	209.02 g	Length	176 mm
Dry + Tare Wt.	151.52 g	Diameter	71 mm
Tare Wt.	30.73 g	Area	3959 mm ²
Wt. Water	57.50 g	Weight	1178.15 g
Dry Wt.	120.79 g	Strain	5.82 %
Moisture Cont.	47.6 %	Avg. Area	4204 mm ²
Wet Density	105.55 lb/ft³		16.58 kN/m³
Pocket Pen: Rdg	1.45 tsf	Torvane: Rdg	0.56 tsf
Su	1.45 ksf	Std vane Su	1.15 ksf
Su	69.4 kPa	Su	54.9 kPa
Qu: Displacement	10.25 mm	GeoPen: Rdg	kg
Load Cell	0.288 kN	10 mm tip Su	ksf
Su	34.3 kPa	10 mm tip Su	kPa
Su	0.72 ksf		

Test Hole	19-01	Depth	25 feet
		Sample No.	T5
Wet + Tare Wt.	231.83 g	Length	170 mm
Dry + Tare Wt.	165.20 g	Diameter	71 mm
Tare Wt.	31.28 g	Area	3959 mm ²
Wt. Water	66.63 g	Weight	1215.96 g
Dry Wt.	133.92 g	Strain	4.26 %
Moisture Cont.	49.8 %	Avg. Area	4136 mm ²
Wet Density	112.78 lb/ft³		17.72 kN/m³
Pocket Pen: Rdg	1.50 tsf	Torvane: Rdg	0.59 tsf
Su	1.50 ksf	Std vane Su	1.21 ksf
Su	71.8 kPa	Su	57.9 kPa
Qu: Displacement	7.25 mm	GeoPen: Rdg	kg
Load Cell	0.282 kN	10 mm tip Su	ksf
Su	34.1 kPa	10 mm tip Su	kPa
Su	0.71 ksf		

Test Hole	19-01	Depth	40 feet
		Sample No.	T8
Wet + Tare Wt.	243.69 g	Length	mm
Dry + Tare Wt.	192.72 g	Diameter	mm
Tare Wt.	31.10 g	Area	mm ²
Wt. Water	50.97 g	Weight	g
Dry Wt.	161.62 g	Strain	%
Moisture Cont.	31.5 %	Avg. Area	mm ²
Wet Density	lb/ft³		kN/m³
Pocket Pen: Rdg	0.75 tsf	Torvane: Rdg	0.50 tsf
Su	0.75 ksf	Std vane Su	1.02 ksf
Su	35.9 kPa	Su	49.0 kPa
Qu: Displacement	mm	GeoPen: Rdg	kg
Load Cell	kN	10 mm tip Su	ksf
Su	kPa	10 mm tip Su	kPa
Su	ksf		

Test Hole	19-02	Depth	20 feet
		Sample No.	T19
Wet + Tare Wt.	208.56 g	Length	171 mm
Dry + Tare Wt.	151.18 g	Diameter	72 mm
Tare Wt.	31.16 g	Area	4072 mm ²
Wt. Water	57.38 g	Weight	1204.54 g
Dry Wt.	120.02 g	Strain	7.16 %
Moisture Cont.	47.8 %	Avg. Area	4386 mm ²
Wet Density	108.01 lb/ft³		16.97 kN/m³
Pocket Pen: Rdg	1.50 tsf	Torvane: Rdg	0.55 tsf
Su	1.50 ksf	Std vane Su	1.13 ksf
Su	71.8 kPa	Su	53.9 kPa
Qu: Displacement	12.25 mm	GeoPen: Rdg	kg
Load Cell	0.554 kN	10 mm tip Su	ksf
Su	63.2 kPa	10 mm tip Su	kPa
Su	1.32 ksf		

Test Hole	19-03	Depth	30 feet
		Sample No.	T29
Wet + Tare Wt.	222.10 g	Length	170 mm
Dry + Tare Wt.	174.56 g	Diameter	71 mm
Tare Wt.	30.54 g	Area	3959 mm ²
Wt. Water	47.54 g	Weight	1236.40 g
Dry Wt.	144.02 g	Strain	6.62 %
Moisture Cont.	33.0 %	Avg. Area	4240 mm ²
Wet Density	114.68 lb/ft³		18.01 kN/m³
Pocket Pen: Rdg	0.85 tsf	Torvane: Rdg	0.50 tsf
Su	0.85 ksf	Std vane Su	1.02 ksf
Su	40.7 kPa	Su	49.0 kPa
Qu: Displacement	11.25 mm	GeoPen: Rdg	kg
Load Cell	0.278 kN	10 mm tip Su	ksf
Su	32.8 kPa	10 mm tip Su	kPa
Su	0.68 ksf		

Test Hole	19-05	Depth	15 feet
		Sample No.	T42
Wet + Tare Wt.	314.81 g	Length	176 mm
Dry + Tare Wt.	217.20 g	Diameter	70 mm
Tare Wt.	30.56 g	Area	3848 mm ²
Wt. Water	97.61 g	Weight	1193.74 g
Dry Wt.	186.64 g	Strain	4.55 %
Moisture Cont.	52.3 %	Avg. Area	4032 mm ²
Wet Density	110.02 lb/ft³		17.28 kN/m³
Pocket Pen: Rdg	1.40 tsf	Torvane: Rdg	0.60 tsf
Su	1.40 ksf	Std vane Su	1.23 ksf
Su	67.0 kPa	Su	58.8 kPa
Qu: Displacement	8.00 mm	GeoPen: Rdg	kg
Load Cell	0.380 kN	10 mm tip Su	ksf
Su	47.1 kPa	10 mm tip Su	kPa
Su	0.98 ksf		

PROJECT: Jefferson East CSR
AECOM PROJECT No.: 60599385

DATE: July 2019

Test Hole 19-05				Depth 25 feet			
				Sample No. T44			
Wet + Tare Wt.	223.96 g	Length	mm				
Dry + Tare Wt.	164.88 g	Diameter	mm				
Tare Wt.	30.67 g	Area	mm ²				
Wt. Water	59.08 g	Weight	g				
Dry Wt.	134.21 g	Strain	%				
Moisture Cont.	44.0 %	Avg. Area	mm²				
Wet Density	109.42 lb/ft³	17.19 kN/m³					
Pocket Pen: Rdg	1.65 tsf	Torvane: Rdg	0.60 tsf				
Su	1.65 ksf	Std vane Su	1.23 ksf				
Su	79.0 kPa	Su	58.8 kPa				
Qu: Displacement	mm	GeoPen: Rdg	kg				
Load Cell	kN	10 mm tip Su	0.00 ksf				
Su	kPa	10 mm tip Su	0.0 kPa				
Su	ksf						

Test Hole 19-06				Depth 20 feet			
				Sample No. T58			
Wet + Tare Wt.	199.52 g	Length	172 mm				
Dry + Tare Wt.	146.96 g	Diameter	72 mm				
Tare Wt.	30.89 g	Area	4072 mm ²				
Wt. Water	52.56 g	Weight	1217.33 g				
Dry Wt.	116.07 g	Strain	5.23 %				
Moisture Cont.	45.3 %	Avg. Area	4296 mm²				
Wet Density	108.52 lb/ft³	17.05 kN/m³					
Pocket Pen: Rdg	1.35 tsf	Torvane: Rdg	0.53 tsf				
Su	1.35 ksf	Std vane Su	1.09 ksf				
Su	64.6 kPa	Su	52.0 kPa				
Qu: Displacement	9.00 mm	GeoPen: Rdg	kg				
Load Cell	0.364 kN	10 mm tip Su	ksf				
Su	42.4 kPa	10 mm tip Su	kPa				
Su	0.88 ksf						

Test Hole 19-07				Depth 25 feet			
				Sample No. T67			
Wet + Tare Wt.	199.34 g	Length	175 mm				
Dry + Tare Wt.	144.83 g	Diameter	71 mm				
Tare Wt.	31.48 g	Area	3959 mm ²				
Wt. Water	54.51 g	Weight	1214.45 g				
Dry Wt.	113.35 g	Strain	5.29 %				
Moisture Cont.	48.1 %	Avg. Area	4180 mm²				
Wet Density	109.42 lb/ft³	17.19 kN/m³					
Pocket Pen: Rdg	1.50 tsf	Torvane: Rdg	0.60 tsf				
Su	1.50 ksf	Std vane Su	1.23 ksf				
Su	71.8 kPa	Su	58.8 kPa				
Qu: Displacement	9.25 mm	GeoPen: Rdg	kg				
Load Cell	0.388 kN	10 mm tip Su	ksf				
Su	46.4 kPa	10 mm tip Su	kPa				
Su	0.97 ksf						

Test Hole 19-08				Depth 20 feet			
				Sample No. T74			
Wet + Tare Wt.	205.36 g	Length	174 mm				
Dry + Tare Wt.	150.29 g	Diameter	71 mm				
Tare Wt.	31.05 g	Area	3959 mm ²				
Wt. Water	55.07 g	Weight	1191.08 g				
Dry Wt.	119.24 g	Strain	5.60 %				
Moisture Cont.	46.2 %	Avg. Area	4194 mm²				
Wet Density	107.94 lb/ft³	16.96 kN/m³					
Pocket Pen: Rdg	1.60 tsf	Torvane: Rdg	0.53 tsf				
Su	1.60 ksf	Std vane Su	1.09 ksf				
Su	76.6 kPa	Su	52.0 kPa				
Qu: Displacement	9.75 mm	GeoPen: Rdg	kg				
Load Cell	0.316 kN	10 mm tip Su	ksf				
Su	37.7 kPa	10 mm tip Su	kPa				
Su	0.79 ksf						

Test Hole 19-10				Depth 25 feet			
				Sample No. T95			
Wet + Tare Wt.	214.95 g	Length	170 mm				
Dry + Tare Wt.	157.80 g	Diameter	72 mm				
Tare Wt.	30.59 g	Area	4072 mm ²				
Wt. Water	57.15 g	Weight	1170.50 g				
Dry Wt.	127.21 g	Strain	5.44 %				
Moisture Cont.	44.9 %	Avg. Area	4306 mm²				
Wet Density	105.57 lb/ft³	16.58 kN/m³					
Pocket Pen: Rdg	1.45 tsf	Torvane: Rdg	0.54 tsf				
Su	1.45 ksf	Std vane Su	1.11 ksf				
Su	69.4 kPa	Su	53.0 kPa				
Qu: Displacement	9.25 mm	GeoPen: Rdg	kg				
Load Cell	0.427 kN	10 mm tip Su	ksf				
Su	49.6 kPa	10 mm tip Su	kPa				
Su	1.04 ksf						

Test Hole 19-11				Depth 15 feet			
				Sample No. T101			
Wet + Tare Wt.	215.72 g	Length	172 mm				
Dry + Tare Wt.	152.69 g	Diameter	72 mm				
Tare Wt.	30.56 g	Area	4072 mm ²				
Wt. Water	63.03 g	Weight	1162.57 g				
Dry Wt.	122.13 g	Strain	4.51 %				
Moisture Cont.	51.6 %	Avg. Area	4264 mm²				
Wet Density	103.64 lb/ft³	16.28 kN/m³					
Pocket Pen: Rdg	1.45 tsf	Torvane: Rdg	0.47 tsf				
Su	1.45 ksf	Std vane Su	0.96 ksf				
Su	69.4 kPa	Su	46.1 kPa				
Qu: Displacement	7.75 mm	GeoPen: Rdg	kg				
Load Cell	0.300 kN	10 mm tip Su	ksf				
Su	35.2 kPa	10 mm tip Su	kPa				
Su	0.73 ksf						

DYREGROV ROBINSON INC.

UNCONFINED COMPRESSION TEST

PROJECT: Jefferson East CSR
AECOM PROJECT No.: 60599385

DATE: July 2019

Test Hole	19-12	Depth	20 feet
		Sample No.	T114
Wet + Tare Wt.	212.23 g	Length	177 mm
Dry + Tare Wt.	149.10 g	Diameter	71 mm
Tare Wt.	30.44 g	Area	3959 mm ²
Wt. Water	63.13 g	Weight	1160.97 g
Dry Wt.	118.66 g	Strain	5.65 %
Moisture Cont.	53.2 %	Avg. Area	4196 mm ²
Wet Density	103.42 lb/ft³		16.25 kN/m³
Pocket Pen: Rdg	1.05 tsf	Torvane: Rdg	0.48 tsf
Su	1.05 ksf	Std vane Su	0.98 ksf
Su	50.3 kPa	Su	47.1 kPa
Qu: Displacement	10.00 mm	GeoPen: Rdg	kg
Load Cell	0.287 kN	10 mm tip Su	ksf
Su	34.2 kPa	10 mm tip Su	kPa
Su	0.71 ksf		

Test Hole	19-13	Depth	15 feet
		Sample No.	T121
Wet + Tare Wt.	221.59 g	Length	171 mm
Dry + Tare Wt.	153.91 g	Diameter	71 mm
Tare Wt.	31.42 g	Area	3959 mm ²
Wt. Water	67.68 g	Weight	1142.30 g
Dry Wt.	122.49 g	Strain	5.12 %
Moisture Cont.	55.3 %	Avg. Area	4173 mm ²
Wet Density	105.33 lb/ft³		16.55 kN/m³
Pocket Pen: Rdg	1.00 tsf	Torvane: Rdg	0.63 tsf
Su	1.00 ksf	Std vane Su	1.28 ksf
Su	47.9 kPa	Su	61.3 kPa
Qu: Displacement	8.75 mm	GeoPen: Rdg	kg
Load Cell	0.304 kN	10 mm tip Su	ksf
Su	36.4 kPa	10 mm tip Su	kPa
Su	0.76 ksf		

Test Hole	19-14	Depth	10 feet
		Sample No.	T128
Wet + Tare Wt.	246.30 g	Length	175 mm
Dry + Tare Wt.	176.16 g	Diameter	72 mm
Tare Wt.	30.90 g	Area	4072 mm ²
Wt. Water	70.14 g	Weight	1179.65 g
Dry Wt.	145.26 g	Strain	6.14 %
Moisture Cont.	48.3 %	Avg. Area	4338 mm ²
Wet Density	103.36 lb/ft³		16.24 kN/m³
Pocket Pen: Rdg	1.30 tsf	Torvane: Rdg	0.63 tsf
Su	1.30 ksf	Std vane Su	1.28 ksf
Su	62.2 kPa	Su	61.3 kPa
Qu: Displacement	10.75 mm	GeoPen: Rdg	kg
Load Cell	0.314 kN	10 mm tip Su	ksf
Su	36.2 kPa	10 mm tip Su	kPa
Su	0.76 ksf		

Test Hole	19-14	Depth	15 feet
		Sample No.	T129
Wet + Tare Wt.	184.35 g	Length	176 mm
Dry + Tare Wt.	133.37 g	Diameter	72 mm
Tare Wt.	30.52 g	Area	4072 mm ²
Wt. Water	50.98 g	Weight	1183.22 g
Dry Wt.	102.85 g	Strain	6.11 %
Moisture Cont.	49.6 %	Avg. Area	4336 mm ²
Wet Density	103.08 lb/ft³		16.19 kN/m³
Pocket Pen: Rdg	1.25 tsf	Torvane: Rdg	0.50 tsf
Su	1.25 ksf	Std vane Su	1.02 ksf
Su	59.9 kPa	Su	49.0 kPa
Qu: Displacement	10.75 mm	GeoPen: Rdg	kg
Load Cell	0.380 kN	10 mm tip Su	ksf
Su	43.8 kPa	10 mm tip Su	kPa
Su	0.92 ksf		

Test Hole	19-14	Depth	20 feet
		Sample No.	T130
Wet + Tare Wt.	323.84 g	Length	176 mm
Dry + Tare Wt.	230.29 g	Diameter	72 mm
Tare Wt.	31.11 g	Area	4072 mm ²
Wt. Water	93.55 g	Weight	1171.11 g
Dry Wt.	199.18 g	Strain	4.40 %
Moisture Cont.	47.0 %	Avg. Area	4259 mm ²
Wet Density	102.03 lb/ft³		16.03 kN/m³
Pocket Pen: Rdg	1.05 tsf	Torvane: Rdg	0.55 tsf
Su	1.05 ksf	Std vane Su	1.13 ksf
Su	50.3 kPa	Su	53.9 kPa
Qu: Displacement	7.75 mm	GeoPen: Rdg	kg
Load Cell	0.338 kN	10 mm tip Su	ksf
Su	39.7 kPa	10 mm tip Su	kPa
Su	0.83 ksf		

Test Hole	19-14	Depth	30 feet
		Sample No.	T132
Wet + Tare Wt.	208.64 g	Length	158 mm
Dry + Tare Wt.	153.44 g	Diameter	73 mm
Tare Wt.	31.30 g	Area	4185 mm ²
Wt. Water	55.20 g	Weight	1084.67 g
Dry Wt.	122.14 g	Strain	4.91 %
Moisture Cont.	45.2 %	Avg. Area	4401 mm ²
Wet Density	102.40 lb/ft³		16.09 kN/m³
Pocket Pen: Rdg	0.80 tsf	Torvane: Rdg	0.53 tsf
Su	0.80 ksf	Std vane Su	1.08 ksf
Su	38.3 kPa	Su	51.5 kPa
Qu: Displacement	7.75 mm	GeoPen: Rdg	kg
Load Cell	0.417 kN	10 mm tip Su	ksf
Su	47.4 kPa	10 mm tip Su	kPa
Su	0.99 ksf		

PROJECT: Jefferson East CSR
AECOM PROJECT No.: 60599385

DATE: July 2019

Test Hole	19-14	Depth	40	feet
		Sample No.	T134	
Wet + Tare Wt.	287.03 g	Length	161 mm	
Dry + Tare Wt.	196.75 g	Diameter	72 mm	
Tare Wt.	31.21 g	Area	4072 mm ²	
Wt. Water	90.28 g	Weight	1079.26 g	
Dry Wt.	165.54 g	Strain	2.02 %	
Moisture Cont.	54.5 %	Avg. Area	4155 mm ²	
Wet Density	102.78 lb/ft³		16.15 kN/m³	
Pocket Pen: Rdg	0.85 tsf	Torvane: Rdg	0.50 tsf	
Su	0.85 ksf	Std vane Su	1.02 ksf	
Su	40.7 kPa	Su	49.0 kPa	
Qu: Displacement	3.25 mm	GeoPen: Rdg	kg	
Load Cell	0.140 kN	10 mm tip Su	ksf	
Su	16.8 kPa	<i>10 mm tip Su</i>	<i>kPa</i>	
Su	0.35 ksf			

Test Hole	19-15	Depth	5	feet
		Sample No.	T139	
Wet + Tare Wt.	217.25 g	Length	mm	
Dry + Tare Wt.	195.20 g	Diameter	mm	
Tare Wt.	31.49 g	Area	mm ²	
Wt. Water	22.05 g	Weight	g	
Dry Wt.	163.71 g	Strain	%	
Moisture Cont.	13.5 %	Avg. Area	mm ²	
Wet Density	lb/ft ³		kN/m ³	
Pocket Pen: Rdg	tsf	Torvane: Rdg	tsf	
Su	ksf	Std vane Su	ksf	
Su	kPa	Su	kPa	
Qu: Displacement	mm	GeoPen: Rdg	kg	
Load Cell	kN	10 mm tip Su	ksf	
Su	kPa	<i>10 mm tip Su</i>	<i>kPa</i>	
Su	ksf			

Test Hole	19-15	Depth	10	feet
		Sample No.	T140	
Wet + Tare Wt.	166.50 g	Length	176 mm	
Dry + Tare Wt.	121.39 g	Diameter	72 mm	
Tare Wt.	31.07 g	Area	4072 mm ²	
Wt. Water	45.11 g	Weight	1164.05 g	
Dry Wt.	90.32 g	Strain	7.39 %	
Moisture Cont.	49.9 %	Avg. Area	4396 mm ²	
Wet Density	101.41 lb/ft³		15.93 kN/m³	
Pocket Pen: Rdg	1.25 tsf	Torvane: Rdg	0.58 tsf	
Su	1.25 ksf	Std vane Su	1.18 ksf	
Su	59.9 kPa	Su	56.4 kPa	
Qu: Displacement	13.00 mm	GeoPen: Rdg	kg	
Load Cell	0.477 kN	10 mm tip Su	ksf	
Su	54.3 kPa	<i>10 mm tip Su</i>	<i>kPa</i>	
Su	1.13 ksf			

Test Hole	19-15	Depth	15	feet
		Sample No.	T141	
Wet + Tare Wt.	244.03 g	Length	176 mm	
Dry + Tare Wt.	177.53 g	Diameter	72 mm	
Tare Wt.	30.55 g	Area	4072 mm ²	
Wt. Water	66.50 g	Weight	1162.97 g	
Dry Wt.	146.98 g	Strain	5.82 %	
Moisture Cont.	45.2 %	Avg. Area	4323 mm ²	
Wet Density	101.32 lb/ft³		15.92 kN/m³	
Pocket Pen: Rdg	0.95 tsf	Torvane: Rdg	0.68 tsf	
Su	0.95 ksf	Std vane Su	1.38 ksf	
Su	45.5 kPa	Su	66.2 kPa	
Qu: Displacement	10.25 mm	GeoPen: Rdg	kg	
Load Cell	0.302 kN	10 mm tip Su	ksf	
Su	34.9 kPa	<i>10 mm tip Su</i>	<i>kPa</i>	
Su	0.73 ksf			

Test Hole	19-15	Depth	25	feet
		Sample No.	T143	
Wet + Tare Wt.	222.36 g	Length	171 mm	
Dry + Tare Wt.	160.00 g	Diameter	71 mm	
Tare Wt.	30.67 g	Area	3959 mm ²	
Wt. Water	62.36 g	Weight	1158.32 g	
Dry Wt.	129.33 g	Strain	3.65 %	
Moisture Cont.	48.2 %	Avg. Area	4109 mm ²	
Wet Density	106.81 lb/ft³		16.78 kN/m³	
Pocket Pen: Rdg	1.00 tsf	Torvane: Rdg	0.35 tsf	
Su	1.00 ksf	Std vane Su	0.72 ksf	
Su	47.9 kPa	Su	34.3 kPa	
Qu: Displacement	6.25 mm	GeoPen: Rdg	kg	
Load Cell	0.306 kN	10 mm tip Su	ksf	
Su	37.2 kPa	<i>10 mm tip Su</i>	<i>kPa</i>	
Su	0.78 ksf			

Test Hole	19-15	Depth	35	feet
		Sample No.	T145	
Wet + Tare Wt.	266.96 g	Length	mm	
Dry + Tare Wt.	197.42 g	Diameter	mm	
Tare Wt.	30.60 g	Area	mm ²	
Wt. Water	69.54 g	Weight	g	
Dry Wt.	166.82 g	Strain	%	
Moisture Cont.	41.7 %	Avg. Area	mm ²	
Wet Density	lb/ft ³		kN/m ³	
Pocket Pen: Rdg	tsf	Torvane: Rdg	tsf	
Su	0.00 ksf	Std vane Su	0.00 ksf	
Su	0.0 kPa	Su	0.0 kPa	
Qu: Displacement	mm	GeoPen: Rdg	kg	
Load Cell	kN	10 mm tip Su	ksf	
Su	kPa	<i>10 mm tip Su</i>	<i>kPa</i>	
Su	ksf			

DYREGROV ROBINSON INC.

UNCONFINED COMPRESSION TEST

PROJECT: Jefferson East CSR
AECOM PROJECT No.: 60599385

DATE: July 2019

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PROJECT: Jefferson East CSR
AECOM PROJECT No.: 60599385

DATE: July 2019

<u>Test Hole</u>	<u>19-17</u>	<u>Depth</u>	<u>35</u>	<u>feet</u>
		<u>Sample No.</u>	<u>T164</u>	
Wet + Tare Wt.	320.44 g	Length	176 mm	
Dry + Tare Wt.	235.45 g	Diameter	71 mm	
Tare Wt.	31.27 g	Area	3959 mm ²	
Wt. Water	84.99 g	Weight	1246.37 g	
Dry Wt.	204.18 g	Strain	5.97 %	
Moisture Cont.	41.6 %	Avg. Area	4210 mm ²	
Wet Density	111.66 lb/ft³		17.54 kN/m³	
Pocket Pen:	Rdg 0.65 tsf	Torvane:	Rdg 0.26 tsf	
	Su 0.65 ksf	Std vane Su	0.53 ksf	
	Su 31.1 kPa	Su	25.5 kPa	
Qu:	Displacement 10.50 mm	GeoPen:	Rdg kg	
	Load Cell 0.322 kN	10 mm tip Su	ksf	
	Su 38.2 kPa	<i>10 mm tip Su</i>	<i>kPa</i>	
	Su 0.80 ksf			

<u>Test Hole</u>	<u>Depth</u>	<u>feet</u>
	<u>Sample No.</u>	
Wet + Tare Wt.	g	Length mm
Dry + Tare Wt.	g	Diameter mm
Tare Wt.	g	Area mm ²
Wt. Water	g	Weight g
Dry Wt.	g	Strain %
Moisture Cont.	%	Avg. Area mm ²
Wet Density	lb/ft ³	kN/m³
Pocket Pen:	Rdg tsf	Torvane: Rdg tsf
	Su ksf	Std vane Su ksf
	Su kPa	Su kPa
Qu:	Displacement mm	GeoPen: Rdg kg
	Load Cell kN	10 mm tip Su ksf
	Su kPa	<i>10 mm tip Su</i> <i>kPa</i>
	Su ksf	

<u>Test Hole</u>	<u>Depth</u>	<u>feet</u>
	<u>Sample No.</u>	
Wet + Tare Wt.	g	Length mm
Dry + Tare Wt.	g	Diameter mm
Tare Wt.	g	Area mm ²
Wt. Water	g	Weight g
Dry Wt.	g	Strain %
Moisture Cont.	%	Avg. Area mm ²
Wet Density	lb/ft ³	0.00
Pocket Pen:	Rdg tsf	Torvane: Rdg tsf
	Su ksf	Std vane Su ksf
	Su kPa	Su kPa
Qu:	Displacement mm	GeoPen: Rdg kg
	Load Cell kN	10 mm tip Su ksf
	Su kPa	<i>10 mm tip Su</i> <i>kPa</i>
	Su ksf	

<u>Test Hole</u>	<u>Depth</u>	<u>feet</u>
	<u>Sample No.</u>	
Wet + Tare Wt.	g	Length mm
Dry + Tare Wt.	g	Diameter mm
Tare Wt.	g	Area mm ²
Wt. Water	g	Weight g
Dry Wt.	g	Strain %
Moisture Cont.	%	Avg. Area mm ²
Wet Density	lb/ft ³	0.00
Pocket Pen:	Rdg tsf	Torvane: Rdg tsf
	Su ksf	Std vane Su ksf
	Su kPa	Su kPa
Qu:	Displacement mm	GeoPen: Rdg kg
	Load Cell kN	10 mm tip Su ksf
	Su kPa	<i>10 mm tip Su</i> <i>kPa</i>
	Su ksf	

<u>Test Hole</u>	<u>Depth</u>	<u>feet</u>
	<u>Sample No.</u>	
Wet + Tare Wt.	g	Length mm
Dry + Tare Wt.	g	Diameter mm
Tare Wt.	g	Area mm ²
Wt. Water	g	Weight g
Dry Wt.	g	Strain %
Moisture Cont.	%	Avg. Area mm ²
Wet Density	lb/ft ³	0.00
Pocket Pen:	Rdg tsf	Torvane: Rdg tsf
	Su ksf	Std vane Su ksf
	Su kPa	Su kPa
Qu:	Displacement mm	GeoPen: Rdg kg
	Load Cell kN	10 mm tip Su ksf
	Su kPa	<i>10 mm tip Su</i> <i>kPa</i>
	Su ksf	

<u>Test Hole</u>	<u>Depth</u>	<u>feet</u>
	<u>Sample No.</u>	
Wet + Tare Wt.	g	Length mm
Dry + Tare Wt.	g	Diameter mm
Tare Wt.	g	Area mm ²
Wt. Water	g	Weight g
Dry Wt.	g	Strain %
Moisture Cont.	%	Avg. Area mm ²
Wet Density	lb/ft ³	0.00
Pocket Pen:	Rdg tsf	Torvane: Rdg tsf
	Su ksf	Std vane Su ksf
	Su kPa	Su kPa
Qu:	Displacement mm	GeoPen: Rdg kg
	Load Cell kN	10 mm tip Su ksf
	Su kPa	<i>10 mm tip Su</i> <i>kPa</i>
	Su ksf	

HYDRAULIC CONDUCTIVITY TEST REPORT

CLIENT: Aecom PROJECT NO. 112-1909
99 Commerce Drive
Winnipeg, MB R3P 0Y7
ATTENTION: Ryan Harras
PROJECT: Jefferson East CSR (Phase 2)

Date Sampled: 24-27-Jun-19 Date Received: 26-Jul-19 Sampled By: Client
Test Started: 26-Jul-19 Test Ended: 15-Aug-19 Sample ID: TH 19-08, T74

Test Result
Corrected Saturated Hydraulic Conductivity, K_s (cm/sec) 1.52×10^{-8}

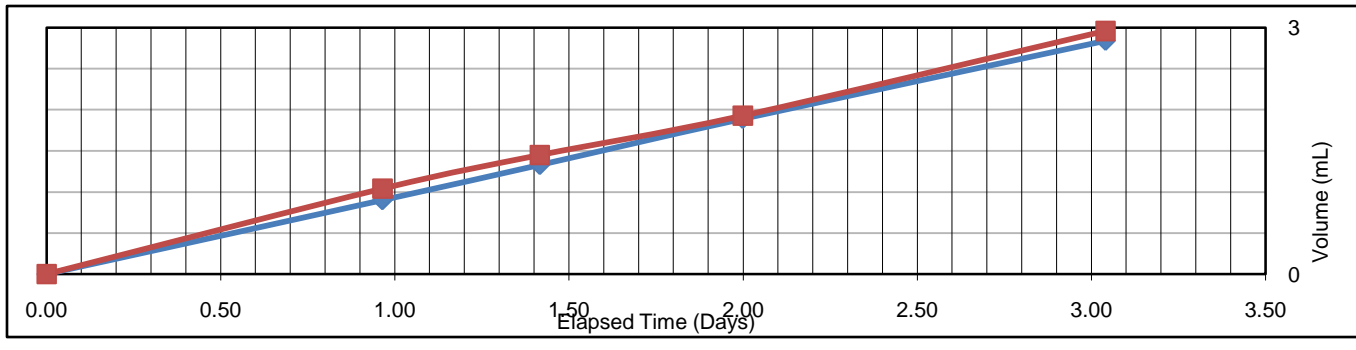
Consolidation Data

	Avg. Height (m)	Avg. Diameter (m)	Moisture Content %	Degree of Saturation %	Cell Pressure kPa	Back Pressure kPa
Initial	0.058	0.072	50.6	95.1	120.0	100.0
Final	0.059	0.072	53.8	99.2	120.0	100.0

Permeation Data

Time Increment (Days)	Elapsed Time (Days)	Q (ml)		In/Out Ratio	Average Flow (ml)	Temperature Correction	Corrected Conductivity, K_s (m/s)
		In	Out				
0.96	0.96	0.90	1.04	0.865	0.97	0.95	1.62E-10
0.45	1.42	0.43	0.41	1.049	0.42	0.95	1.50E-10
0.58	2.00	0.56	0.48	1.167	0.52	0.95	1.44E-10
1.04	3.04	0.95	1.03	0.922	0.99	0.95	1.53E-10

Permeant: De-aired tap water
Hydraulic Gradient: 17.30



Comments
Specific gravity of soil was assumed to be 2.75

Remarks: Test Method: ASTM D5084 (Constant Head)
Technician: NS

P. Bevel
Reviewed by: Paul Bevel

HYDRAULIC CONDUCTIVITY TEST REPORT

CLIENT: Aecom 99 Commerce Drive Winnipeg, MB R3P 0Y7	PROJECT NO. 112-1909
ATTENTION: Ryan Harras	
PROJECT: Jefferson East CSR (Phase 2)	

Date Sampled: 24-27-Jun-19	Date Received: 26-Jul-19	Sampled By: Client
Test Started: 26-Jul-19	Test Ended: 15-Aug-19	Sample ID: TH 19-08, T74

Test Result

Corrected Saturated Hydraulic Conductivity, Ks (cm/sec) 1.52×10^{-8}

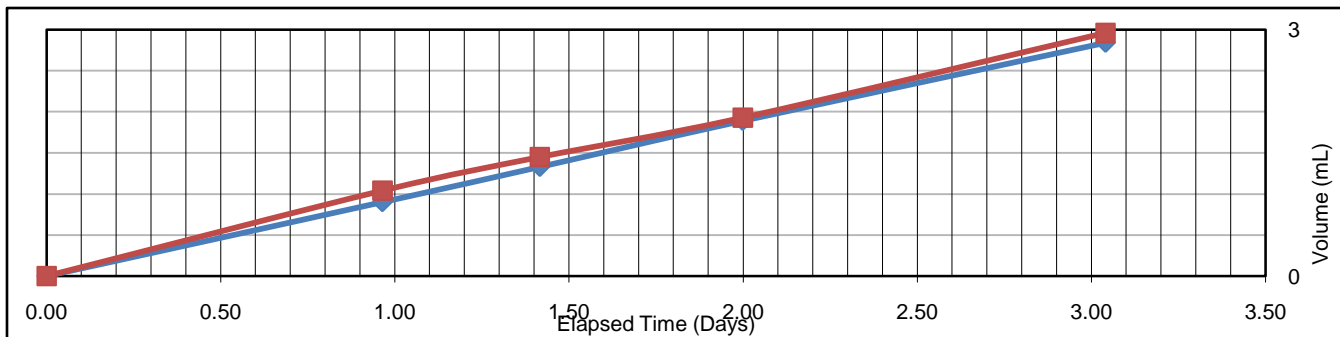
Consolidation Data

	Avg. Height (m)	Avg. Diameter (m)	Moisture Content %	Degree of Saturation %	Cell Pressure kPa	Back Pressure kPa
Initial	0.058	0.072	50.6	95.1	120.0	100.0
Final	0.059	0.072	53.8	99.2	120.0	100.0

Permeation Data

Time Increment (Days)	Elapsed Time (Days)	Q (ml)		In/Out Ratio	Average Flow (ml)	Temperature Correction	Corrected Conductivity, Ks (m/s)
		In	Out				
0.96	0.96	0.90	1.04	0.865	0.97	0.95	1.62E-10
0.45	1.42	0.43	0.41	1.049	0.42	0.95	1.50E-10
0.58	2.00	0.56	0.48	1.167	0.52	0.95	1.44E-10
1.04	3.04	0.95	1.03	0.922	0.99	0.95	1.53E-10

Permeant: De-aired tap water
Hydraulic Gradient: 17.30



Comments

Specific gravity of soil was assumed to be 2.75

Remarks: Test Method: ASTM D5084 (Constant Head)
Technician: NS

P. Bevel
Reviewed by: Paul Bevel

HYDRAULIC CONDUCTIVITY TEST REPORT

CLIENT: Aecom	PROJECT NO. 112-1909
99 Commerce Drive	
Winnipeg, MB R3P 0Y4	
ATTENTION: Ryan Harras	
PROJECT: Jefferson East CSR (Phase 2)	

Date Sampled: June 24-27	Date Received: 26-Jul-19	Sampled By: Client
Test Started: 05-Aug-19	Test Ended: 26-Aug-19	Sample ID: TH 19-15 (T140)

Test Result

Corrected Saturated Hydraulic Conductivity, Ks (cm/sec) 2.98×10^{-8}

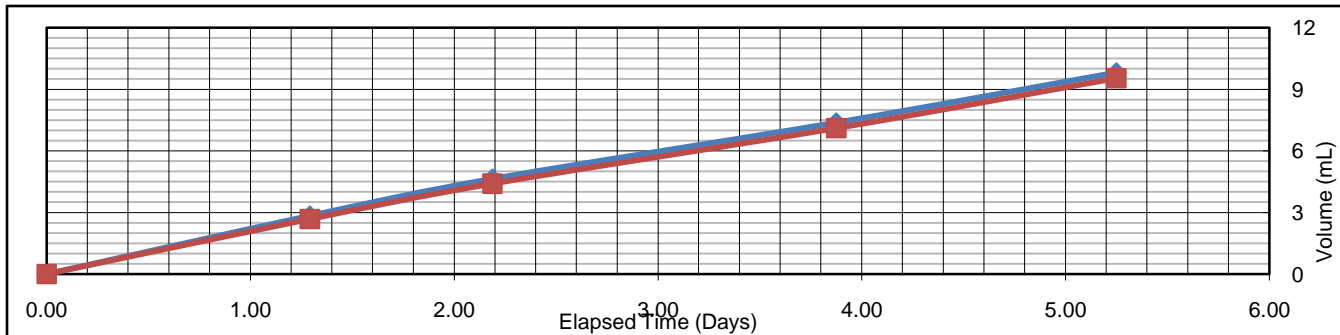
Consolidation Data

	Avg. Height (m)	Avg. Diameter (m)	Moisture Content %	Degree of Saturation %	Cell Pressure kPa	Back Pressure kPa
Initial	0.086	0.070	50.8	92.8	130.0	100.0
Final	0.087	0.071	58.8	99.1	130.0	100.0

Permeation Data

Time Increment (Days)	Elapsed Time (Days)	Q (ml)		In/Out Ratio	Average Flow (ml)	Temperature Correction	Corrected Conductivity, Ks (m/s)
		In	Out				
1.29	1.29	2.84	2.68	1.060	2.76	0.95	3.40E-10
0.90	2.19	1.80	1.72	1.047	1.76	0.95	3.13E-10
1.69	3.88	2.74	2.70	1.015	2.72	0.95	2.57E-10
1.38	5.25	2.44	2.43	1.004	2.44	0.95	2.82E-10

Permeant: De-aired tap water
Hydraulic Gradient: 17.54



Comments

Specific gravity of soil was assumed to be 2.75

Remarks: Test Method: ASTM D5084 (Constant Head)
Technician: NS

P. Bevel
Reviewed by: Paul Bevel

SWELL TEST REPORT

ASTM D4546-14 TEST METHOD A

Client AECOM C/O Dyregrov Robinson Inc.
 Project Jefferson East CST (Phase 2)
 Project No. WX11735

Test Hole TH19-02 Test Start: 5-Sep-19
 Sample T19 Tested By: NM
 Depth 20 ft

Before Test

Consolidation ring no. (new) #4
 Mass of ring 110.17 g
 Inside diameter of the ring 6.367 cm
 Height of the specimen, H_0 2.474 cm
 Area of the specimen 31.839 cm²
 Mass (specimen + ring) 246.23 g
 Mass of wet sample 136.1 g
 Initial Moisture Content 39.3%

After Test

Mass(sample_{wet}+ring+tare) 360.87 g
 Mass of tare 114.46 g
 Mass (wet soil + ring) 246.41 g
 Mass of wet sample 136.24 g
 Mass (dry soil+ring+can) 322.27 g
 Mass of dry specimen 97.64 g
 Final MC of specimen 39.5%
 Specific gravity of Solids 2.7
 Seating pressure 1 kPa

Soil Properties

Mass of solids 97.64 g
 Mass of water in specimen before test 38.42 g
 Mass of water in specimen after test 38.60 g
 Height of Solids 1.1358 cm
 Height of water before test 1.2067 cm
 Height of water after test 1.2123 cm
 Change in height of specimen after test 0.0259 cm
 Height of specimen after test 2.4481 cm
 Void ratio before test 1.178
 Void ratio after test 1.155
 Degree of saturation before test 90.17%
 Degree of saturation after test 92.38%
 Dry Density before test 1.240 g/cm³

Visual Description of Soil

Clay (CH) - silty, trace sand, high plastic, moist,
dark greyish brown

void ratio, e vs. log pressure

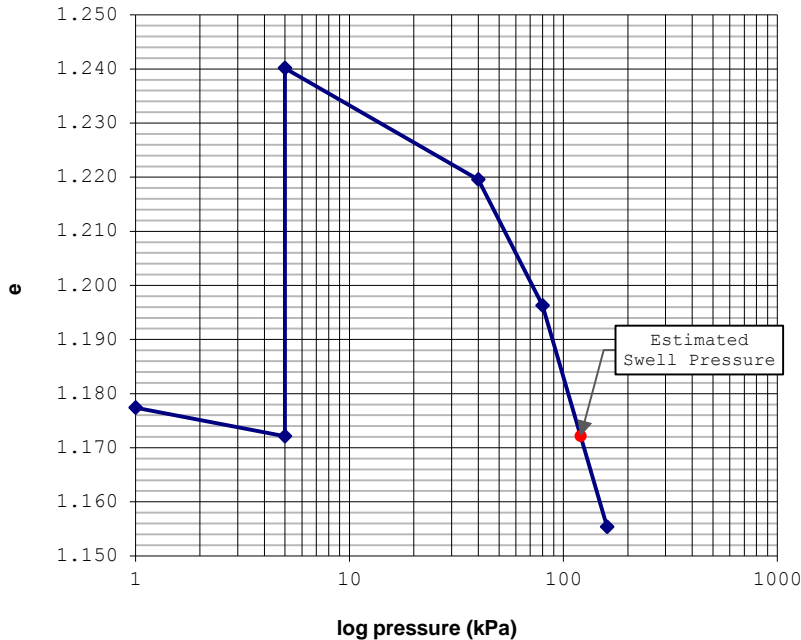


TABLE 1: Test Summary

Load No.	Pressure	Void Ratio
Seating	1	1.177
1	5	1.172
2	5	1.240
3	40	1.220
4	80	1.196
5	160	1.155

Final Results:

Swell (+) / Collpase (-) Strain = 3.1% Swell
 Estimated Swell Pressure = 120 kPa

SWELL TEST REPORT

ASTM D4546-14 TEST METHOD B

Client AECOM C/O Dyregrov Robinson Inc.
 Project Jefferson East CST (Phase 2)
 Project No. WX11735

Test Hole TH19-06 Test Start: 6-Sep-19
 Sample T19 Tested By: NM
 Depth 20 ft

Before Test

Consolidation ring no. **#12**
 Mass of ring **90.64 g**
 Inside diameter of the ring **6.494 cm**
 Height of the specimen, H_0 **2.324 cm**
 Area of the specimen **33.122 cm²**
 Mass (specimen + ring) **224.19 g**
 Mass of wet sample **133.6 g**
 Initial Moisture Content **42.0%**

After Test

Mass(sample_{wet}+ring+tare) **340.60 g**
 Mass of tare **114.74 g**
 Mass (wet soil + ring) **225.86 g**
 Mass of wet sample **135.22 g**
 Mass (dry soil+ring+can) **299.44 g**
 Mass of dry specimen **94.06 g**
 Final MC of specimen **43.8%**
 Specific gravity of Solids **2.7**
 Seating pressure **1 kPa**

Soil Properties

Mass of solids **94.06 g**
 Mass of water in specimen before test **39.49 g**
 Mass of water in specimen after test **41.16 g**
 Height of Solids **1.0518 cm**
 Height of water before test **1.1923 cm**
 Height of water after test **1.2427 cm**
 Change in height of specimen after test **0.0216 cm**
 Height of specimen after test **2.3024 cm**
 Void ratio before test **1.210**
 Void ratio after test **1.189**
 Degree of saturation before test **93.72%**
 Degree of saturation after test **99.37%**
 Dry Density before test **1.222 g/cm³**

Visual Description of Soil

Clay (CH) - silty, trace sand, high plastic, moist,
dark greyish brown

void ratio, e vs. log pressure

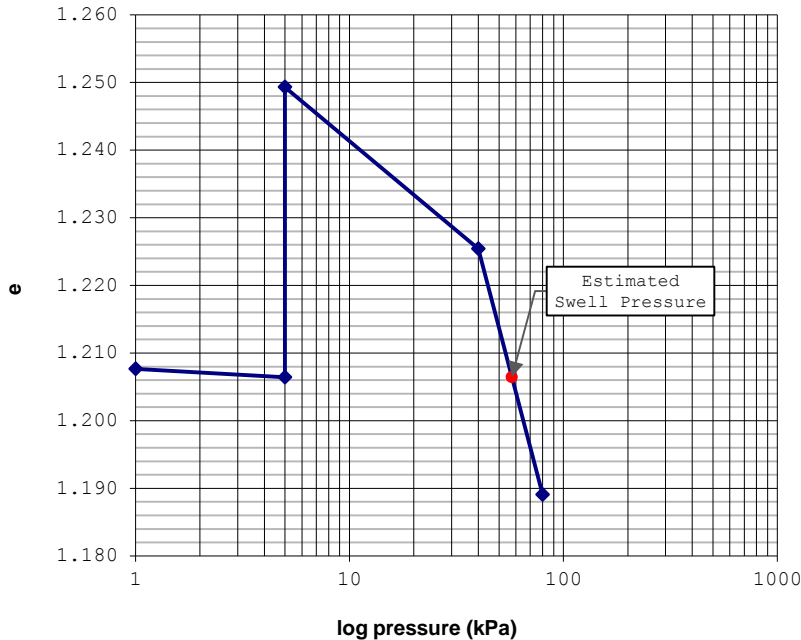


TABLE 1: Test Summary

Load No.	Pressure	Void Ratio
Seating	1	1.208
1	5	1.206
2	5	1.249
3	40	1.225
4	80	1.189

Final Results:

Swell (+) / Collpase (-) Strain = **1.9%** Swell
 Estimated Swell Pressure = **57** kPa

SWELL TEST REPORT

ASTM D4546-14 TEST METHOD B

Client AECOM C/O Dyregrov Robinson Inc.
 Project Jefferson East CST (Phase 2)
 Project No. WX11735

Test Hole TH19-11 Test Start: 13-Sep-19
 Sample T101 Tested By: NM
 Depth 15 ft

Before Test

Consolidation ring no. **#12**
 Mass of ring **90.64 g**
 Inside diameter of the ring **6.494 cm**
 Height of the specimen, H_o **2.386 cm**
 Area of the specimen **33.122 cm²**
 Mass (specimen + ring) **224.85 g**
 Mass of wet sample **134.2 g**
 Initial Moisture Content **34.3%**

After Test

Mass(sample_{wet}+ring+tare) **341.47 g**
 Mass of tare **114.74 g**
 Mass (wet soil + ring) **226.73 g**
 Mass of wet sample **136.09 g**
 Mass (dry soil+ring+can) **305.35 g**
 Mass of dry specimen **99.97 g**
 Final MC of specimen **36.1%**
 Specific gravity of Solids **2.7**
 Seating pressure **1 kPa**

Soil Properties

Mass of solids **99.97 g**
 Mass of water in specimen before test **34.24 g**
 Mass of water in specimen after test **36.12 g**
 Height of Solids **1.1179 cm**
 Height of water before test **1.0338 cm**
 Height of water after test **1.0905 cm**
 Change in height of specimen after test **0.0244 cm**
 Height of specimen after test **2.3616 cm**
 Void ratio before test **1.134**
 Void ratio after test **1.113**
 Degree of sat **81.52%**
 Degree of saturation after test **87.68%**
 Dry Density before test **1.265 g/cm³**

Visual Description of Soil

Clay (CH) - silty, trace sand, high plastic, moist,
dark greyish brown

void ratio, e vs. log pressure

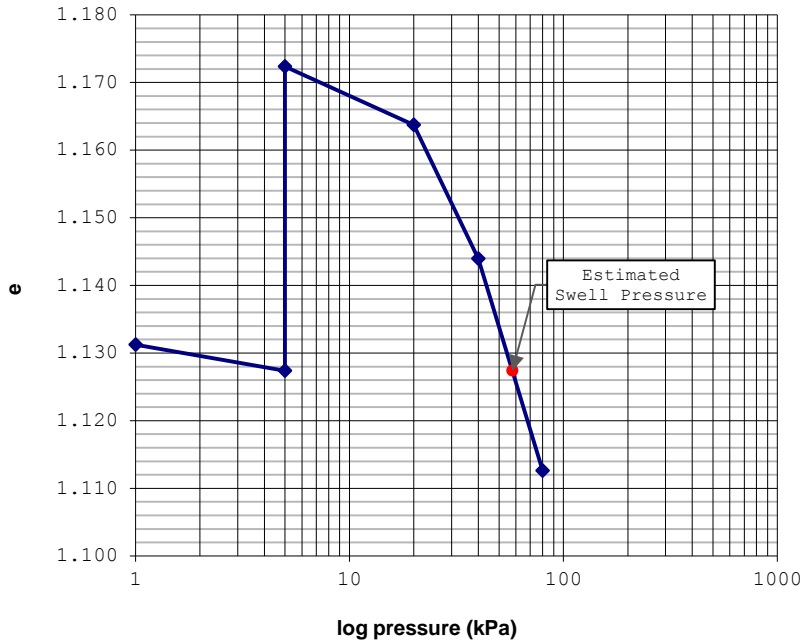


TABLE 1: Test Summary

Load No.	Pressure	Void Ratio
Seating	1	1.131
1	5	1.127
2	5	1.172
3	20	1.164
4	40	1.144
5	80	1.113

Final Results:

Swell (+) / Collpase (-) Strain = **2.1%** Swell
 Estimated Swell Pressure = **58** kPa

SWELL TEST REPORT

ASTM D4546-14 TEST METHOD A

Client AECOM C/O Dyregrov Robinson Inc.
 Project Jefferson East CST (Phase 2)
 Project No. WX11735

Test Hole TH19-14 Test Start: 13-Sep-19
 Sample T130 Tested By: NM
 Depth 20 ft

Before Test

Consolidation ring no. **(new) #4**
 Mass of ring **110.17 g**
 Inside diameter of the ring **6.366 cm**
 Height of the specimen, H_o **2.449 cm**
 Area of the specimen **31.829 cm²**
 Mass (specimen + ring) **238.47 g**
 Mass of wet sample **128.3 g**
 Initial Moisture Content **56.2%**

After Test

Mass(sample_{wet}+ring+tare) **353.94 g**
 Mass of tare **114.23 g**
 Mass (wet soil + ring) **239.71 g**
 Mass of wet sample **129.54 g**
 Mass (dry soil+ring+can) **306.54 g**
 Mass of dry specimen **82.14 g**
 Final MC of specimen **57.7%**
 Specific gravity of Solids **2.7**
 Seating pressure **1 kPa**

Soil Properties

Mass of solids **82.14 g**
 Mass of water in specimen before test **46.16 g**
 Mass of water in specimen after test **47.40 g**
 Height of Solids **0.9558 cm**
 Height of water before test **1.4502 cm**
 Height of water after test **1.4892 cm**
 Change in height of specimen after test **cm**
 Height of specimen after test **2.4490 cm**
 Void ratio before test **1.562**
 Void ratio after test **1.562**
 Degree of saturation before test **97.12%**
 Degree of saturation after test **99.73%**
 Dry Density before test **1.054 g/cm³**

Visual Description of Soil

Clay (CH) - silty, trace sand, high plastic, moist,
dark greyish brown

void ratio, e vs. log pressure

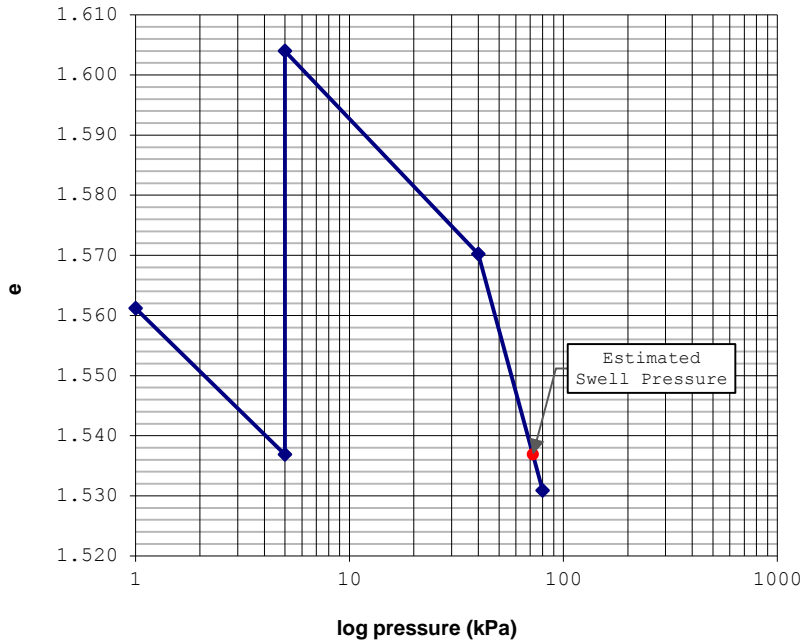


TABLE 1: Test Summary

Load No.	Pressure	Void Ratio
Seating	1	1.561
1	5	1.537
2	5	1.604
3	40	1.570
4	80	1.531

Final Results:

Swell (+) / Collpase (-) Strain = **2.6%** Swell
 Estimated Swell Pressure = **72** kPa

SWELL TEST REPORT

ASTM D4546-14 TEST METHOD B

Client AECOM C/O Dyregrov Robinson Inc.
 Project Jefferson East CST (Phase 2)
 Project No. WX11735

Test Hole TH19-16 Test Start: 13-Sep-19
 Sample T147 Tested By: NM
 Depth 10 ft

Before Test

Consolidation ring no. **#12**
 Mass of ring **110.19 g**
 Inside diameter of the ring **6.367 cm**
 Height of the specimen, H_o **2.438 cm**
 Area of the specimen **31.839 cm²**
 Mass (specimen + ring) **239.18 g**
 Mass of wet sample **129.0 g**
 Initial Moisture Content **48.6%**

After Test

Mass(sample_{wet}+ring+tare) **353.80 g**
 Mass of tare **114.30 g**
 Mass (wet soil + ring) **239.50 g**
 Mass of wet sample **129.31 g**
 Mass (dry soil+ring+can) **311.27 g**
 Mass of dry specimen **86.78 g**
 Final MC of specimen **49.0%**
 Specific gravity of Solids **2.7**
 Seating pressure **1 kPa**

Soil Properties

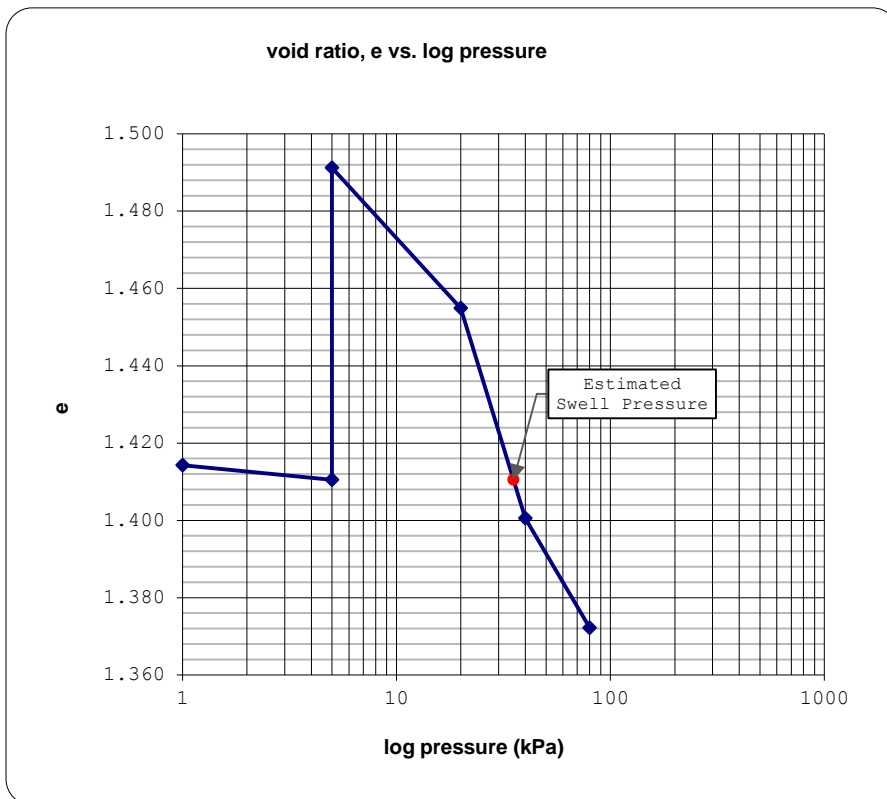
Mass of solids **86.78 g**
 Mass of water in specimen before test **42.21 g**
 Mass of water in specimen after test **42.53 g**
 Height of Solids **1.0095 cm**
 Height of water before test **1.3257 cm**
 Height of water after test **1.3358 cm**
 Change in height of specimen after test **0.0433 cm**
 Height of specimen after test **2.3947 cm**
 Void ratio before test **1.415**
 Void ratio after test **1.372**
 Degree of sat **92.80%**
 Degree of saturation after test **96.43%**
 Dry Density before test **1.118 g/cm³**

Visual Description of Soil

Clay (CH) - silty, trace sand, high plastic, moist,
dark greyish brown

TABLE 1: Test Summary

Load No.	Pressure	Void Ratio
Seating	1	1.414
1	5	1.410
2	5	1.491
3	20	1.455
4	40	1.401
5	80	1.372



Final Results:

Swell (+) / Collpase (-) Strain = **3.4%** Swell
 Estimated Swell Pressure = **35** kPa



AECOM Canada Ltd.
ATTN: RYAN HARRAS
99 Commerce Drive
Winnipeg MB R3P 0Y7

Date Received: 29-JUL-19
Report Date: 08-AUG-19 14:00 (MT)
Version: FINAL

Client Phone: 204-928-7444

Certificate of Analysis

Lab Work Order #: L2318801
Project P.O. #: 60599385
Job Reference: 60599385
C of C Numbers:
Legal Site Desc:

Hua Wo
Chemistry Laboratory Manager

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ADDRESS: 1329 Niakwa Road East, Unit 12, Winnipeg, MB R2J 3T4 Canada | Phone: +1 204 255 9720 | Fax: +1 204 255 9721
ALS CANADA LTD Part of the ALS Group An ALS Limited Company

ALS ENVIRONMENTAL ANALYTICAL REPORT

Sample Details/Parameters	Result	Qualifier*	D.L.	Units	Extracted	Analyzed	Batch
L2318801-1 TH19-01; G3 @ 15' Sampled By: CLIENT on 24-JUL-19 Matrix: SOIL Miscellaneous Parameters							
% Moisture	31.8		0.10	%	01-AUG-19	02-AUG-19	R4737126
Resistivity	561		1.0	ohm*cm		08-AUG-19	
Sulphate	927		20	mg/kg	02-AUG-19	06-AUG-19	R4742614
pH (1:2 soil:water)	7.96		0.10	pH	07-AUG-19	07-AUG-19	R4740588
Conductivity	1.78		0.0040	mS/cm		08-AUG-19	R4743353
L2318801-2 TH19-05; G43 @ 20' Sampled By: CLIENT on 24-JUL-19 Matrix: SOIL Miscellaneous Parameters							
% Moisture	31.7		0.10	%	01-AUG-19	02-AUG-19	R4737126
Resistivity	1400		1.0	ohm*cm		08-AUG-19	
Sulphate	511		20	mg/kg	02-AUG-19	06-AUG-19	R4742614
pH (1:2 soil:water)	8.12		0.10	pH	07-AUG-19	07-AUG-19	R4740588
Conductivity	0.713		0.0040	mS/cm		08-AUG-19	R4743353
L2318801-3 TH19-10; G91 @ 5' Sampled By: CLIENT on 24-JUL-19 Matrix: SOIL Miscellaneous Parameters							
% Moisture	16.8		0.10	%	01-AUG-19	02-AUG-19	R4737126
Resistivity	4950		1.0	ohm*cm		08-AUG-19	
Sulphate	46		20	mg/kg	02-AUG-19	06-AUG-19	R4742614
pH (1:2 soil:water)	9.13		0.10	pH	07-AUG-19	07-AUG-19	R4740588
Conductivity	0.202		0.0040	mS/cm		08-AUG-19	R4743353
L2318801-4 TH19-13; G120 @ 10' Sampled By: CLIENT on 24-JUL-19 Matrix: SOIL Miscellaneous Parameters							
% Moisture	34.9		0.10	%	01-AUG-19	02-AUG-19	R4737126
Resistivity	3580		1.0	ohm*cm		08-AUG-19	
Sulphate	30		20	mg/kg	02-AUG-19	06-AUG-19	R4742614
pH (1:2 soil:water)	8.18		0.10	pH	07-AUG-19	07-AUG-19	R4740588
Conductivity	0.279		0.0040	mS/cm		08-AUG-19	R4743353
L2318801-5 TH19-15; G142 @ 20' Sampled By: CLIENT on 24-JUL-19 Matrix: SOIL Miscellaneous Parameters							
% Moisture	35.0		0.10	%	01-AUG-19	02-AUG-19	R4737126
Resistivity	940		1.0	ohm*cm		08-AUG-19	
Sulphate	890		20	mg/kg	02-AUG-19	06-AUG-19	R4742614
pH (1:2 soil:water)	8.28		0.10	pH	07-AUG-19	07-AUG-19	R4740588
Conductivity	1.06		0.0040	mS/cm		08-AUG-19	R4743353

* Refer to Referenced Information for Qualifiers (if any) and Methodology.

Reference Information

Test Method References:

ALS Test Code	Matrix	Test Description	Method Reference**
EC-WT	Soil	Conductivity (EC)	MOEE E3138
<p>A representative subsample is tumbled with de-ionized (DI) water. The ratio of water to soil is 2:1 v/w. After tumbling the sample is then analyzed by a conductivity meter.</p> <p>Analysis conducted in accordance with the Protocol for Analytical Methods Used in the Assessment of Properties under Part XV.1 of the Environmental Protection Act (July 1, 2011).</p>			
MOISTURE-WT	Soil	% Moisture	CCME PHC in Soil - Tier 1 (mod)
PH-1:2-SK	Soil	pH (1:2 Soil:Water Extraction)	AB Ag (1988) p.7
<p>1 part dry soil and 2 parts de-ionized water (by volume) is mixed. The slurry is allowed to stand with occasional stirring for 30 - 60 minutes. After equilibration, pH of the slurry is measured using a pH meter.</p>			
RESISTIVITY-CALC-WT	Soil	Resistivity Calculation	APHA 2510 B
<p>Resistivity are calculated based on the conductivity using APHA 2510B where Conductivity is the inverse of Resistivity.</p>			
RESISTIVITY-CALC-WT	Soil	Resistivity Calculation	MOECC E3138
<p>Resistivity are calculated based on the conductivity using APHA 2510B where Conductivity is the inverse of Resistivity.</p>			
SO4-WT	Soil	Sulphate	EPA 300.0
<p>5 grams of soil is mixed with 50 mL of distilled water for a minimum of 30 minutes. The extract is filtered and analyzed by ion chromatography.</p>			

** ALS test methods may incorporate modifications from specified reference methods to improve performance.

The last two letters of the above test code(s) indicate the laboratory that performed analytical analysis for that test. Refer to the list below:

Laboratory Definition Code	Laboratory Location
SK	ALS ENVIRONMENTAL - SASKATOON, SASKATCHEWAN, CANADA
WT	ALS ENVIRONMENTAL - WATERLOO, ONTARIO, CANADA

Chain of Custody Numbers:

GLOSSARY OF REPORT TERMS

Surrogates are compounds that are similar in behaviour to target analyte(s), but that do not normally occur in environmental samples. For applicable tests, surrogates are added to samples prior to analysis as a check on recovery. In reports that display the D.L. column, laboratory objectives for surrogates are listed there.

mg/kg - milligrams per kilogram based on dry weight of sample

mg/kg wwt - milligrams per kilogram based on wet weight of sample

mg/kg lwt - milligrams per kilogram based on lipid-adjusted weight

mg/L - unit of concentration based on volume, parts per million.

< - Less than.

D.L. - The reporting limit.

N/A - Result not available. Refer to qualifier code and definition for explanation.

Test results reported relate only to the samples as received by the laboratory.

UNLESS OTHERWISE STATED, ALL SAMPLES WERE RECEIVED IN ACCEPTABLE CONDITION.

Analytical results in unsigned test reports with the DRAFT watermark are subject to change, pending final QC review.



Quality Control Report

Workorder: L2318801

Report Date: 08-AUG-19

Page 1 of 2

Client: AECOM Canada Ltd.
 99 Commerce Drive
 Winnipeg MB R3P 0Y7

Contact: RYAN HARRAS

Test	Matrix	Reference	Result	Qualifier	Units	RPD	Limit	Analyzed
EC-WT		Soil						
Batch	R4743353							
WG3126300-2	IRM	WT SAR3						
Conductivity			86.4		%		70-130	08-AUG-19
WG3126843-1	LCS							
Conductivity			97.5		%		90-110	08-AUG-19
WG3126300-1	MB							
Conductivity			<0.0040		mS/cm		0.004	08-AUG-19
MOISTURE-WT		Soil						
Batch	R4737126							
WG3122283-2	LCS							
% Moisture			100.8		%		90-110	02-AUG-19
WG3122283-1	MB							
% Moisture			<0.10		%		0.1	02-AUG-19
PH-1:2-SK		Soil						
Batch	R4740588							
WG3121916-2	IRM	SAL814						
pH (1:2 soil:water)			7.90		pH		7.65-8.25	07-AUG-19
WG3121916-3	LCS							
pH (1:2 soil:water)			6.88		pH		6.66-7.06	07-AUG-19
SO4-WT		Soil						
Batch	R4742614							
WG3123166-4	CRM	AN-CRM-WT						
Sulphate			96.2		%		60-140	06-AUG-19
WG3123166-2	LCS							
Sulphate			103.3		%		80-120	06-AUG-19
WG3123166-1	MB							
Sulphate			<20		mg/kg		20	06-AUG-19

Quality Control Report

Workorder: L2318801

Report Date: 08-AUG-19

Page 2 of 2

Legend:

Limit	ALS Control Limit (Data Quality Objectives)
DUP	Duplicate
RPD	Relative Percent Difference
N/A	Not Available
LCS	Laboratory Control Sample
SRM	Standard Reference Material
MS	Matrix Spike
MSD	Matrix Spike Duplicate
ADE	Average Desorption Efficiency
MB	Method Blank
IRM	Internal Reference Material
CRM	Certified Reference Material
CCV	Continuing Calibration Verification
CVS	Calibration Verification Standard
LCSD	Laboratory Control Sample Duplicate

Hold Time Exceedances:

All test results reported with this submission were conducted within ALS recommended hold times.

ALS recommended hold times may vary by province. They are assigned to meet known provincial and/or federal government requirements. In the absence of regulatory hold times, ALS establishes recommendations based on guidelines published by the US EPA, APHA Standard Methods, or Environment Canada (where available). For more information, please contact ALS.

The ALS Quality Control Report is provided to ALS clients upon request. ALS includes comprehensive QC checks with every analysis to ensure our high standards of quality are met. Each QC result has a known or expected target value, which is compared against pre-determined data quality objectives to provide confidence in the accuracy of associated test results.

Please note that this report may contain QC results from anonymous Sample Duplicates and Matrix Spikes that do not originate from this Work Order.

