# **APPENDIX 'F'**

# **GEOTECHNICAL REPORT**



# GEOTECHNICAL INVESTIGATION RESIDENTIAL STREETS RECONSTRUCTION WINNIPEG, MANITOBA

Submitted to:

MMM Group Limited

111-93 Lombard Avenue Winnipeg, Manitoba R3B 3B1

Attention: Mr. Vilko Maroti

Submitted by:

Amec Foster Wheeler Environment & Infrastructure

440 Dovercourt Drive Winnipeg, Manitoba R3Y 1N4

21 January 2015

Amec Foster Wheeler File No. WX17565R1

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#### **1.0 INTRODUCTION**

Amec Foster Wheeler Environment & Infrastructure (Amec Foster Wheeler), was retained by MMM Group Limited (MMM) to conduct a pavement coring and geotechnical investigation for proposed upgrading projects for several streets in Winnipeg, Manitoba. The investigation was conducted on sections of 13 residential Winnipeg streets, as follows:

Coring and Geotechnical Investigation:

- Andrews Street: Jefferson Avenue To Hartford Avenue
- Church Avenue: Aikins Street to Salter Street
- Machray Avenue: Main Street to Emslie Street
- Powers Street: Burrows Avenue To Pritchard Avenue
- Redwood Avenue: Battery Street to Sergeant Tommy Prince Street

Coring Only:

- Airlies Street: Merriwood Avenue to Jefferson Avenue
- Ashmore Drive: Mandalay Drive to Adsum Drive
- Burrows Avenue: Keewatin Street to Dorset Street
- Duval Street: Adsum Drive to Margate Road
- Hallet Street: Rover Avenue to Euclid Avenue
- Redwood Avenue: Shaughnessy Street to Sheppard Street
- Rose Hill Place: Rose Hill Way to Rose Hill Way
- Rose Hill Way: Garton Avenue to Rose Hill Place.

The purpose of the investigation was to determine the pavement condition and soil profile at selected locations along the subject section of each street. The numbers of test holes per street were determined by MMM. Amec Foster Wheeler selected the test hole locations and confirmed the locations with MMM prior to the investigation.

#### 2.0 SITE CONDITIONS

At the streets investigated, the roadway surface consisted of either asphalt or concrete. At the time of the investigation, the roadways were snow and ice covered and therefore a detailed review of the pavement condition was not possible. Typical of roads in the Winnipeg area, the roads were generally flat lying and level, with local slopes between catch basins to facilitate drainage.

#### 3.0 FIELD INVESTIGATON

Prior to coring and drilling, Amec Foster Wheeler had public underground utilities located. Between 3 and 9 December 2014, following utility clearances, all 40 test hole locations were cored in order to determine the asphalt and/or concrete thicknesses. Because of variable conditions encountered in the initial coring at Airlies Street and Hallet Street, two additional locations were cored on each of these streets on 15 January 2015. All coring was completed

with a 150 mm diameter diamond coring barrel. Each of the cores was photographed and photos are presented in Appendix B.

On 4 and 5 December 2014, Amec Foster Wheeler supervised the drilling of 24 test holes by Maple Leaf Drilling Ltd. using their B40 truck mounted drill rig, equipped with 125 mm solid stem augers. The test holes were drilled in previously cored locations except for Powers Street TH05 which was obstructed by parked vehicles, and Church Street TH03, which conflicted with underground utilities, and were drilled through the pavement about 3 m away from the cored location. The coring and test hole locations are shown on Figures 1 to 6. Amec Foster Wheeler provided traffic control using flashing light-board trailers, cones, and signs from Guardian Traffic Services.

All soils observed during test hole drilling were visually classified on site according to the Modified Unified Soil Classification System and in accordance with the City of Winnipeg geotechnical guidelines. Groundwater and drilling conditions, as well as any pertinent subsurface observations, were also recorded at the time of the investigation.

Disturbed soil samples were taken at 0.3 m intervals from the auger flights in each test hole. All soil samples obtained during the field investigation were labelled, sealed in plastic bags to limit moisture loss and transported to Amec Foster Wheeler's soils laboratory in Winnipeg for further examination and testing. The test hole logs are presented in Appendix A, Figures 7 to 30, and show the soil profile, results of the field and laboratory testing, and comments relative to groundwater and sloughing conditions encountered.

Each test hole was backfilled with the auger cuttings and patched with asphalt after coring and drilling, with excess cuttings removed from the road.

#### 4.0 LABORATORY TESTING

Soil samples were returned to Amec Foster Wheeler's soils laboratory in Winnipeg for geotechnical laboratory testing. The soil samples were visually classified and tested for in-situ moisture contents, and selected samples were tested for Atterberg limits and hydrometer analysis, in accordance with City requirements.

#### 5.0 SUBSURFACE CONDITIONS

The generalized stratigraphy, as noted in the test holes for each investigated street is summarized in the following sections. It should be noted that the maximum gravel sizes noted are inferred, since drilling in freezing conditions can break the gravel into finer particles. Detailed soil stratigraphy is illustrated in the test hole logs presented in Appendix A.

#### 5.1 Andrews Street – Test Holes TH01 to TH06

Concrete pavement, between 130 and 155 mm thick, was present at the ground surface at every test hole, and was underlain by 80 to 460 mm of clay fill. Alternating layers of native clay and silt, between 0.3 to 0.7 m thick, were encountered below the pavement and fill layers and extended to the maximum depths explored. The ground was frozen to 0.5 to 0.8 m below the pavement surface.

The clay fill generally contained variable silt, sand, and gravel, and was medium to high plastic, damp to moist, firm to stiff or friable when thawed, and brown or grey.

The silt generally contained trace to some clay, and was low plastic, moist to very moist, soft to stiff, grey or tan-brown, and contained occasional clay inclusions.

The native clay generally contained variable silt (some silt to silty), and was high plastic, damp to moist, stiff to very stiff, and brown or grey.

Table 1 summarizes the thickness and types of pavements and soils encountered at each of the test hole locations on Andrews Street.

Test Hole #	TH01	TH02	TH03	TH04	TH05	TH06
Concrete	145	155	130	140	145	150
Fill – Clay	80	305	320	460	460	165
Clay	700			610		460
Silt	900	300	760	610	460	610
Clay		300	610		760	460
Silt		300				
Clay		460				

# Table 1: Andrews Street Pavement and Soil Thickness (mm)

## 5.2 Church Avenue – Test Holes TH01 to TH04

Asphalt pavement, between 27 and 65 mm thick, was present at the ground surface at every test hole, and was underlain by 30 to 120 mm of gravel fill. In TH02 and TH03 the asphalt pavement was observed to contain two distinct layers. In TH02 and TH03 clay fill between 275 and 640 mm thick was encountered below the gravel fill. Alternating layers of native high plastic clay and low plastic silt, between 0.3 to 0.8 m thick, were encountered below the pavement and fill layers and extended to the maximum depths explored. The ground was frozen to 0.5 to 0.8 m below the pavement surface.

The gravel fill was generally well graded, compact, with sub-rounded to angular particles, was damp to moist, and had maximum aggregate diameters between 19 and 50 mm at the test hole locations.

The clay fill generally was silty and contained traces of sand and gravel, was high plastic, damp to moist, firm or friable when thawed, and grey.

The silt generally contained trace to some clay, and was low plastic, moist to very moist, soft to stiff, and tan-brown.

The native clay generally contained variable silt (some silt to silty), and was high plastic, damp to moist, firm to very stiff, and brown or grey.

Table 2 summarizes the thickness and types of pavements and soils encountered at each of the test hole locations on Church Street.

Test Hole #	TH01	TH02	TH03	TH04
Asphalt	30	25+28	28+37	27
Fill – Gravel	120	120	60	30
Fill – Clay		275	640	
Clay	610		300	670
Silt	1070	760	760	300
Clay		610	610	760

## Table 2: Church Avenue Pavement and Soil Thickness (mm)

## 5.3 Machray Avenue – Test Holes TH01 to TH04

Asphalt pavement, between 73 and 133 mm thick, was present at the ground surface at every test hole. In TH01 the asphalt pavement was observed to contain two distinct layers, and in TH03 the asphalt pavement was underlain by concrete pavement measuring 95 mm thick. Gravel fill between 30 and 90 mm thick was encountered immediately below the pavement in all test holes. The gravel fill in TH04 was underlain by a 150 mm thick layer of clay fill. In TH01 to TH03, native high plastic clay was encountered immediately below the pavement and fill layers and extended to the test hole termination depths. In TH02 a layer of silt 0.8 m thick was encountered immediately below the fill, and was underlain by clay which extended to the maximum depth explored. The ground was frozen up to 0.6 m below the pavement surface.

The gravel fill was poorly graded in TH02, but was otherwise generally well graded. The gravel was compact, with sub-rounded to angular particles, was damp, brown or grey, and had maximum aggregate diameters between 12 and 25 mm at the test hole locations.

The clay fill was silty and contained traces of sand and gravel, was high plastic, damp to moist, friable when thawed, and grey.

The silt generally contained trace to some clay, and was low to medium plastic, moist to very moist, soft to stiff, and tan-brown.

The native clay generally contained variable silt (some silt to silty), and was high plastic, damp to moist, stiff to very stiff, and brown or grey.

Table 3 summarizes the thickness and types of pavements and soils encountered at each of the test hole locations on Machray Avenue.

Test Hole #	TH01	TH02	TH03	TH04
Asphalt	52+37	133	73	110
Concrete	0	0	95	0
Fill – Gravel	90	30	60	45
Fill – Clay				150
Clay	1650	760	1580	
Silt		760		1220
Clay		150		300

## Table 3: Machray Avenue Pavement and Soil Thickness (mm)

## 5.4 Powers Street – Test Holes TH01 to TH05

Asphalt pavement, between 20 and 90 mm thick, was present at the ground surface at every test hole. In TH04 the asphalt pavement was observed to contain two distinct layers. The asphalt was underlain by 30 to 120 mm of concrete rubble or gravel fill in all test holes. In TH04 and TH05 clay fill was encountered immediately below the rubble or gravel fill. The pavement and fill layers were underlain by high plastic clay in all test holes, and extended to the maximum depths explored. In TH01, TH02, and TH05 a layer of silt, between 0.3 and 0.6 m thick, was encountered within the clay layer. The ground was frozen to 0.6 to 0.8 m below the pavement surface.

The gravel fill was generally well graded, compact, with sub-rounded to angular particles, was damp, brown or grey, and had maximum aggregate diameters between 19 and 50 mm at the test hole locations.

The clay fill contained variable silt (silty to "and silt") and traces of sand and gravel, was high plastic, damp to moist, stiff or friable when thawed, and brown or grey.

The silt generally contained some clay, and was low plastic, moist to very moist, very soft to soft, and tan-brown.

The native clay generally contained variable silt (trace silt to silty), and was high plastic, damp to moist, soft to very stiff, and black, grey or brown.

Table 4 summarizes the thickness and types of pavements and soils encountered at each of the test hole locations on Powers Street.

Test Hole #	TH01	TH02	TH03	TH04	TH05
Asphalt	54	35	20	15+75	30
Concrete	100			30	
Concrete	(Rubble)			(Rubble)	
Fill – Gravel		120	120		120
Fill – Clay				185	610
Clay	460	610	1830	1520	460
Silt	610	550			300
Clay	610	520			300

## Table 4: Powers Street Pavement and Soil Thickness (mm)

#### 5.5 Redwood Avenue (Battery St. to Sgt. Tommy Prince St.) – Test Holes TH01 to TH05

Asphalt pavement, between 45 and 105 mm thick, was present at the ground surface at every test hole, and was underlain by gravel fill between 25 and 185 mm thick. In all test holes a layer of silt, between 0.5 and 0.9 m thick, was encountered immediately below the pavement and fill layers and was underlain by high plastic clay, which extended to the maximum depths explored. The ground was frozen up to 0.3 m below the pavement surface.

The gravel fill was poorly graded in TH03 but otherwise generally well graded, compact, with sub-rounded to angular particles, and was damp, brown or grey, and had maximum aggregate diameters between 12 and 35 mm.

The silt generally contained trace to some clay, and was low plastic, moist to very moist, soft to very stiff, and tan-brown.

The native clay generally contained variable silt (some silt to silty), and was high plastic, moist, firm to very stiff, and brown or grey.

Table 5 summarizes the thickness and types of pavements and soils encountered at each of the test hole locations on Redwood Avenue between Battery Street and Sergeant Tommy Prince Street.

Table 5: Redwood Avenue (Battery St. to Sgt. Tommy Prince St.) Pavement and
Soil Thickness (mm)

Test Hole #	TH01	TH02	TH03	TH04	TH05
Asphalt	55	60	105	45	78
Fill – Gravel	170	105	25	185	65
Silt	530	750	940	820	620
Clay	1370	1220	1070	1070	1370

#### 5.6 Coring Summary

Table 6 summarizes the thickness and types of pavements encountered at each of the coring locations not previously described with the drilling results. Photographs of all cores along with detailed coring location descriptions are presented in Appendix B.

Street	Pavement		Core #		
Street	Туре	C01	C02	C03	C04
Airlies Street	Asphalt	42	0	60	55
Annes Street	Concrete	Rubble	158	150	160
Ashmore Drive	Asphalt	0	0	0	
Asimole Drive	Concrete	154	155	140	
	Asphalt	55	0	45	50
Hallet Street	Concrete	0	150	230	50 (Rubble) + 130
Burrows Avenue	Asphalt	78	28		
Bullows Avenue	Concrete	235	220		
Duval Street	Asphalt	0	0		
Duval Street	Concrete	145	140		
Redwood Avenue	Asphalt	0	0	0	
(Shaughnessy St. to Sheppard St.)	Concrete	235	225	190	
Rose Hill Way	Asphalt	0	0		
	Concrete	155	175		

# Table 6: Pavement Thickness (mm)

The results from the initial and additional cores on Airlies Street and Hallet Street were as follows:

#### Airlies Street

- C01 indicated asphalt over concrete rubble. This location was not subsequently drilled and the depth of rubble was not determined.
- C02 indicated concrete with no asphalt overlay. Since the street was covered in approximately 10 cm of compacted snow and ice during the coring, no visual assessment of the pavement surface was made.
- C03 and C04 showed asphalt over a concrete pavement structure.

#### Hallet Street

- C01 indicated asphalt over a gravel base. It is possible that there was additional concrete below the gravel base. This location was not subsequently drilled and the depth of gravel was not determined.
- C02 indicated concrete with no asphalt overlay. Since the street was covered in approximately 10 cm of compacted snow and ice during the coring, no visual assessment of the pavement surface was made.
- C03 showed asphalt over a concrete pavement structure.
- C04 showed asphalt over concrete rubble over weathered but intact concrete.

## 6.0 CLOSURE

The findings of this report were based on the results of field and laboratory investigations at test hole locations as selected by MMM Group Limited.

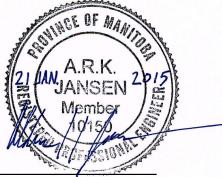
The site investigation was conducted for the sole purpose of profiling the pavement and subsurface conditions. Although no environmental issues were identified during the fieldwork, this does not indicate that no such issues exist. If the owner or other parties have any concern regarding the presence of environmental issues, then an appropriate level environmental assessment should be conducted.

Soil conditions, by their nature, can be highly variable across a site. The placement of fill and prior construction activities on a site can contribute to the variability especially near surface soil conditions. A contingency should always be included in any construction budget to allow for the possibility of variation in soil conditions, which may result in modification of any potential design and construction procedures which may arise from this factual investigative report.

This report was prepared exclusively for MMM Group Limited, and their clients and agents for the proposed development as described in the report. The data provided herein are presented in a factual manner only with no engineering interpretation provided, and should not be used for any other purpose, or by any other parties, without review and advice from a qualified geotechnical engineer. No other warranty, expressed or implied, is given.

Sincerely,

## Amec Foster Wheeler Environment & Infrastructure



Aldin Jansen, P.Eng. Geotechnical Engineer

## Reviewed By:

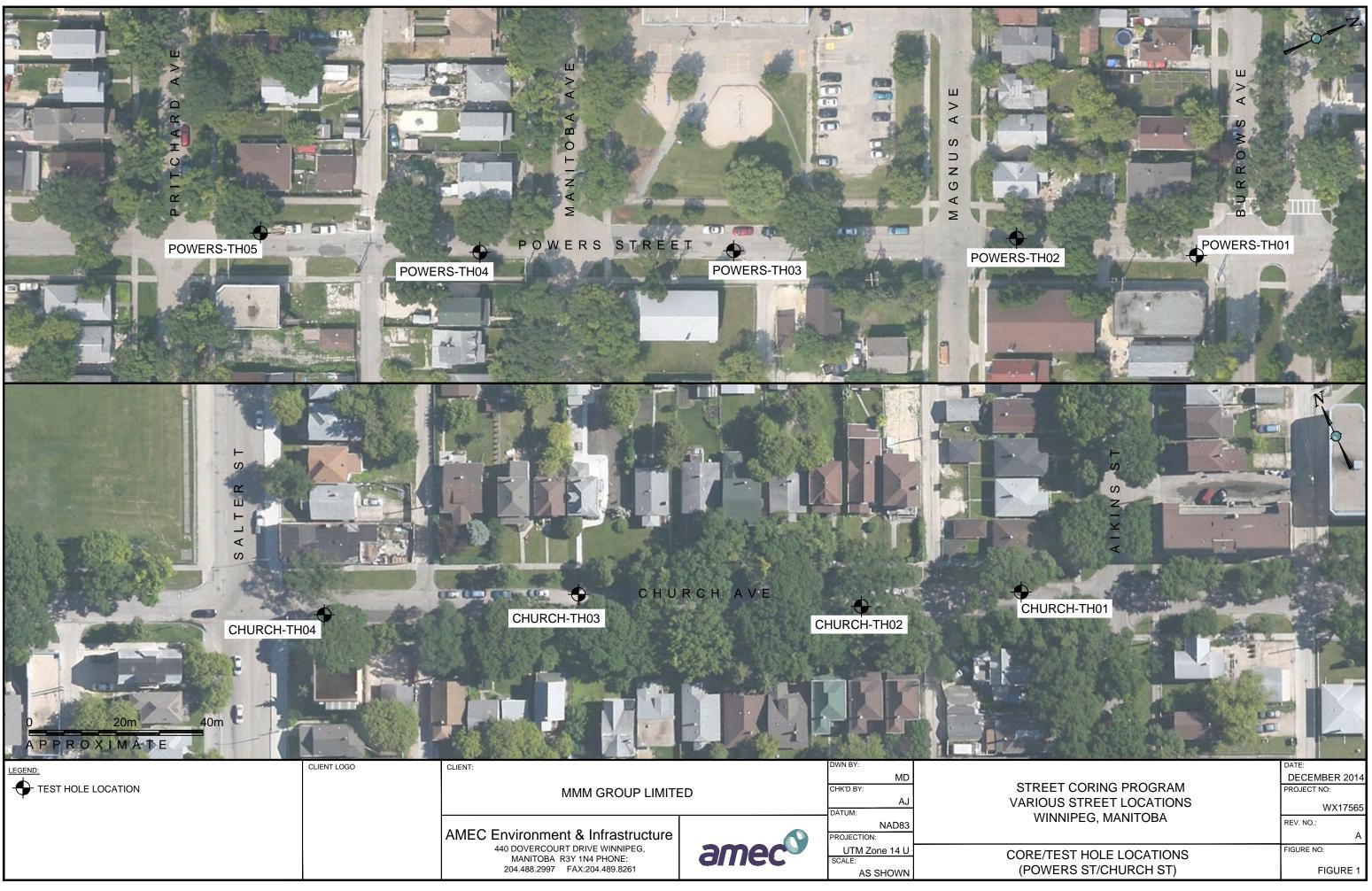


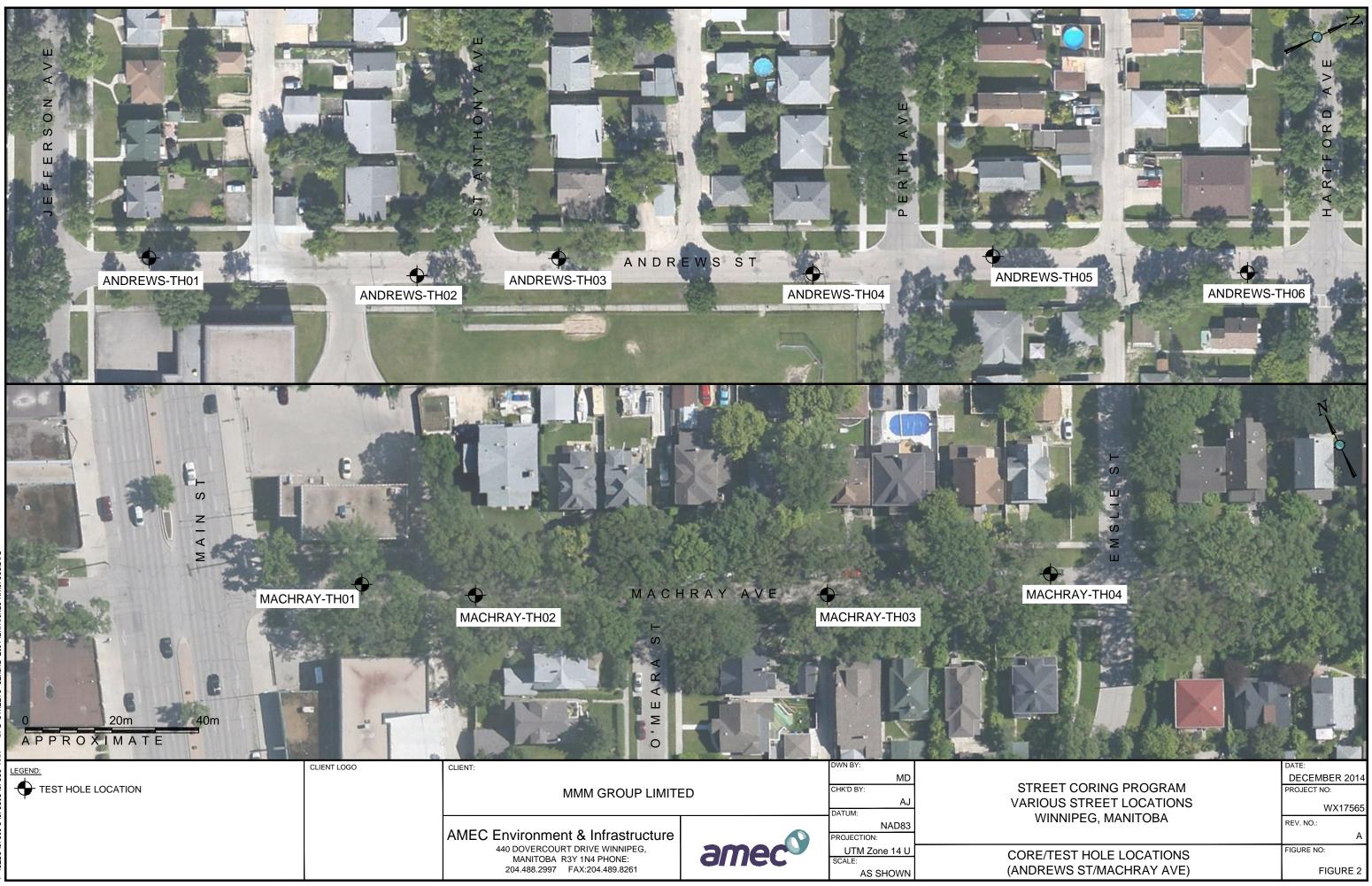
Harley Pankratz, P.Eng. Senior Associate Geotechnical Engineer VP; Eastern Prairies and Northern Alberta



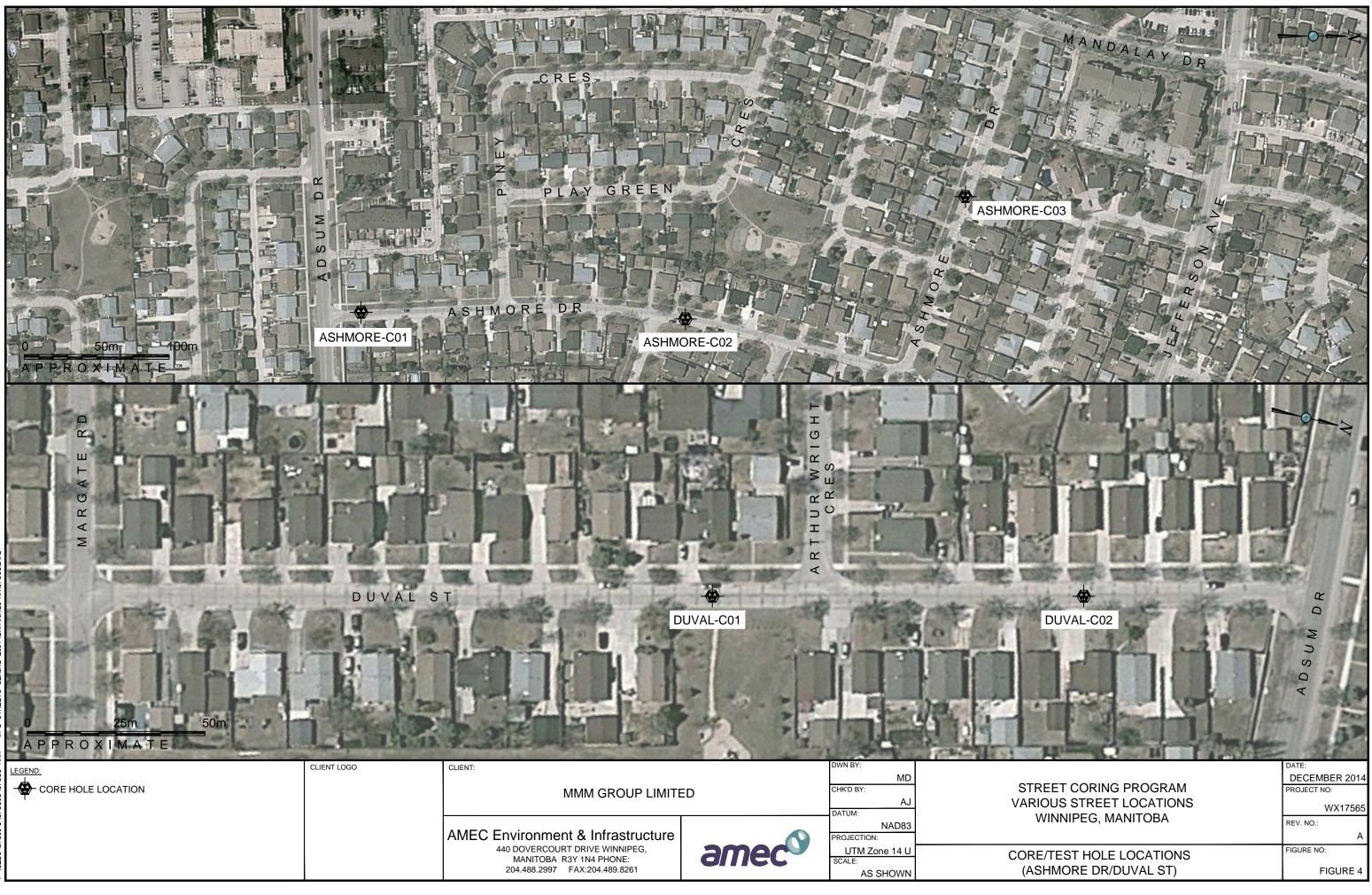
# FIGURES

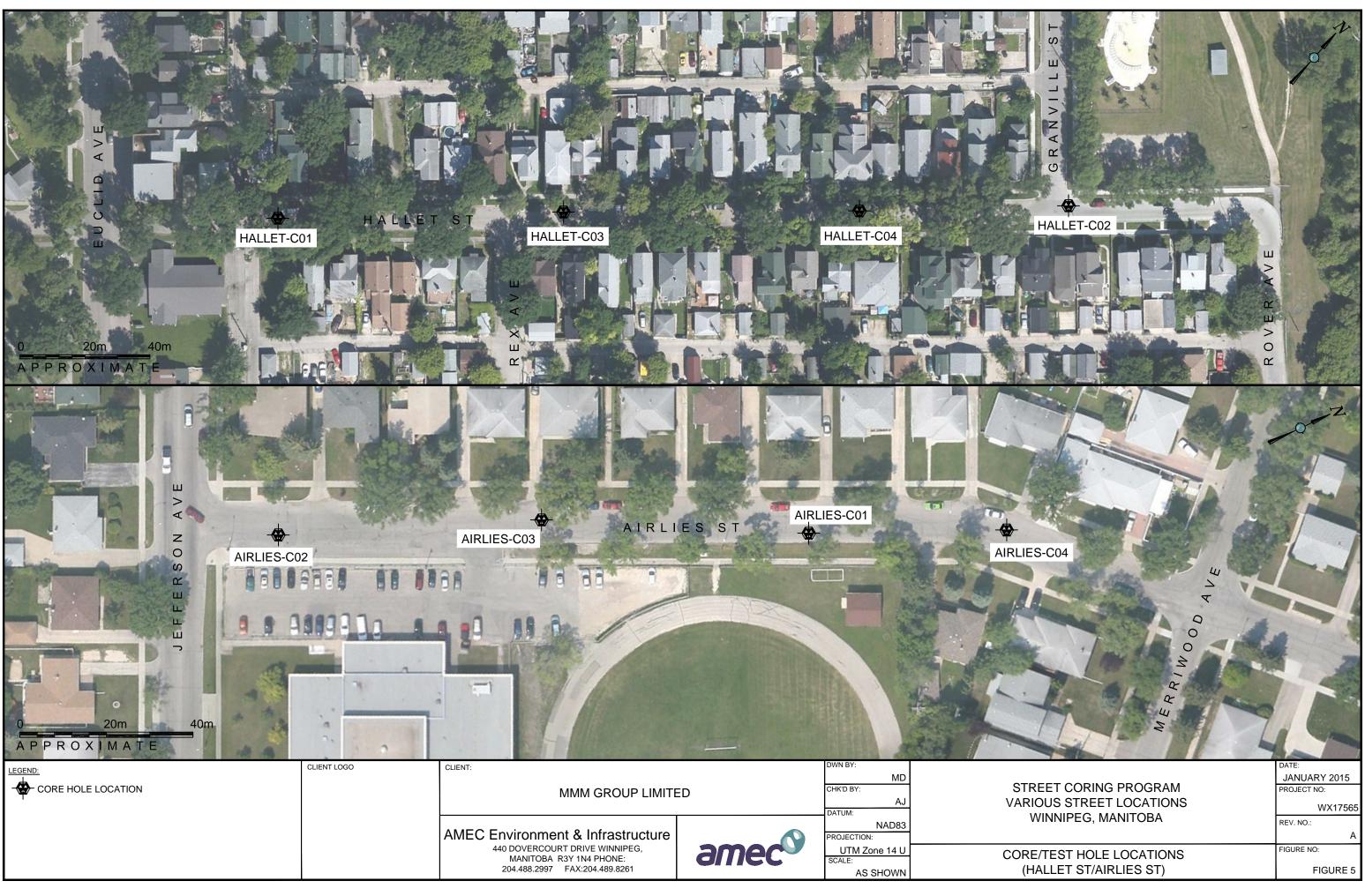
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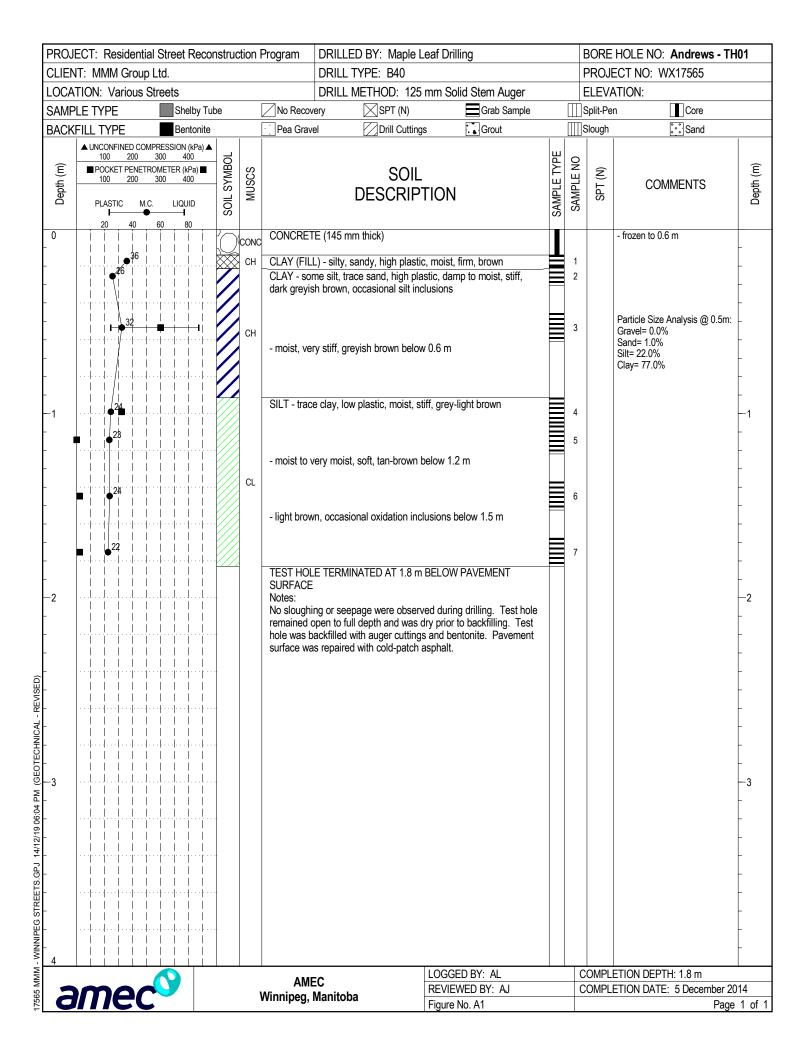


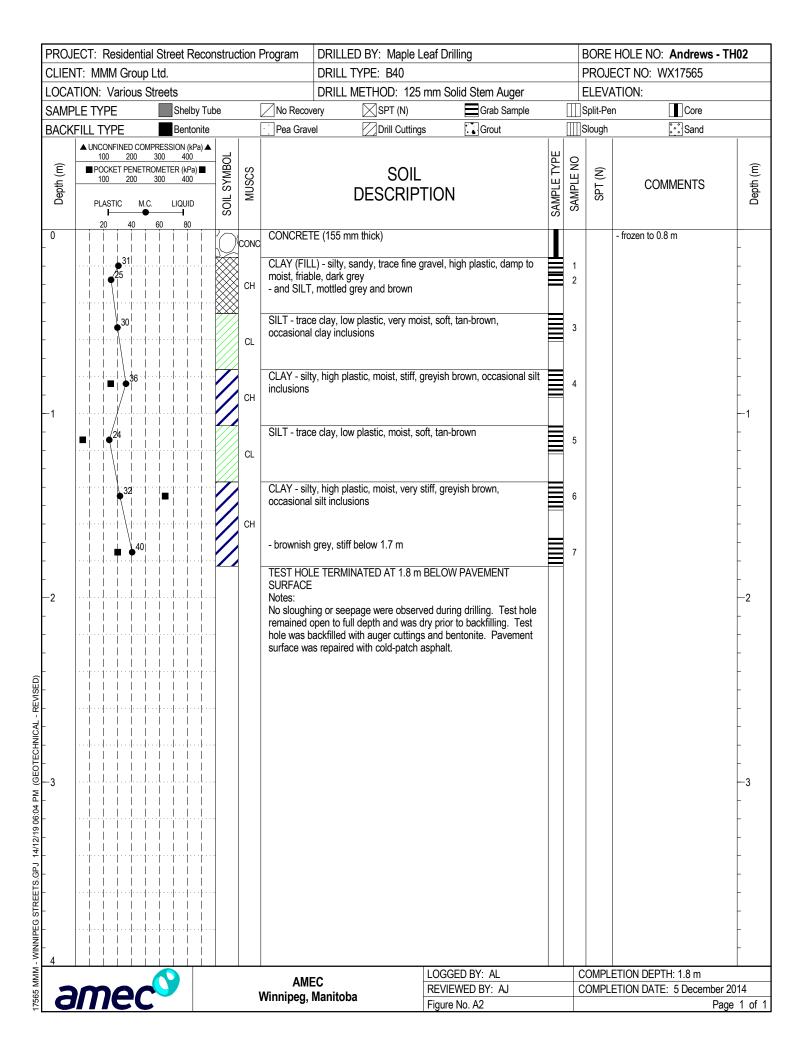


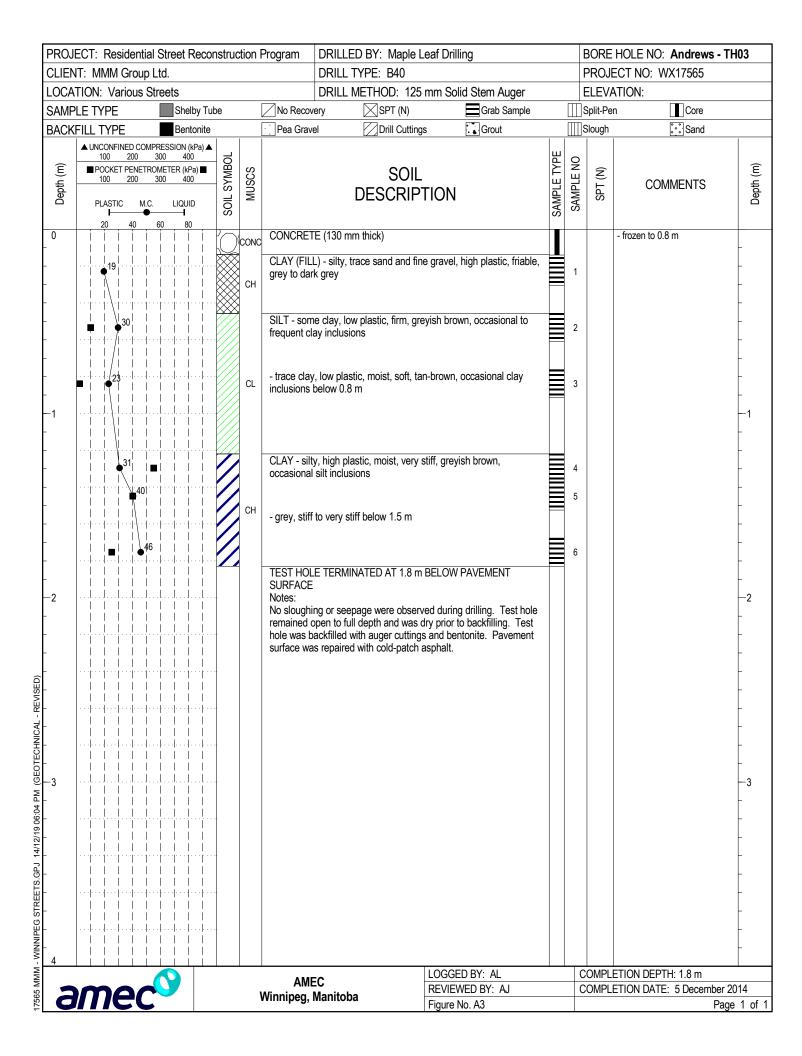


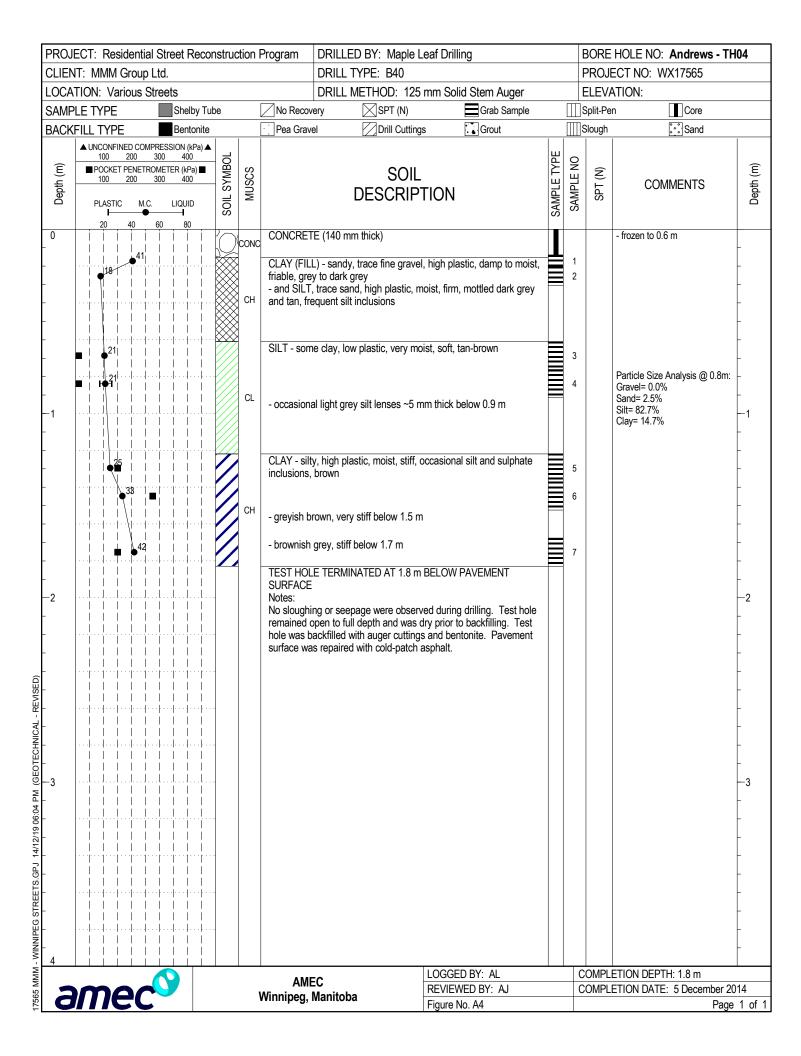
# **APPENDIX A**

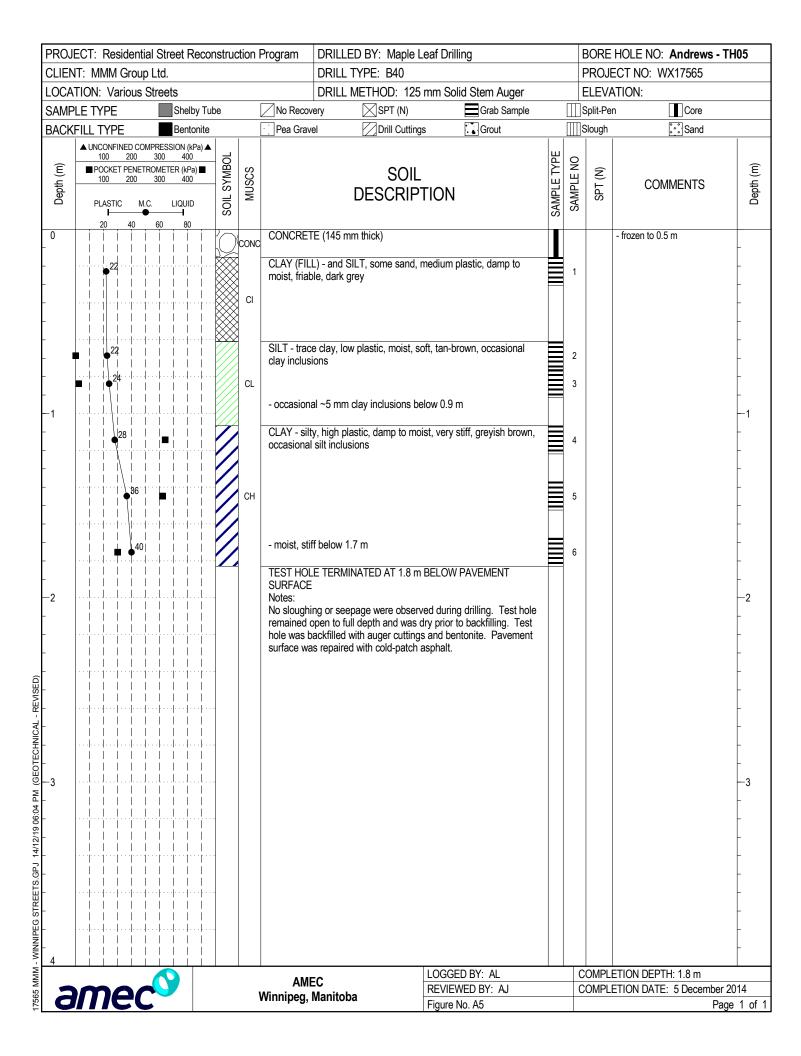
# **TEST HOLE LOGS**

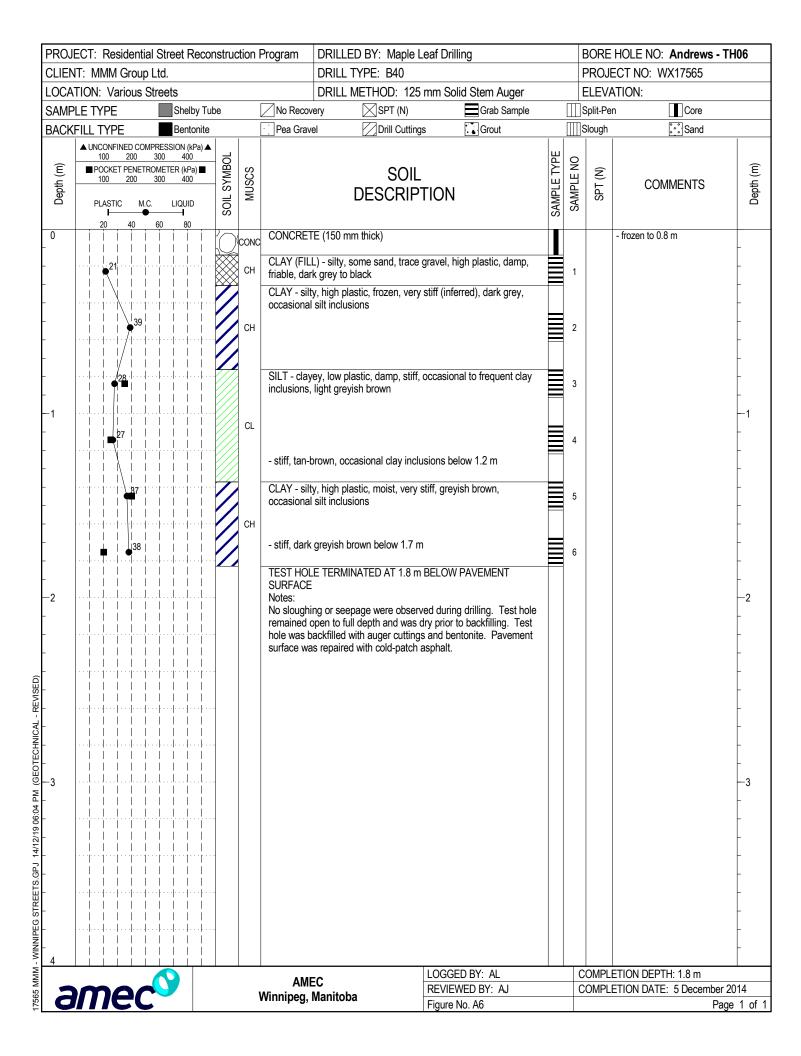


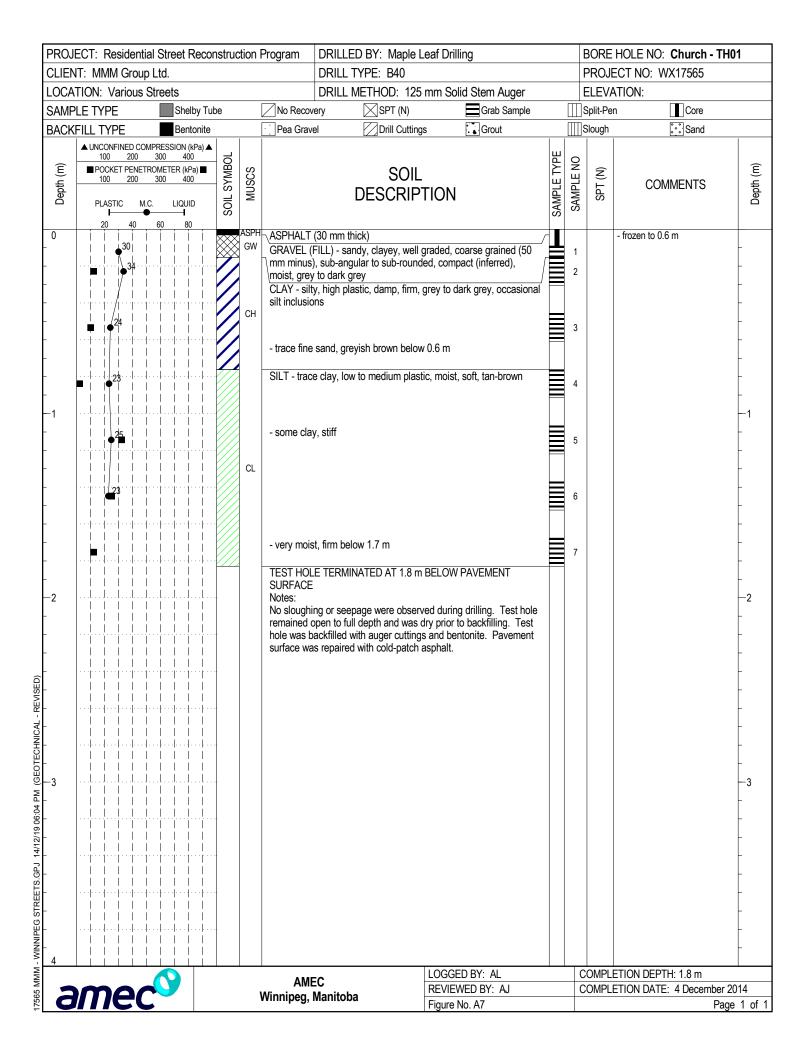


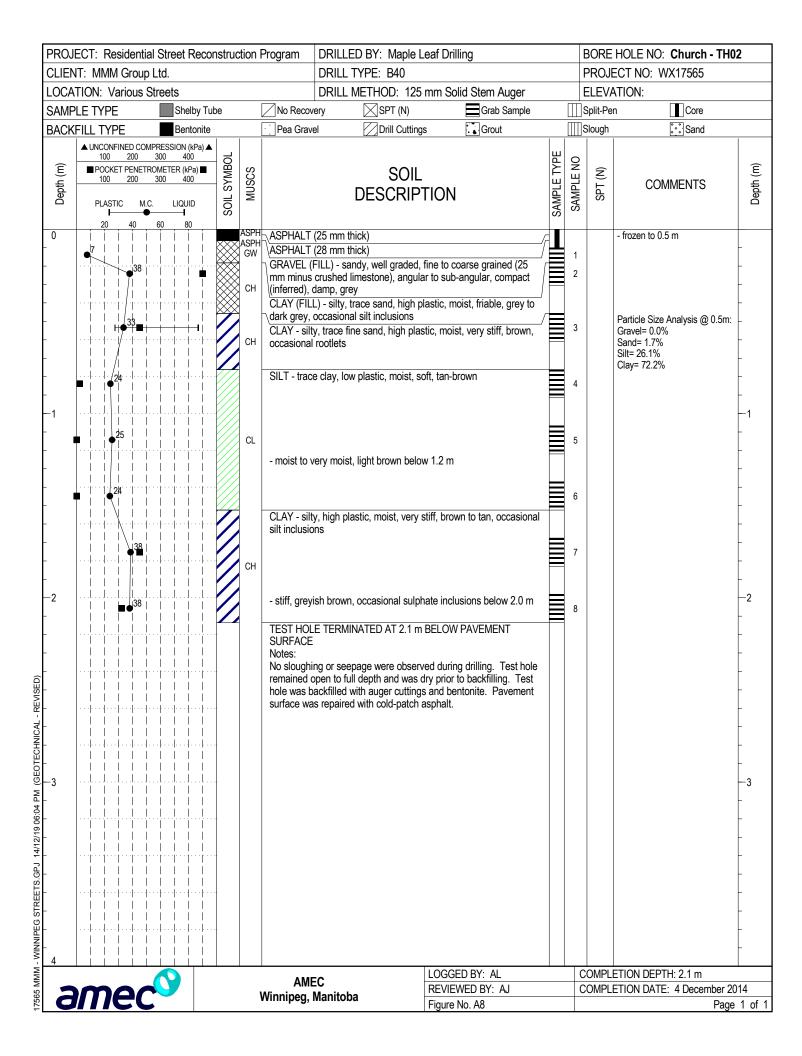


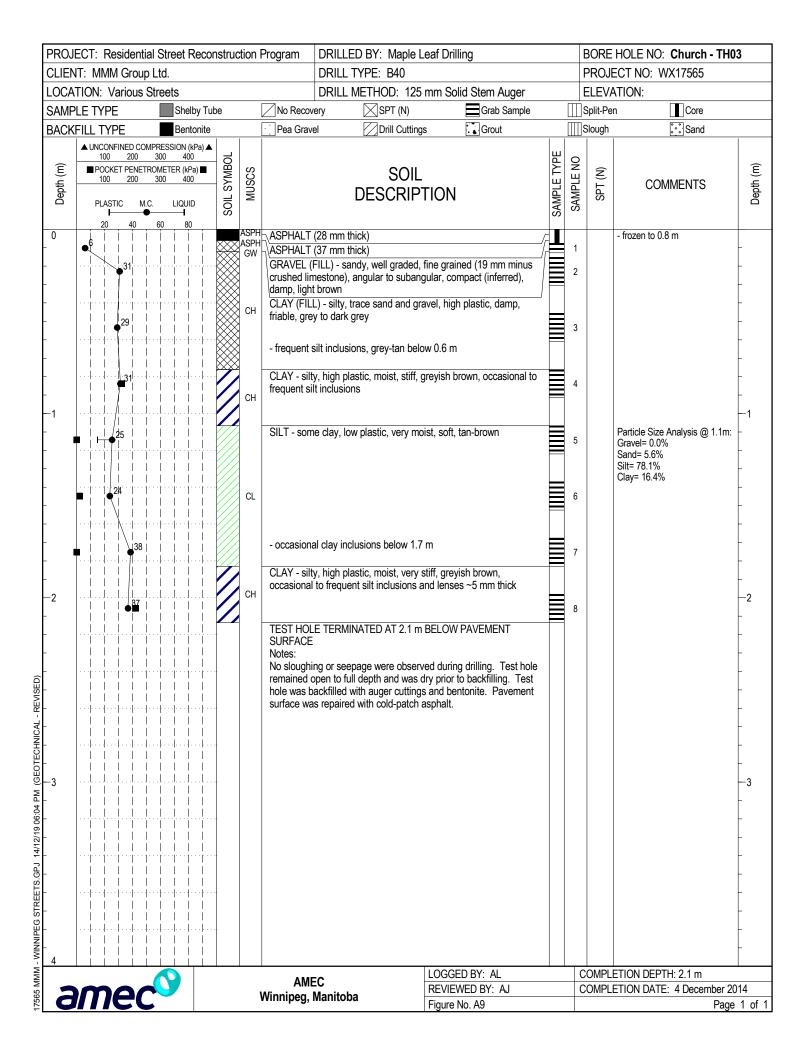


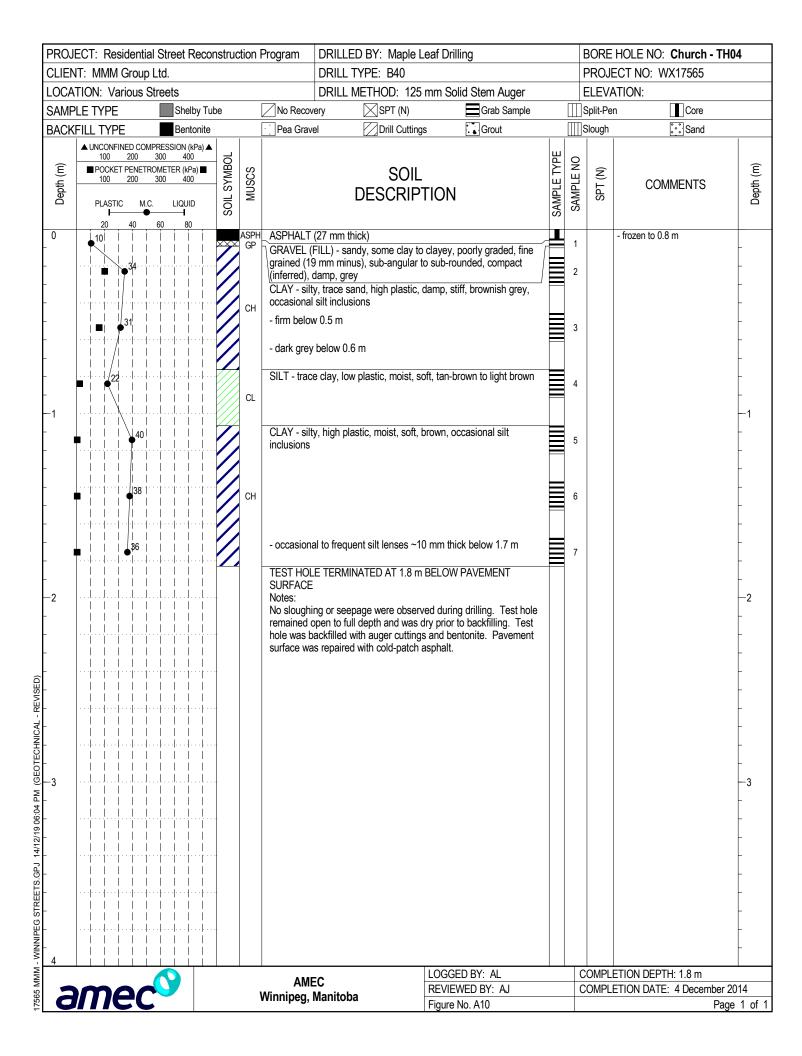


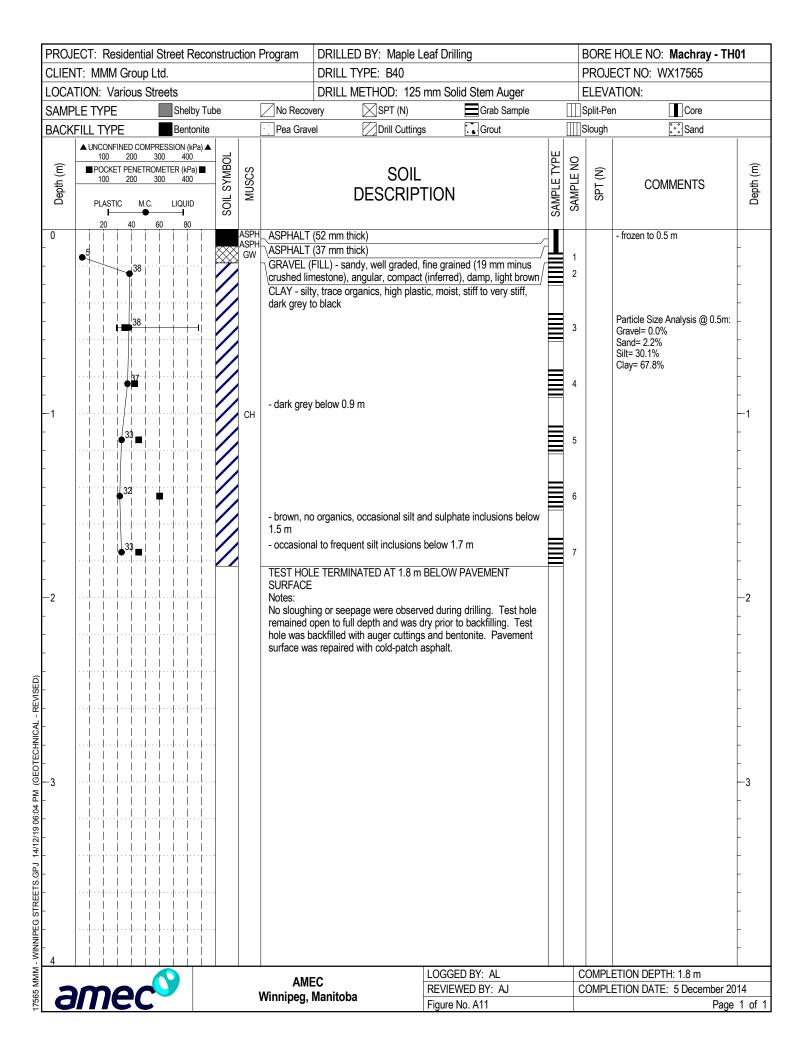


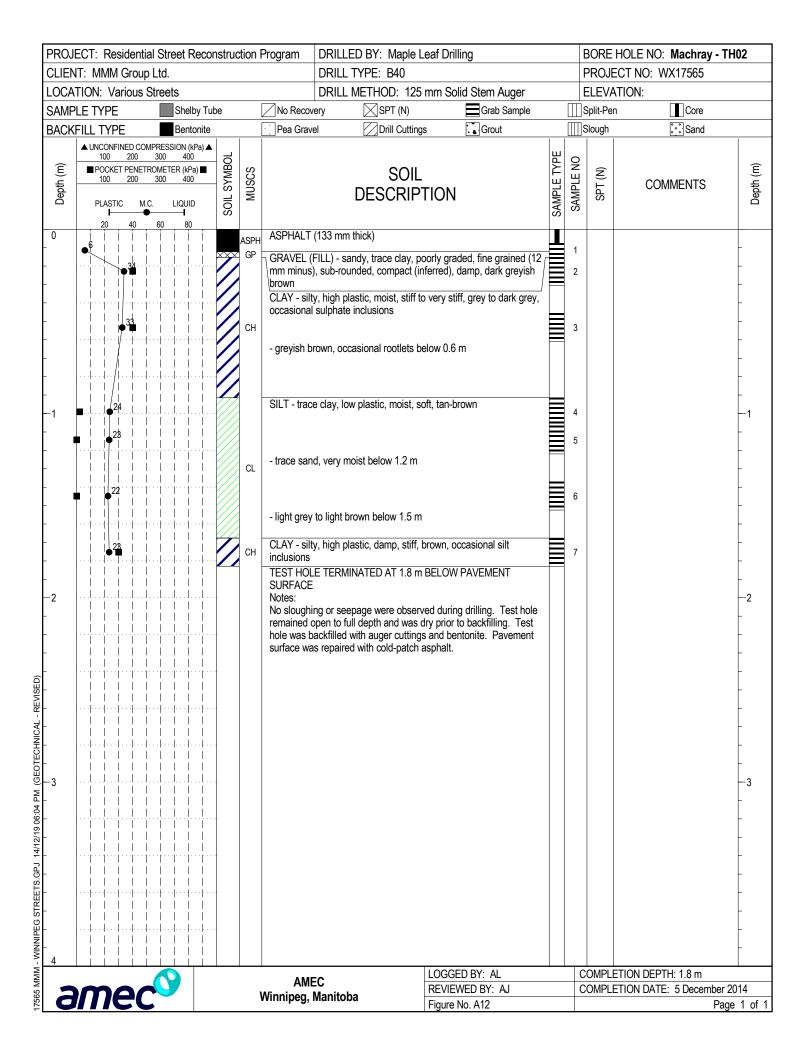


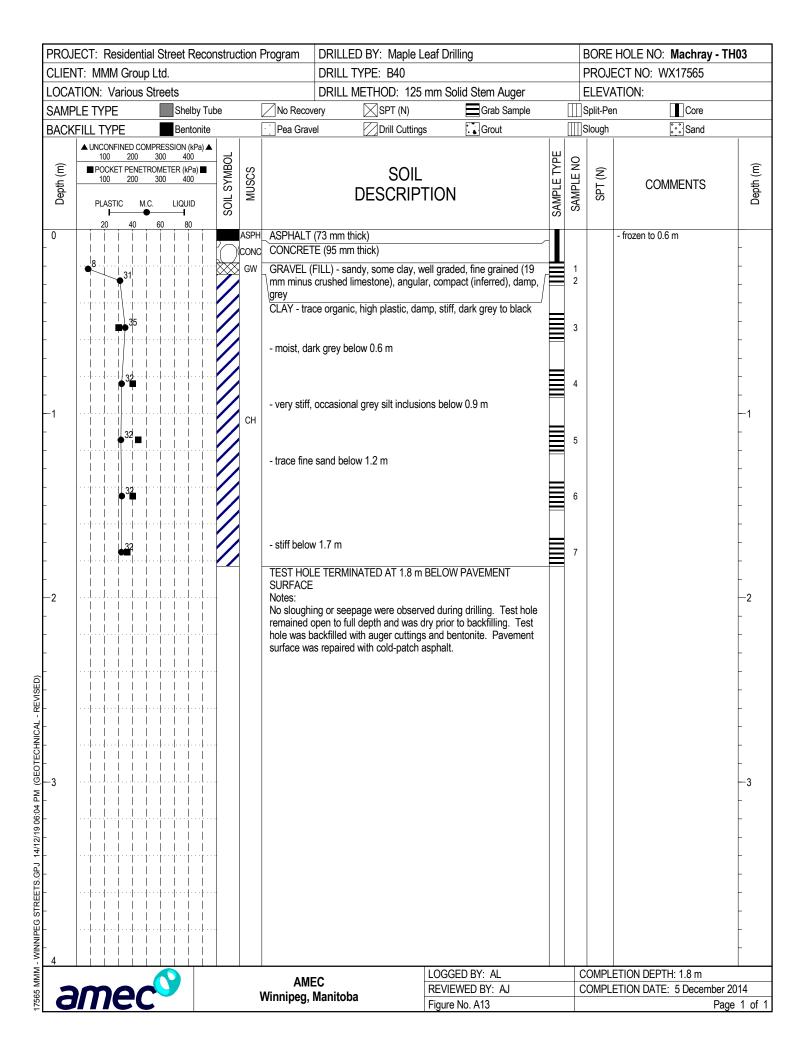


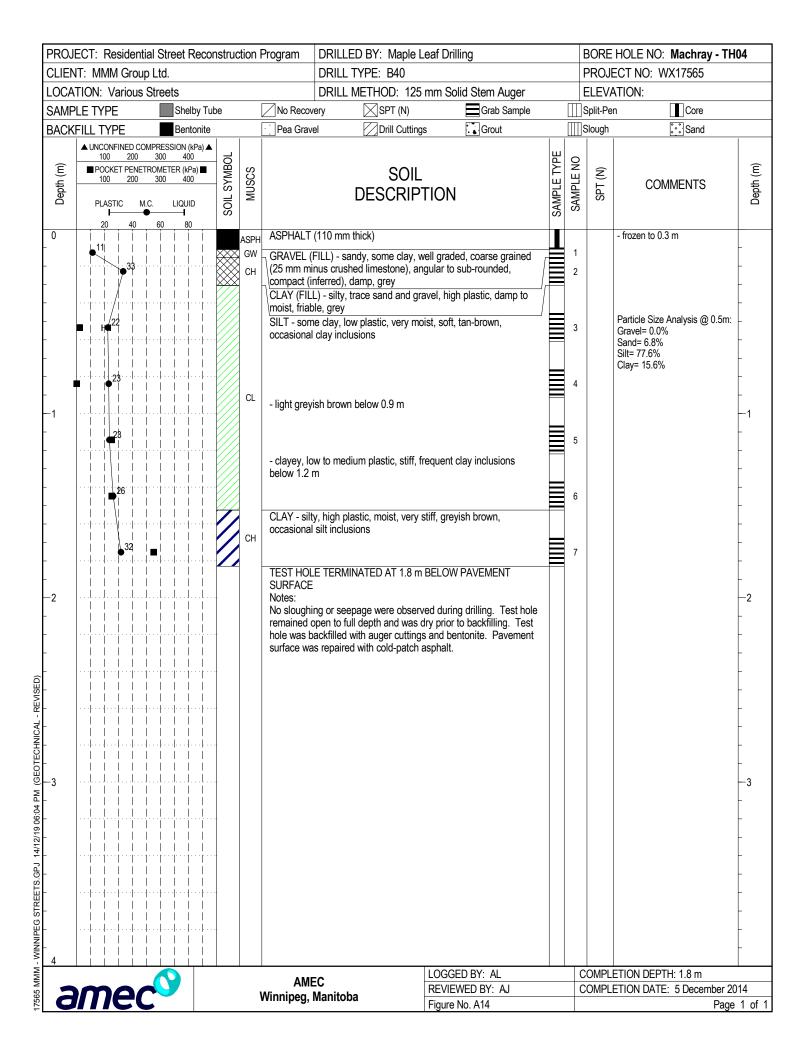


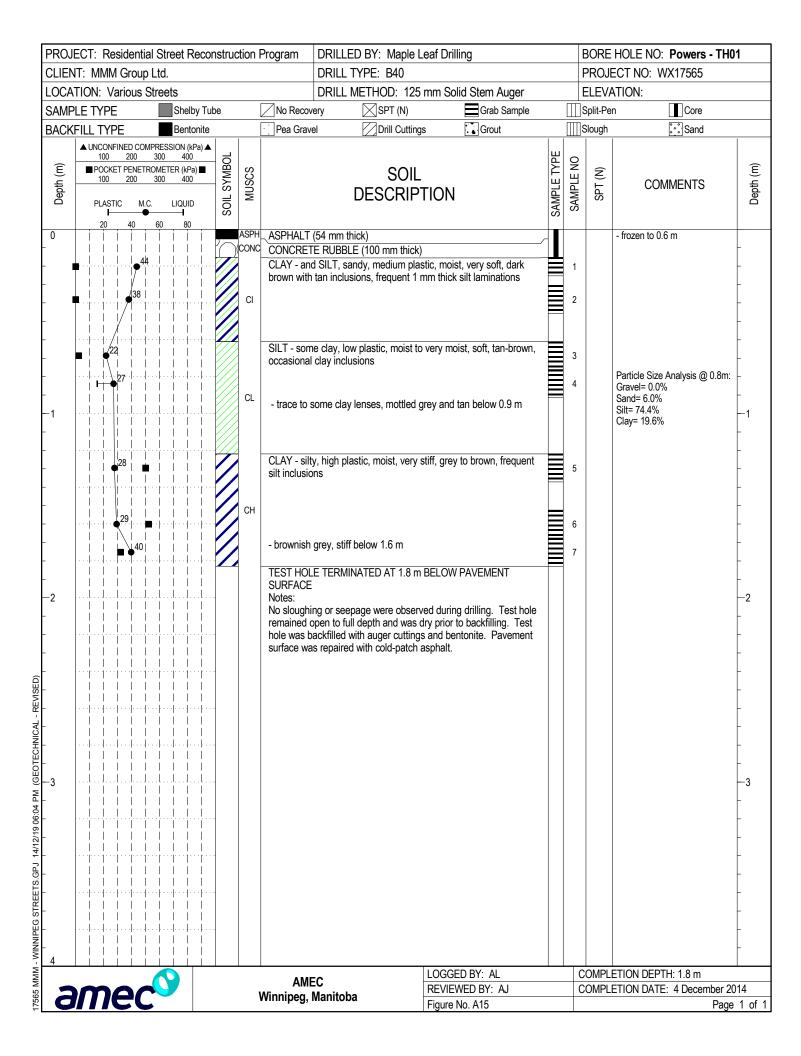


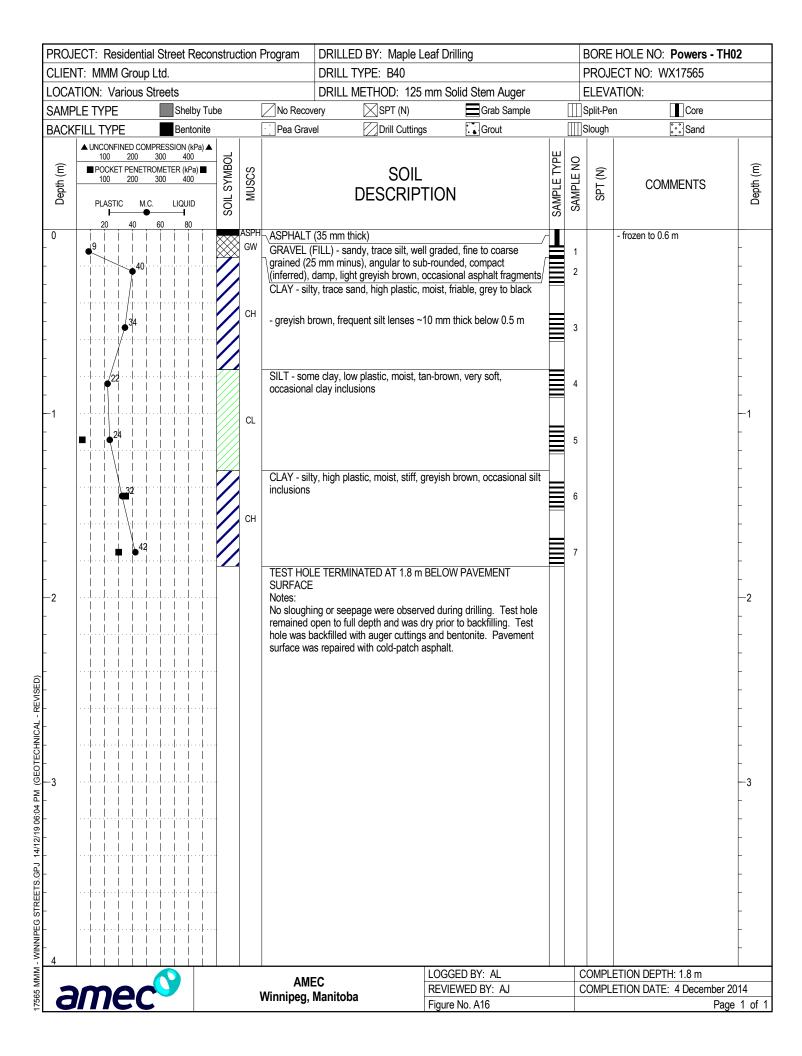


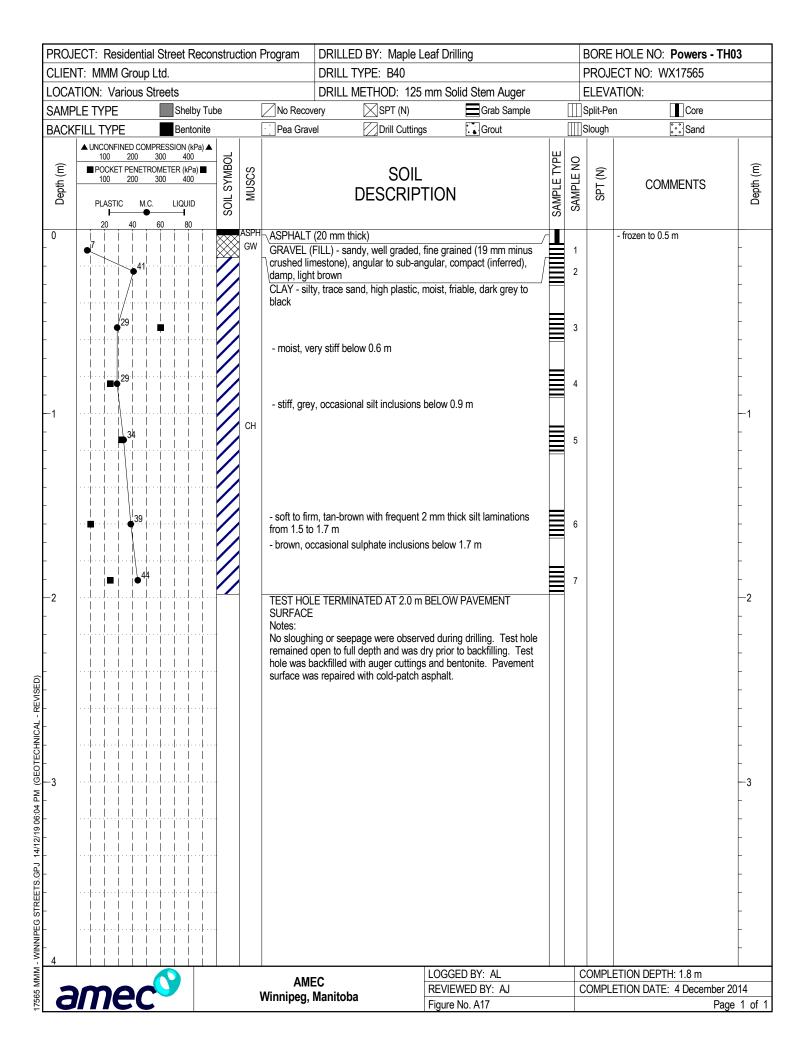


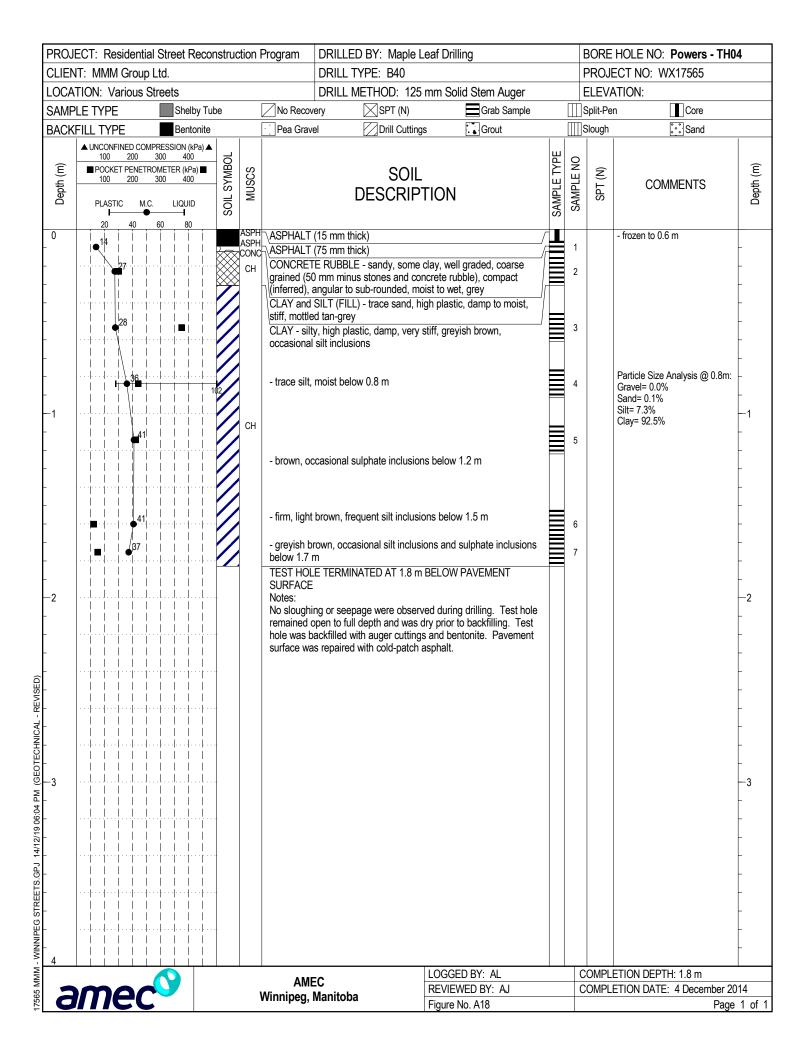


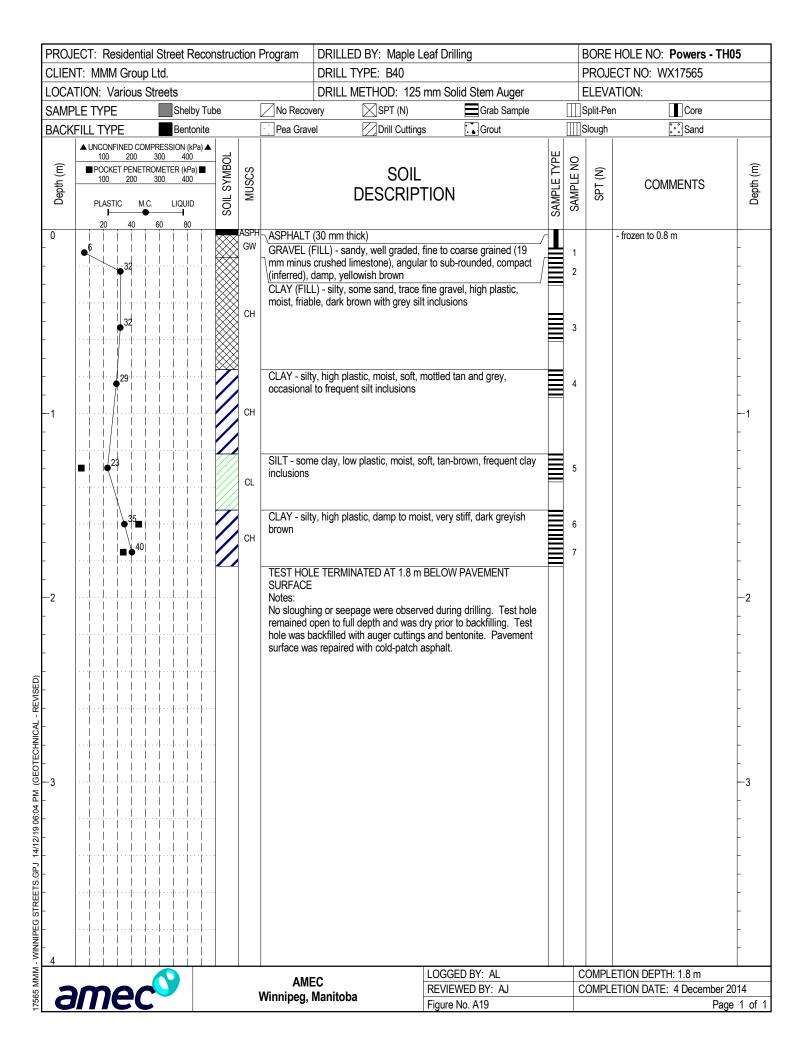


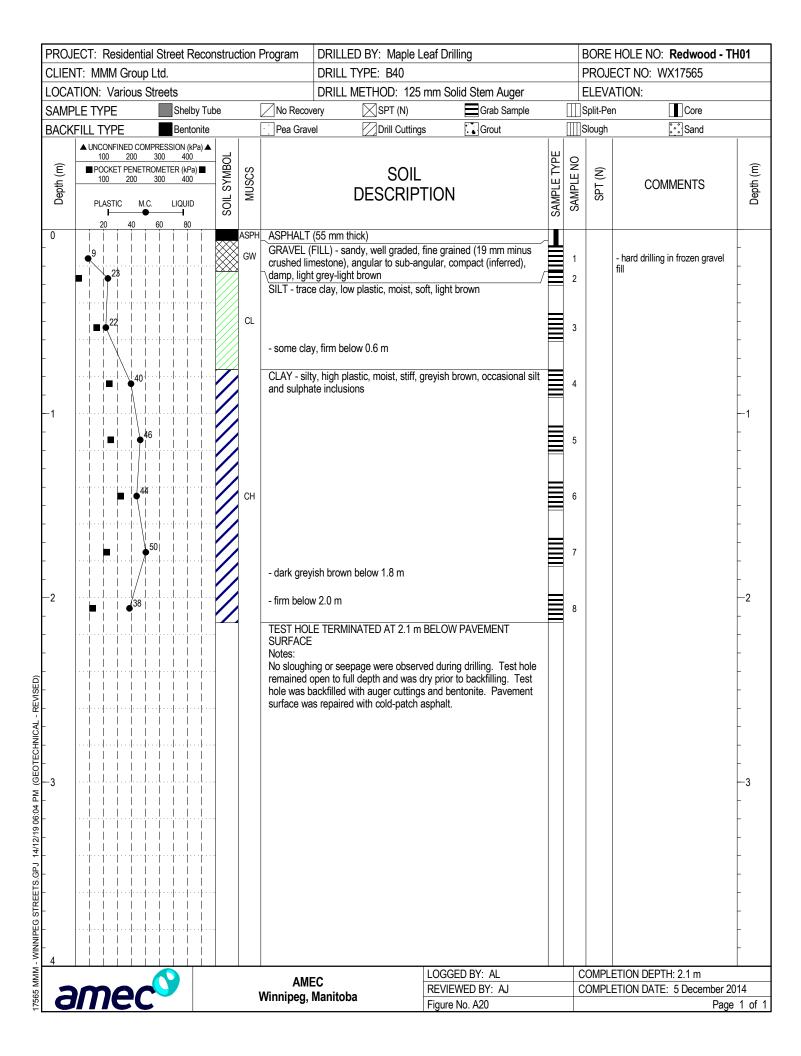


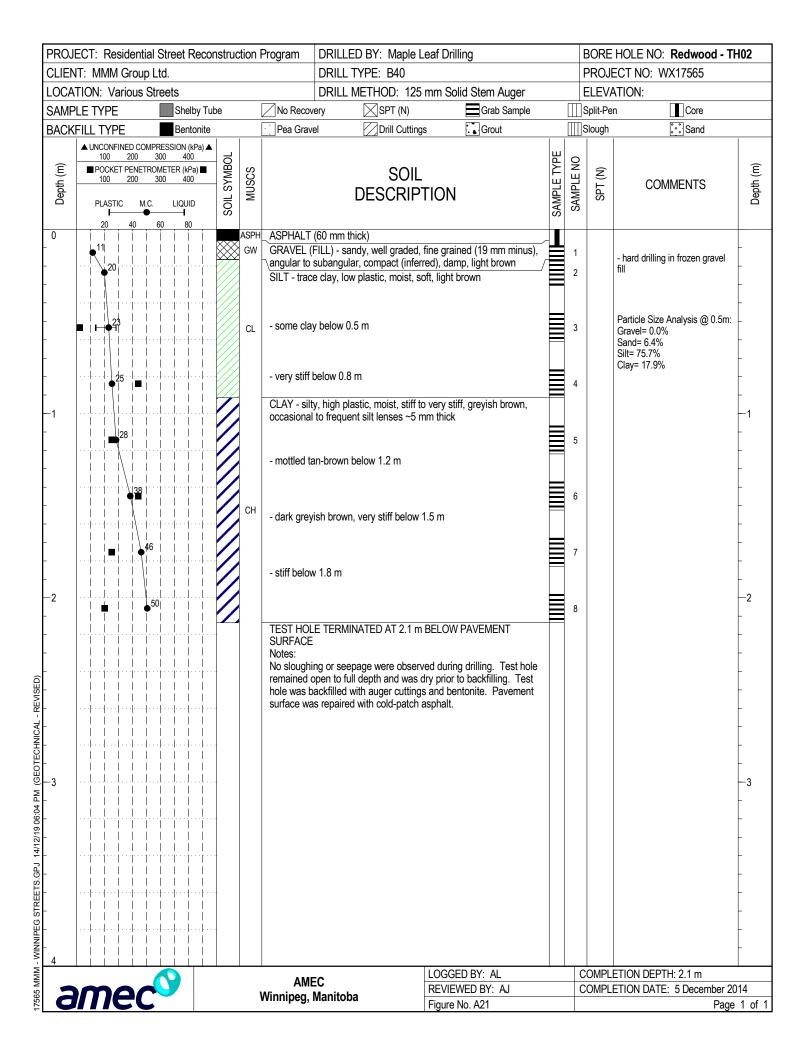


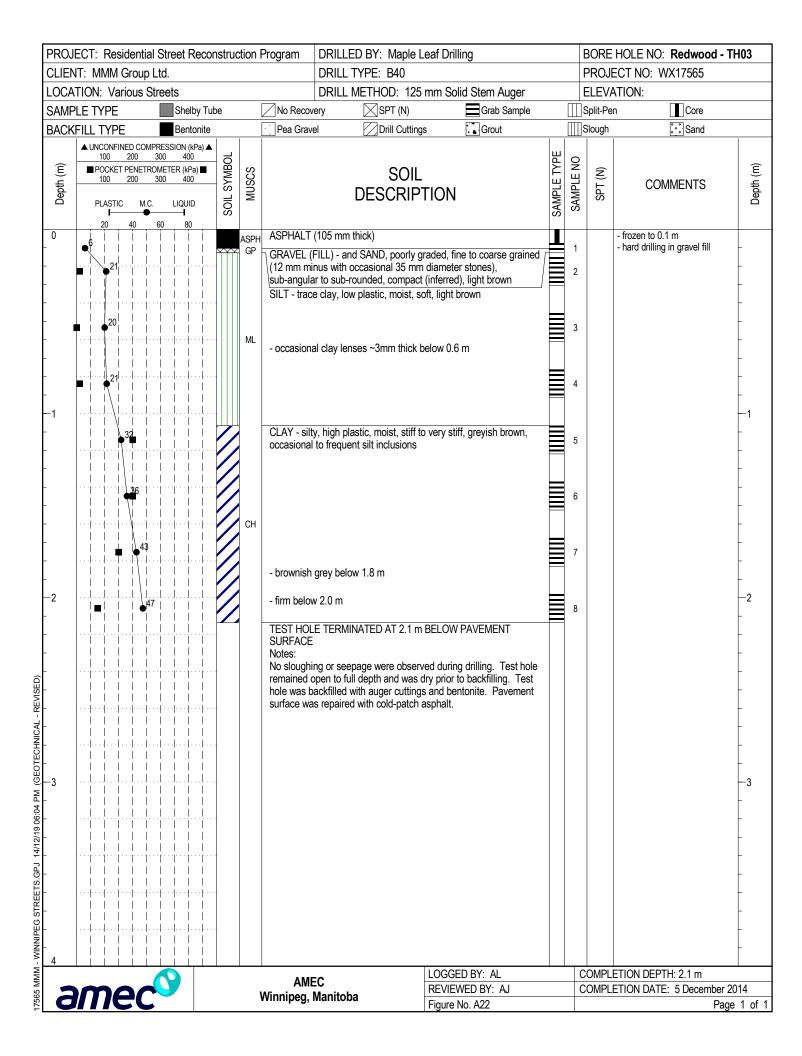


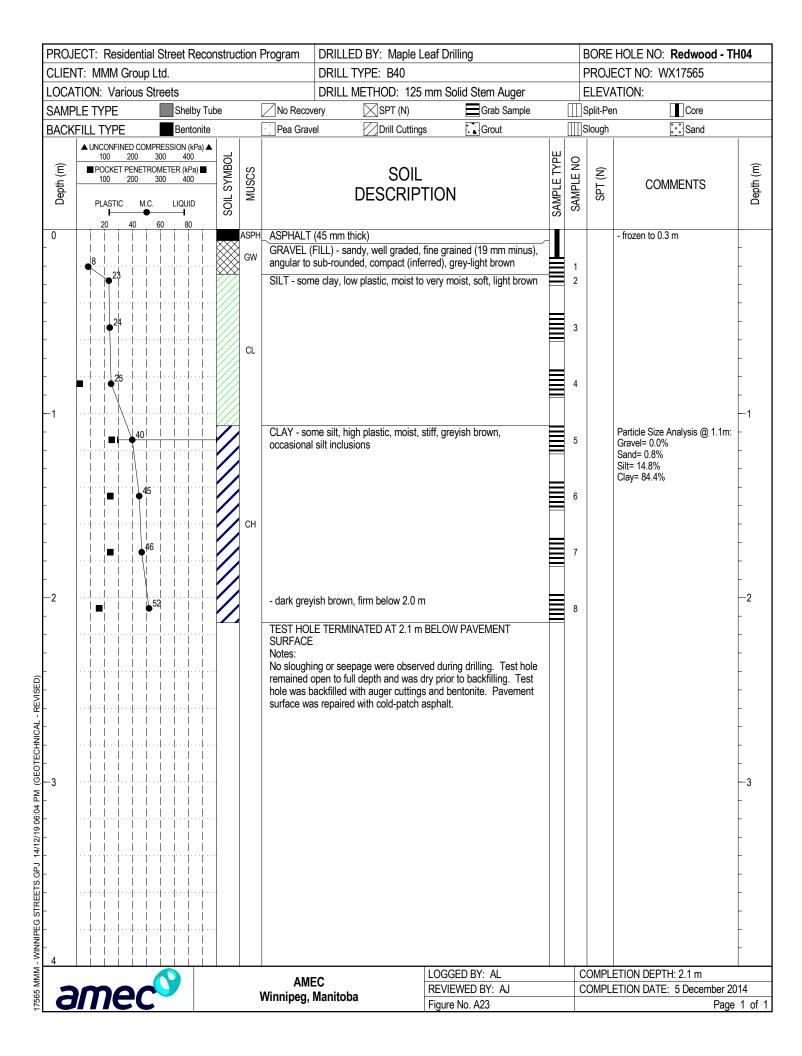


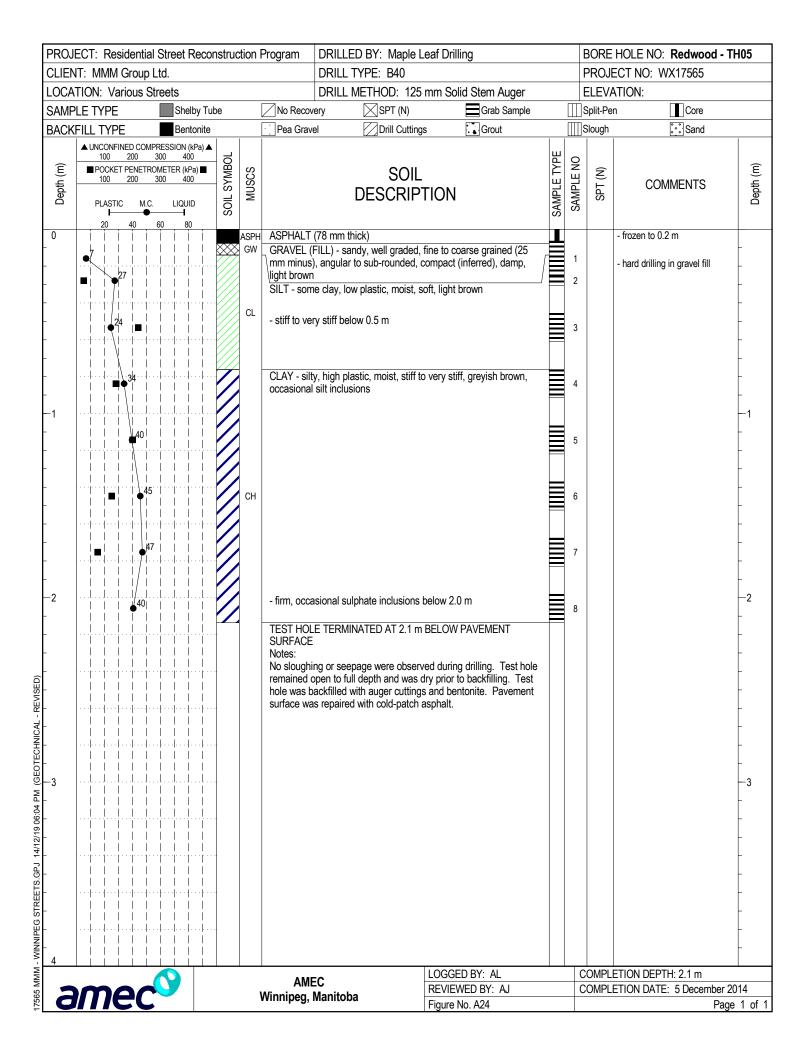












MMM Group Limited WX17565R1 - Geotechnical Investigation Residential Streets Reconstruction Winnipeg, Manitoba 21 January 2015



**APPENDIX B** 

**CORE PHOTOS** 



**Photo 1:** C01: Northbound lane at 884 Airlies Street, 1.0 m from curb



**Photo 2:** C02: Southbound lane at 856 Airlies Street, 1.0 m from curb

Environment and Infrastructure MMM Group			CHNICAL INVESTIGATIO AIRLIES STREET NNIPEG, MANITOBA	N
Drawn: N/A	Scale: N/A	December 2014	Project No.: WX17565	Figure: B1

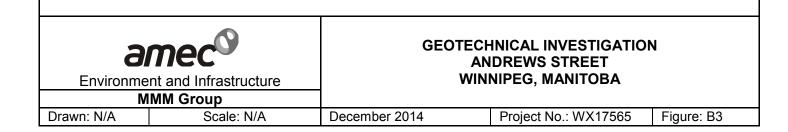




**Photo 1:** TH01: Southbound lane at 409 Jefferson Avenue, 1.0 m from curb



**Photo 2:** TH02: Northbound lane at 418 St. Anthony Avenue, 1.0 m from curb





**Photo 3:** TH03: Southbound lane at 417 St. Anthony Street, 1.0 m from curb



**Photo 4:** TH04: Northbound lane at 418 Perth Avenue, 1.0 m from curb

Environment and Infrastructure		AN	HNICAL INVESTIGATION IDREWS STREET NIPEG, MANITOBA	
Drawn: N/A	Scale: N/A	December 2014	Project No.: WX17565 Figure: B4	

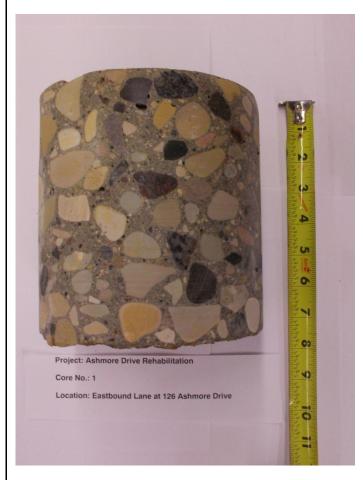


**Photo 5:** TH05: Southbound lane at 401 Perth Avenue, 1.0 m from curb

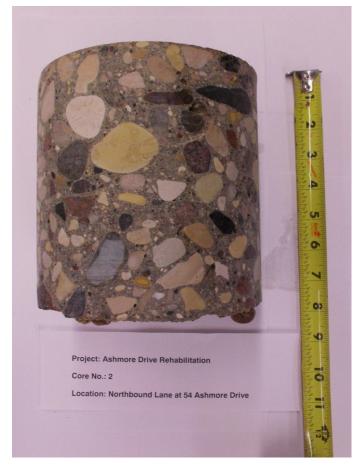


**Photo 6:** TH06: Northbound lane at 412 Hartford Avenue, 1.0 m from curb

Environment and Infrastructure		GEOTECHNICAL INVESTIGATION ANDREWS STREET WINNIPEG, MANITOBA		
MMM Group				
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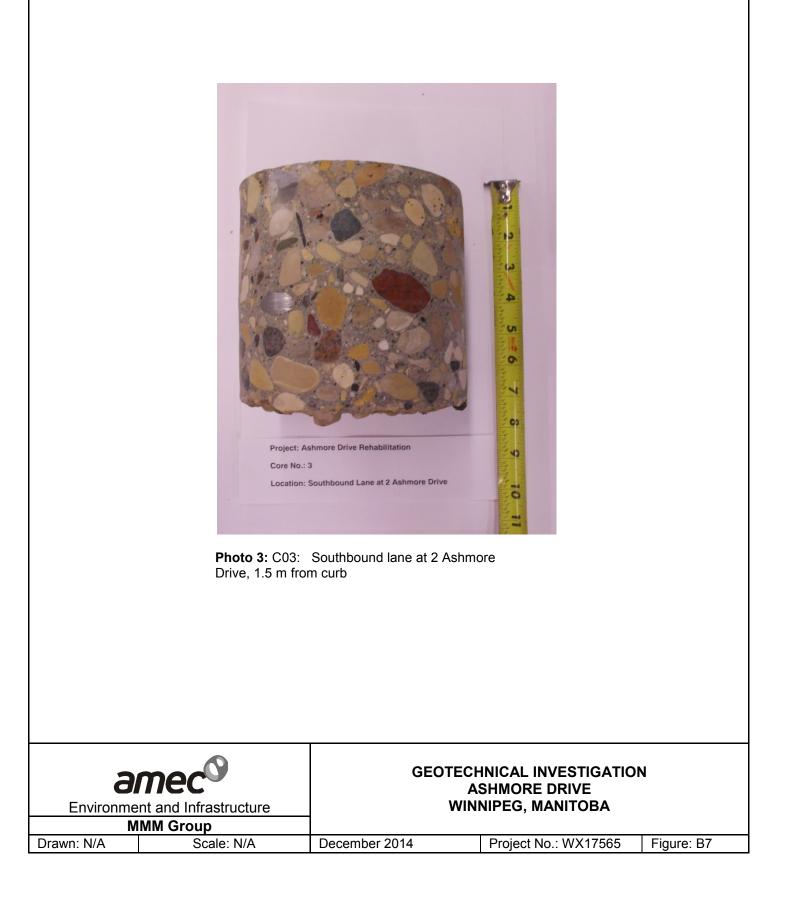


**Photo 1:** C01: Eastbound lane at 126 Ashmore Drive, 0.9 m from curb



**Photo 2:** C02: Northbound lane at 54 Ashmore Drive, 1.0 m from curb

amec		GEOTECHNICAL INVESTIGATION ASHMORE DRIVE			
Environme	ent and Infrastructure	VII	NNIPEG, MANITOBA		
MMM Group					
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**Photo 1:** C01: Eastbound lane 100 m east of Keewatin Street



Photo 2: C02: Westbound lane 20 m west of Dorset Street

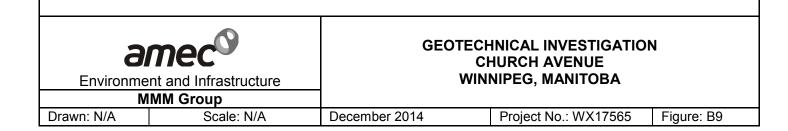
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MMM Group				
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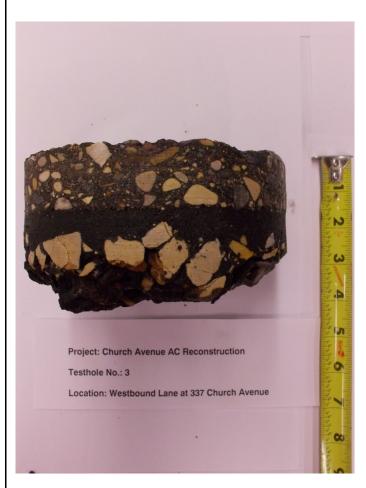


**Photo 1:** C01: Westbound lane at 464 Aikins Street, 1.0 m from curb



**Photo 2:** C02: Eastbound lane at 311 Church Avenue, 1.0 m from curb

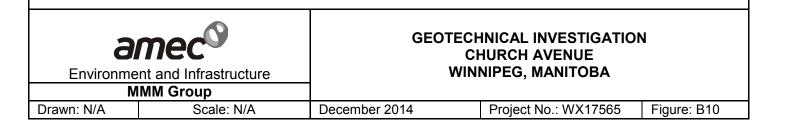


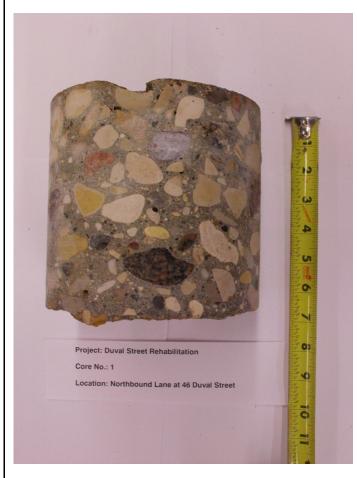


**Photo 3:** C03: Westbound lane at 337 Church Avenue, 1.5 m from curb

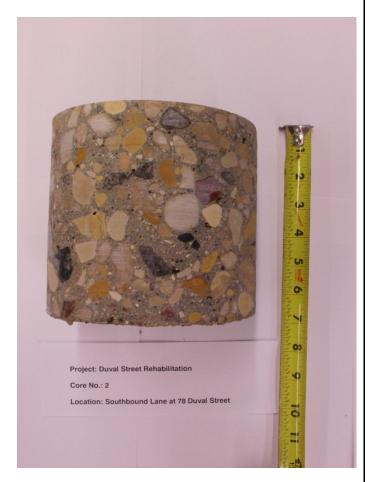


**Photo 4:** C04: Eastbound lane at 350 Church Avenue, 1 m from curb





**Photo 1:** C01: Northbound lane at 46 Duval Street, 0.5 m from curb

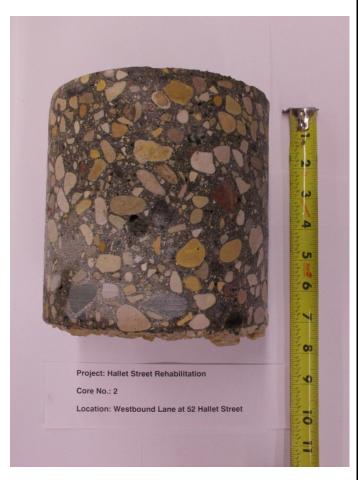


**Photo 2:** C02: Southbound lane at 78 Duval Street, 0.6 m from curb

Environment and Infrastructure		1	HNICAL INVESTIGATIO DUVAL STREET NIPEG, MANITOBA	N
Drawn: N/A	Scale: N/A	December 2014	Project No.: WX17565	Figure: B11



**Photo 1:** C01: Eastbound lane at 133 Hallet Street, 0.9 m from curb



**Photo 2:** C02: Westbound lane at 52 Hallet Street, 1.0 m from curb

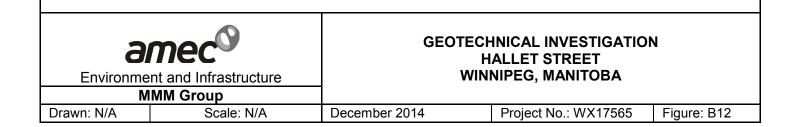


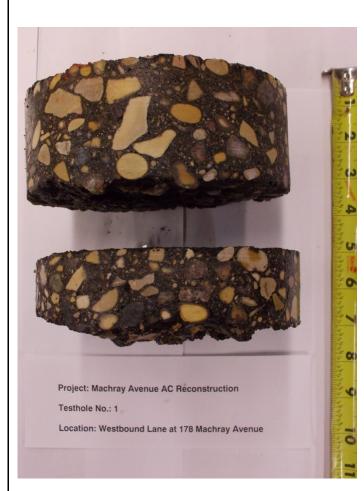
Photo 3: C03: Westbound lane at 105 Hallet Street, 2.0 m from curb

Photo 4: C04: Eastbound lane between 72 and 78 Hallet Street, 1.5 m from curb

Environment and Infrastructure		GEOTECHNICAL INVESTIGATION HALLET STREET WINNIPEG, MANITOBA		
MMM Group				
Drawn: N/A	Scale: N/A	January 2015	Project No.: WX17565	Figure: B13
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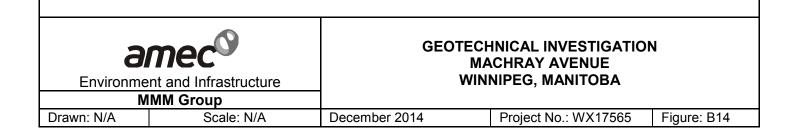




**Photo 1:** TH01: Westbound lane at 178 Machray Avenue, 1.0 m from curb



**Photo 2:** TH02: Eastbound lane at 177 Machray Avenue, 1.0 m from curb





**Photo 3:** TH03: Eastbound lane between 155 and 167 Machray Avenue, 1.0 m from curb



**Photo 4:** TH04: Westbound lane at 145 Machray Avenue, 1.0 m from curb

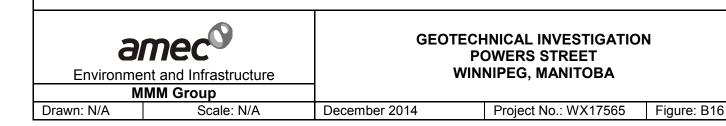
Environment and Infrastructure		GEOTECHNICAL INVESTIGATION MACHRAY AVENUE WINNIPEG, MANITOBA		
MMM Group				
Drawn: N/A	Scale: N/A	December 2014	Project No.: WX17565	Figure: B15



**Photo 1:** TH01: Northbound lane at 470 Burrows Avenue, 1.0 m from curb



**Photo 2:** TH02: Southbound lane at 210 Powers Street, 1.5 m from curb



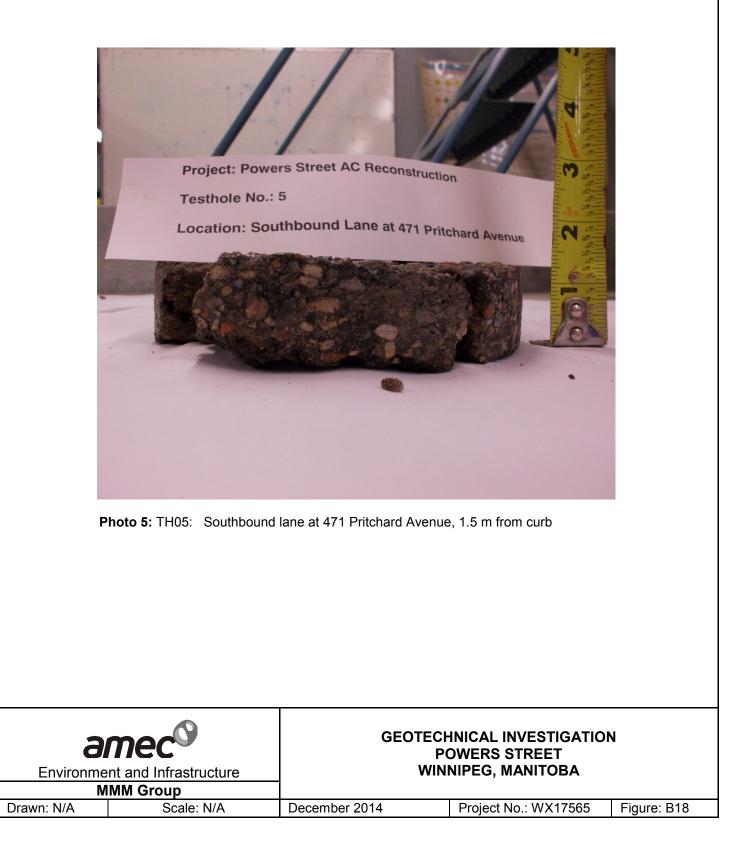


**Photo 3:** Testhole 03: Southbound lane at 201 Powers Street, 1.5 m from curb



**Photo 4:** Testhole 04: Northbound lane at 572 Manitoba Avenue, 1 m of curb

Environment and Infrastructure		PC	INICAL INVESTIGATION OWERS STREET NIPEG, MANITOBA
Drawn: N/A	Scale: N/A	December 2014	Project No.: WX17565 Figure: B17



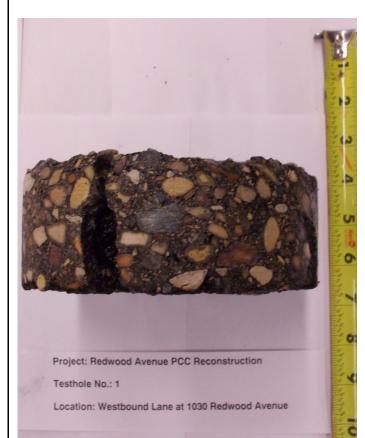


Photo 1: TH01: Westbound lane at 1030 Redwood Avenue, 1.0 m from curb



Photo 2: TH02: Westbound lane at 1035 Redwood Avenue, 1.0 m from curb



Drawn: N/A

## **GEOTECHNICAL INVESTIGATION REDWOOD AVENUE (BATTERY TO SGT. TOMMY PRINCE)** WINNIPEG, MANITOBA

Scale: N/A

December 2014

Project No.: WX17565

<image><image><text><text>

**Photo 3:** TH03: Eastbound lane at 1050 Redwood Avenue, 1.0 m from curb



**Photo 4:** TH04: Westbound lane at 1066 Redwood Avenue, 1.0 m from curb

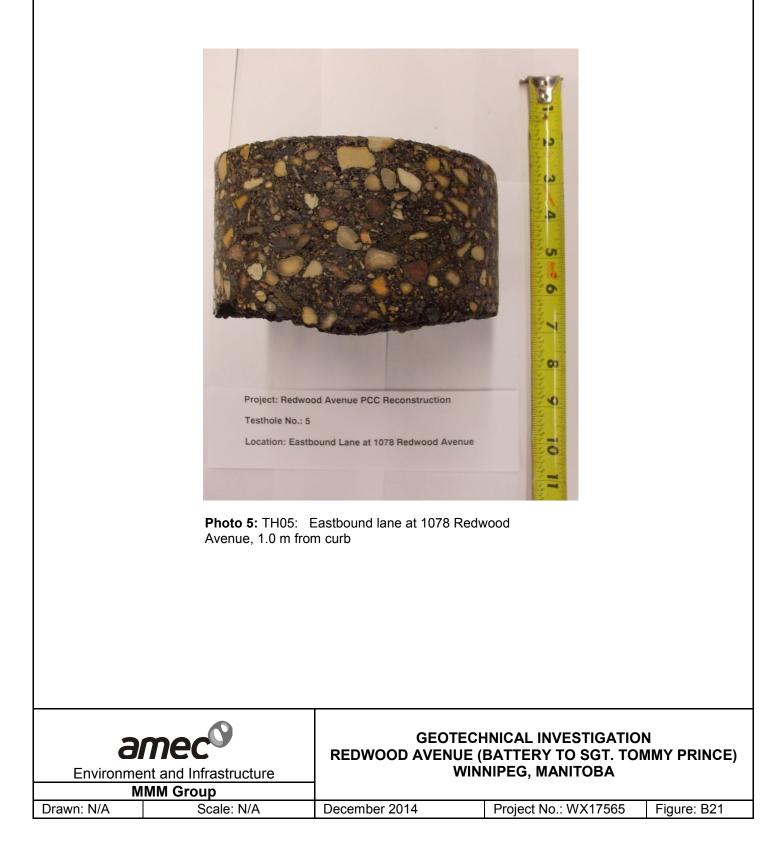
Environment and Infrastructure MMM Group		<b>REDWOOD AVENUE</b> (	HNICAL INVESTIGATIO BATTERY TO SGT. TOM NIPEG, MANITOBA	
Drawn: N/A	Scale: N/A	December 2014	Project No.: WX17565	Figure: B20

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**Photo 1:** C01: Westbound lane 2m east of west corner of 19 Bentall Street, 1.0 m from curb



**Photo 2:** C02: Eastbound lane 75m west of Bentall Street, 1.5 m from curb

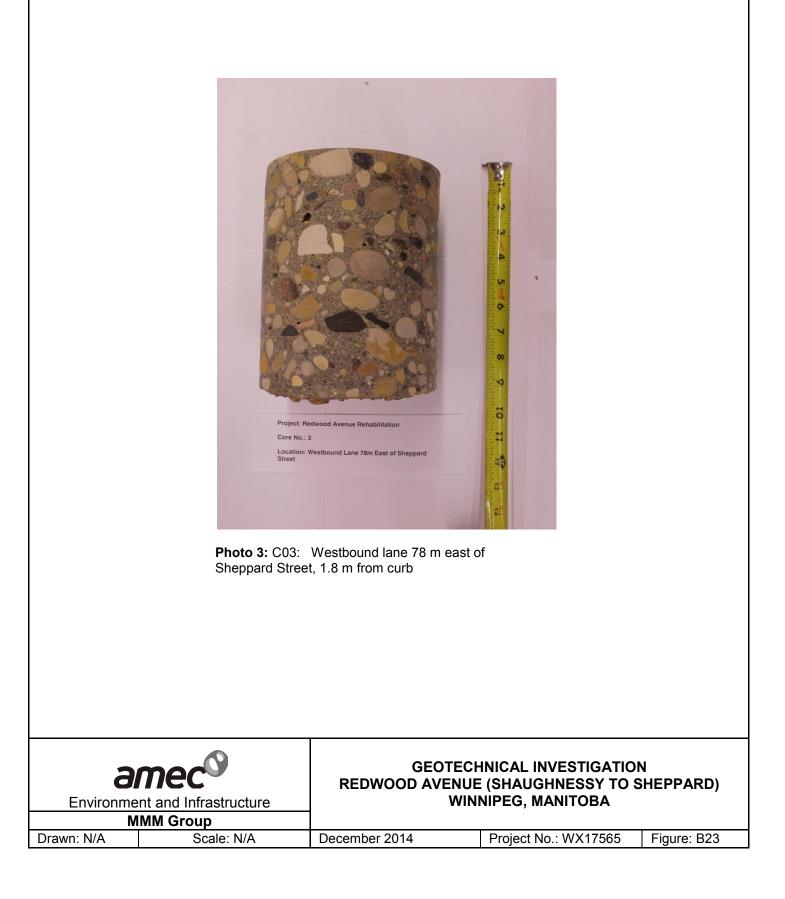


## GEOTECHNICAL INVESTIGATION REDWOOD AVENUE (SHAUGHNESSY TO SHEPPARD) WINNIPEG, MANITOBA

December 2014

Project

Project No.: WX17565 F





**Photo 1:** C01: Northbound lane at 67 Rose Hill Way, 0.3 m from curb



**Photo 2:** C02: 2.0 m from outside curb at 131 Rose Hill Way

