

A. GENERAL NOTES

- THIS STRUCTURE IS DESIGNED IN ACCORDANCE WITH, AND SHALL BE CONSTRUCTED IN COMPLIANCE WITH, THE NATIONAL BUILDING CODE OF CANADA 2010, THE MANITOBA BUILDING CODE 2011, AND ALL APPLICABLE LOCAL BYLAWS.
- DESIGN LOADS ARE INDICATED ON THE DRAWINGS.
- DESIGN LIVE LOADS SHALL NOT BE EXCEEDED AT ANY TIME DURING CONSTRUCTION.
- DO NOT SCALE DRAWINGS.
- ALL DIMENSIONS SHALL BE CHECKED AND VERIFIED PRIOR TO COMMENCING CONSTRUCTION.
- VERIFY ALL DIMENSIONS, ELEVATIONS, SLOPES, DETAILS, CONDITIONS, ETC., SHOWN ON THE STRUCTURAL DRAWINGS; WITH THE LATEST ARCHITECTURAL DRAWINGS, OTHER CONSULTANT DRAWINGS AND WITH SITE CONDITIONS, PRIOR TO CONSTRUCTION OR PREFABRICATION OF ANY BUILDING COMPONENT.
- MODIFICATIONS, ALTERNATIONS OR SUBSTITUTIONS MUST BE AUTHORIZED IN WRITING BY THE CONTRACT ADMINISTRATOR.
- LOCATE ALL EXISTING SUBGRADE SERVICES PRIOR TO CONSTRUCTION.
- DESIGN AND DETAIL ALL NECESSARY SHORING, BRACING AND FORMWORK. FORMWORK FOR CONSTRUCTION SHALL BE BRIDGED OVER EXISTING SERVICES. PROCEDURE MUST BE APPROVED BY THE CONTRACT ADMINISTRATOR.
- FORMWORK SHALL BE APPROVED BY AN INDEPENDENT PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF MANITOBA AND HOLDS A CURRENT "CERTIFICATE OF AUTHORIZATION" OF APEGM.
- FOR OPENINGS IN SLAB, FLOOR, WALLS, ROOF, ETC. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL AND/OR OTHER PERTINENT DRAWINGS.
- REVIEW LOCATION OF INTENDED CONSTRUCTION JOINTS WITH CONTRACT ADMINISTRATOR PRIOR TO PROCEEDING.
- CONSTRUCTION SAFETY REQUIREMENTS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
- DEFECTIVE OR UNACCEPTABLE WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE CONTRACT ADMINISTRATOR AT NO ADDITIONAL COST TO THE PROJECT.
- WHERE THERE IS A DISCREPANCY BETWEEN PROJECT SPECIFICATIONS AND GENERAL NOTES, INFORMATION SHOWN IN SPECIFICATIONS SHALL GOVERN.

B. DESIGN LOADS

1. ENVIRONMENTAL LOAD PARAMETERS
- SNOW LOAD: $S_s = 1.72 \text{ kPa}$
 $S_r = 0.2 \text{ kPa}$
 $I_w = 1.0$
 - WIND LOAD: **HOURLY WIND PRESSURE (1/50) = 0.45 kPa**
 $I_w = 1.0$

2. MAIN FLOOR LOADS: SEE DRAWING S0010
3. ROOF LOADS: SEE DRAWING S0010

C. EXCAVATION AND BACKFILLING

- DEWATER SITE PRIOR TO, AND DURING CONSTRUCTION SEE GEOTECHNICAL REPORT.
- REMOVE ALL TOPSOIL, ORGANIC MATERIAL AND LOOSE OR UNSUITABLE FILL TO THE APPROVAL OF THE GEOTECHNICAL ENGINEER.
- PROVIDE BACKFILL MATERIAL AS NOTED IN SPECIFICATIONS.
- CONSTRUCTION EQUIPMENT SHOULD NOT TRAVEL DIRECTLY ON THE FOUNDATION BEARING SURFACE. USE FLAT BUCKET EXCAVATION AT THE FOUNDATION LEVEL. TAKE MEASURES TO PREVENT CHANGES IN SOIL MOISTURE CONTENT AT THE FOUNDATION BEARING SURFACE.
- PROVIDE BACKFILL AS FOLLOWS:

- UNDER SLABS ON GRADE (MIN 200mm THICK)
- COMPACT NATIVE SOIL SUBGRADE TO 98% STANDARD PROCTOR.
- AGAINST PERIMETER BEAMS/WALL (OUTSIDE)
- TO WITHIN 300MM OF FINISHED GRADE:
SUITABLE EXCAVATED MATERIAL COMPACTED TO 95% STANDARD PROCTOR TO FINISHED GRADE:
TYPE 2 MATERIAL TO UNDERSIDE CONCRETE SLABS, TOPSOIL AT UNPAVED AREAS

- SUBMIT AN EXCAVATION AND SHORING PLAN PREPARED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN MANITOBA.

D. CAST-IN-PLACE-CONCRETE

- ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH CSA-A23.1-09 CONCRETE MATERIALS AND METHODS OF CONCRETE CONSTRUCTION / METHODS OF TEST AND STANDARD PRACTICES FOR CONCRETE.
- SUPPLEMENTARY CEMENTITIOUS MATERIALS TO CSA-A3000-08 CEMENTITIOUS MATERIALS COMPENDIUM.
- CHEMICAL ADMIXTURES TO ASTM C494/C494M-12 AND ASTM C1017/C1017M-07.
- GENERAL CONTRACTOR TO PROVIDE PROPRIETARY MIX DESIGN PERFORMANCE RECORD AS REQUIRED BY THE MANITOBA READY-MIX ASSOCIATION.
- CONCRETE SPECIFICATIONS:
SEE TABLE D.1
- CONSTRUCT FORMWORK, SHORING AND BRACING TO MEET DESIGN, CODE CSA 289.3-M92 (REAFFIRMED 2013) AND CSA-A23.1-09 REQUIREMENTS. CONSTRUCT ACCURATELY, SO THAT RESULTANT FINISHED CONCRETE

CONFORMS TO SHAPES, LINES AND DIMENSIONS INDICATED ON THE DRAWINGS.

- VIBRATE ALL CONCRETE WORK WITH APPROPRIATE INTERNAL VIBRATORS.
- CONCRETE WORKING TIME, FROM BATCHING TO PLACEMENT AND CONSOLIDATION, SHALL NOT EXCEED 1-1/2 HOURS.
- CONCRETE CONTRACTOR SHALL PLACE ALL COMPONENTS TO BE EMBEDDED IN THE CONCRETE (i.e. WELD PLATES, DOWELS FOR CONCRETE AND/OR MASONRY, ANCHOR BOLTS, INSERTS, WATER STOP BARS, SLEEVING, ETC.). SEE STRUCTURAL, ARCHITECTURAL, MECHANICAL, ELECTRICAL AND ANY OTHER PERTINENT DRAWINGS.
- CLEAR CONCRETE COVER TO REINFORCING STEEL SHALL BE AS FOLLOWS U.N.O.:

SEE TABLE D.2

- WHERE NO DOWEL EMBEDMENT OR EMBEDMENT TYPE IS INDICATED ON THESE DRAWINGS IT SHALL BE A TENSION EMBEDMENT EXCEPT FOR COLUMNS WHICH SHALL BE A COMPRESSION EMBEDMENT:
SEE TABLE D.3
- WHERE NO SPLICE OR SPLICE TYPE IS INDICATED ON THESE DRAWINGS IT SHALL BE A TENSION SPLICE EXCEPT FOR COLUMNS WHICH SHALL BE A COMPRESSION SPLICE (UNLESS DETAILED OTHERWISE):
SEE TABLE D.4

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- INSTALL WATER STOP BAR IN ALL TRENCH WALL AND PIT CONSTRUCTION JOINTS.
- SEE ARCHITECTURAL DRAWINGS FOR SURFACE FINISHES, EDGE TREATMENTS, ETC.
- VOID FORMS UNDER SLABS, BEAMS AND WALLS SHALL BE HONEYCOMB TYPE BIODEGRADABLE CARDBOARD, 300 MM THICK TREATED TO PROVIDE SUFFICIENT STRUCTURAL SUPPORT FOR CONCRETE UNTIL CONCRETE IS CURED (UNO).
- AS AN ALTERNATE, CONTRACTOR MAY USE STYROFOAM VOID FILLER WHICH MUST MAINTAIN AND SPACE NOTED ABOVE WHEN COLLAPSED COMPRESSED. STYROFOAM VOID FILLER SHALL BE SELECTED AND DESIGNED BY MANUFACTURER. VOID FILLER SELECTED TO BE FORWARDED TO CONTRACT ADMINISTRATOR FOR REVIEW PRIOR TO CONSTRUCTION.
- ALL FORMWORK INCLUDING CARDBOARD "SONO-TUBES" TO BE REMOVED UPON COMPLETION.
- FULL TIME INSPECTION BY AN APPROVED GEOTECHNICAL CONSULTANT RETAINED AND PAID FOR BY THE CONTRACTOR IS TO BE PROVIDED DURING THE DRIVING OF ALL PILES. A DRIVING RECORD OF PENETRATION PER BLOW SHALL BE KEPT BY THE INSPECTOR. THESE RECORDS SHALL INCLUDE FINAL PENETRATION RESISTANCE, PILE HEAVE AND AMOUNT OF DOWNWARD MOVEMENT OF REDRIVE.
- OUT OF PLACE, DEFECTIVE OR PILES WHICH ARE DAMAGED IN HANDLING OR ALL WELDS/UNIT PILES WHICH ARE DAMAGED IN HANDLING OR SUBSTITUTED AT NO EXTRA COST TO THE CITY.
- SURVEY THE STRUCTURAL CONDITIONS OF SURROUNDING EXISTING BUILDINGS AND KEEP PHOTO RECORDS PRIOR TO START OF DRIVING PILES. ANY DAMAGE TO BUILDING COMPONENTS SHALL BE REPAIRED TO THE SATISFACTION OF THE CONTRACT ADMINISTRATOR AT NO EXTRA COST TO THE CONTRACT.

- CONSTRUCT FORMWORK, SHORING AND BRACING TO MEET DESIGN, CODE AND CAN3-A23.1-09 REQUIREMENTS.
- CONSTRUCTION JOINTS, CONCRETE PLACEMENT SCHEDULING AND WORK PROCEDURES SHALL BE DISCUSSED WITH THE CONTRACT ADMINISTRATOR PRIOR TO COMMENCING CONSTRUCTION.
- TYPICAL HOUSE KEEPING PAD UNDER ALL EQUIPMENT AND STORAGE AREAS = 125 THICK CONCRETE.
- FOR COLD WEATHER CONCRETE WORK, ALL ICE, SNOW AND FROST SHALL BE REMOVED FROM FORMWORK AND THE TEMPERATURE OF ALL CONTACT SURFACES SHALL BE RAISED ABOVE 10C FOR 24 HOURS PRIOR TO PLACING CONCRETE. CONCRETE SHALL BE NOT LESS THAN 20 DEGREES CELSIUS NOR MORE THAN 30 DEGREES CELSIUS WHEN DEPOSITED. CONCRETE SHALL BE ENCLOSED AND THIS AREA SHALL HAVE A TEMPERATURE OF NOT LESS THAN 20 DEGREES CELSIUS FOR THREE (3) DAYS AND NOT LESS THAN 5C FOR AN ADDITIONAL FOUR (4) DAYS.
- NOTIFY THE CONTRACT ADMINISTRATOR AT LEAST 48 HOURS PRIOR TO ALL CONCRETE PLACEMENT TO ALLOW FOR SITE INSPECTIONS.
- CONCRETE TESTING SHALL BE PERFORMED BY AN INDEPENDENT CSA APPROVED TESTING COMPANY RETAINED AND PAID FOR BY THE CONTRACTOR. THREE (3) CONCRETE TEST CYLINDERS AND ONE (1) SLUMP TEST SHALL BE TAKEN FOR EVERY 75 (OR LESS) CUBIC METERS, OR EACH DAY CONCRETE IS PLACED, WHICHEVER IS GREATER. TESTING SHALL BE PERFORMED IN ACCORDANCE WITH CSA-A23.2-09 AND THE RESULTS SHALL BE FORWARDED TO THE CONTRACT ADMINISTRATOR.

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- REFER TO MECHANICAL AND ELECTRICAL DRAWINGS FOR SERVICES INSTALLED BELOW THE SLAB AND SLEEVING REQUIRED FOR SERVICES PENETRATING THE SLAB.
- REFER TO ARCHITECTURAL DRAWINGS FOR SURFACE LEVEL TOLERANCES, SLOPES, FINISHES, SURFACE SEALERS OR HARDENERS, ETC.
- INSTALL SAWCUTS AS SHOWN ON STRUCTURAL PLANS. SAWCUTS TO BE 1" (25 mm) DEEP AND 1/8" (3mm) WIDE. CUT NO SOONER THAN 24 HOURS BUT NOT LATER THAN 48 HOURS AFTER SLAB IS POURED. FILL SAWCUTS WITH APPROVED BITUMINOUS COMPOUND OR CAULKING.
- PROVIDE CONSTRUCTION JOINTS C/W 1/2" (12mm) FLEXCELL AND GREASED DOWELS TO MATCH SLAB REINFORCING.

F. FOUNDATION - PRECAST DRIVEN PILES

- PRECAST DRIVEN PILES ARE DESIGNED FOR MAXIMUM CAPACITY AS FOLLOWS:
• 300 MM HEXAGONAL - 444 KN
• 350 MM HEXAGONAL - 622 KN
• 400 MM HEXAGONAL - 800 KN

- ALL PILES SHALL BE DRIVEN TO REFUSAL AND IN ACCORDANCE WITH RECOMMENDATIONS MADE BY DYREGROV CONSULTANTS IN THEIR REPORT DATED DECEMBER 2004 AND LETTER BY T. CRILL DATED JANUARY 7 2016.
- THE ACTUAL SET SHALL BE ESTABLISHED FROM THE HAMMER RATINGS TO THE SATISFACTION OF THE CONTRACT ADMINISTRATOR BEFORE DRIVING COMMENCES.
- IF ANY MEASURABLE UPHEAVING OCCURS TO A PILE DURING THE DRIVING OF ADJACENT PILES, THE PILE IS TO BE REDRIVEN TO THE ORIGINAL ELEVATION AND SET.
- PILES SHALL BE CUT OFF SQUARE AND NEAT FOR A MINIMUM OF 450 MM (UNLESS OTHERWISE NOTED) STRAND PROJECTION FROM THE PILE. SHATTERING OR SPLITTING OF THE TOP OF THE PILE SHALL BE CAUSE FOR REJECTION TO DRAWING.
- ALL PILES AND PILE CAPS SHALL BE MADE WITH SULPHATE RESISTANT CEMENT TYPE HS AND HAVE A 28-DAY STRENGTH OF 35 MPa. AT TIME OF DRIVING CONCRETE FOR PILES SHALL HAVE A COMPRESSIVE STRENGTH OF 35 MPa. CONCRETE STRENGTH AT TRANSFER OF PRESTRESS SHALL BE MINIMUM 25 MPa.
- FULL TIME INSPECTION BY AN APPROVED GEOTECHNICAL CONSULTANT RETAINED AND PAID FOR BY THE CONTRACTOR IS TO BE PROVIDED DURING THE DRIVING OF ALL PILES. A DRIVING RECORD OF PENETRATION PER BLOW SHALL BE KEPT BY THE INSPECTOR. THESE RECORDS SHALL INCLUDE FINAL PENETRATION RESISTANCE, PILE HEAVE AND AMOUNT OF DOWNWARD MOVEMENT OF REDRIVE.
- OUT OF PLACE, DEFECTIVE OR PILES WHICH ARE DAMAGED IN HANDLING OR ALL WELDS/UNIT PILES WHICH ARE DAMAGED IN HANDLING OR SUBSTITUTED AT NO EXTRA COST TO THE CITY.
- SURVEY THE STRUCTURAL CONDITIONS OF SURROUNDING EXISTING BUILDINGS AND KEEP PHOTO RECORDS PRIOR TO START OF DRIVING PILES. ANY DAMAGE TO BUILDING COMPONENTS SHALL BE REPAIRED TO THE SATISFACTION OF THE CONTRACT ADMINISTRATOR AT NO EXTRA COST TO THE CONTRACT.

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