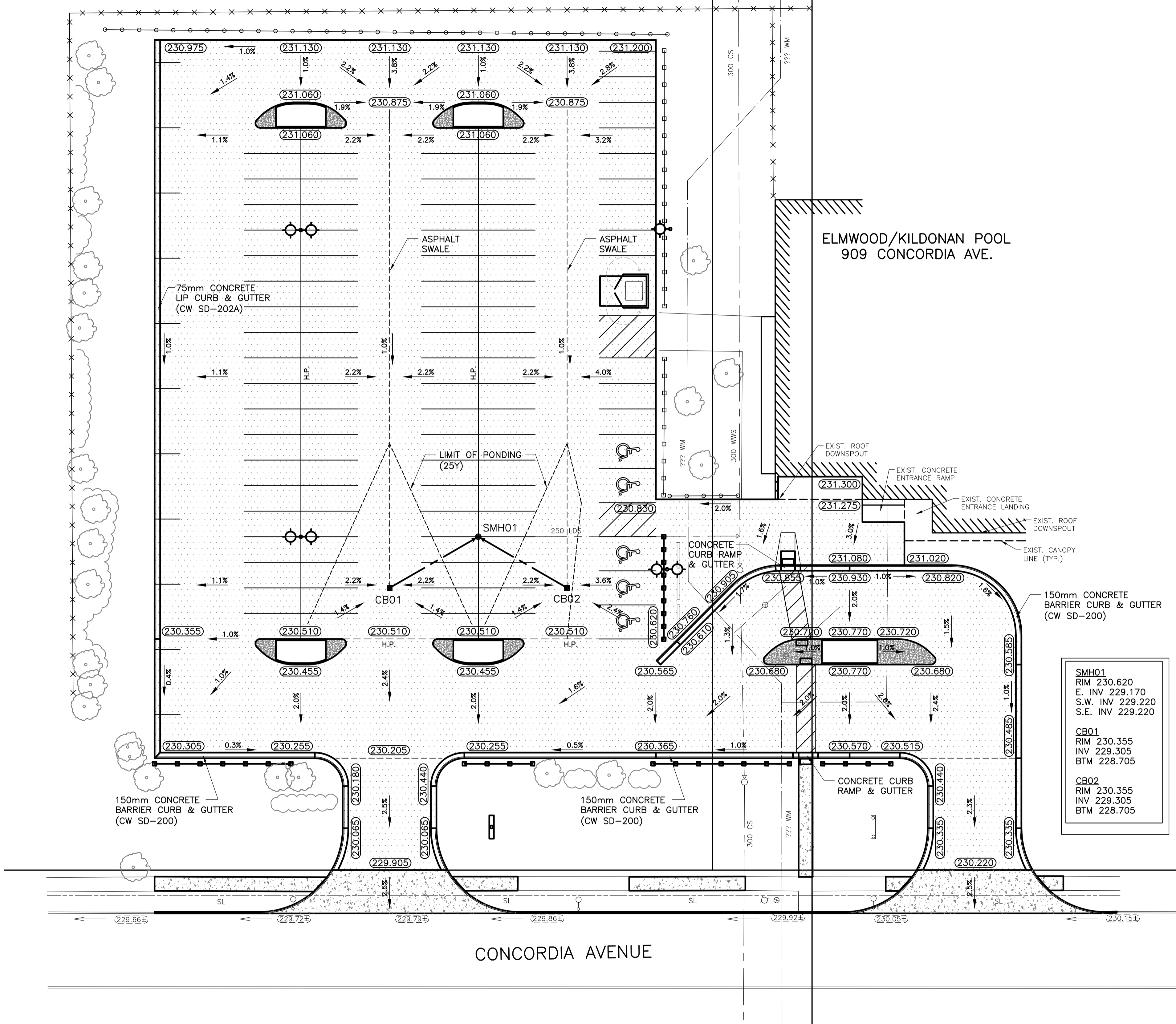


250 LDS



STORM WATER DISCHARGE CRITERIA

- NO SEPERATE LAND DRAINAGE SYSTEM ON CONCORDIA AVENUE IN FRONT OF PARKING LOT.
- EXISTING PARKING LOT CATCH BASIN IS CURRENTLY CONNECTED TO A DEDICATED WASTE WATER SEWER LINE. UPON CONSULTATION WITH C.O.W. WATER WASTE, PROPOSED LAND DRAINAGE RUN-OFF TO EXISTING WASTE WATER SEWER LINE SHALL BE LIMITED TO THE EXISTING CATCH BASIN DRAINAGE AREA. REMAINING DEVELOPMENT AREA TO BE DRAINED THROUGH PARKING LOT APPROACHES.

TOTAL CATCHMENT AREA = 0.88 HA = 2.18 ACRES
 INTERNAL LAND DRAINAGE CATCHMENT AREA = 0.39 HA = 0.96 ACRES

ISOCHRONE METHOD based on calculating impervious runoff based on filling depression storage and pervious runoff based on Horton infiltration equation

PROJECT: EXP PARKING LOT	LOCATION: 909 CONCORDIA	DATE: MARCH 24/11
Impervious area, surface storage capacity	0.10	Total Impervious Area
Horton Equation		0.80 acres
metric used in SWMM		Total Pervious Area
to (in/hr)	3.00	0.90 acres
to (mm/hr)	0.118	3.72 cfs
to (mm)	-0.009	1154 m ³
		Required Site Storage (10 min)
		33 m ³

Allowable offsite runoff (5yr storm)	3.72 cfs
Enter "C" Value =	0.900
Enter from 5 year storm	4.300
in Acres	0.960

C value Assigned by City of Winnipeg
 Please use a Time of Concentration of 10 min for Commercial developments

City of Wpg 25 yr storm		Pervious Flow Calculation		Total per. + imp. flow		allowable discharge		net runoff requiring storage		mass sum of storage	
Time (min)	discretized storm (in/hr)	rainfall depth (in)	sum of rainfall depths (in)	infiltration (in/hr)	intensity after infiltration (in/hr)	pervious runoff (cfs)	total per. (cfs)	total per. + imp. flow (cfs)	allowable discharge (cfs)	net runoff (cfs)	mass sum of storage (m ³)
0	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0
5	0.12	0.01	0.01	0.00	0.12	0.12	0.12	0.12	0.12	0.00	0
10	0.15	0.01	0.02	0.00	0.15	0.15	0.15	0.15	0.15	0.00	0
15	0.15	0.01	0.03	0.00	0.14	0.14	0.14	0.14	0.14	0.00	0
20	0.17	0.01	0.04	0.00	0.14	0.14	0.14	0.14	0.14	0.00	0
25	0.19	0.02	0.06	0.00	0.13	0.13	0.13	0.13	0.13	0.00	0
30	0.21	0.02	0.08	0.00	0.12	0.12	0.12	0.12	0.12	0.00	0
35	0.23	0.02	0.10	0.00	0.11	0.11	0.11	0.11	0.11	0.00	0
40	0.25	0.02	0.12	0.00	0.10	0.10	0.10	0.10	0.10	0.00	0
45	0.29	0.02	0.15	0.00	0.08	0.08	0.08	0.08	0.08	0.00	0
50	0.33	0.03	0.17	0.00	0.06	0.06	0.06	0.06	0.06	0.00	0
55	0.42	0.04	0.21	0.00	0.04	0.04	0.04	0.04	0.04	0.00	0
60	0.53	0.04	0.25	0.00	0.02	0.02	0.02	0.02	0.02	0.00	0
65	0.75	0.05	0.32	0.01	0.15	0.15	0.15	0.15	0.15	0.00	0
70	1.24	0.10	0.42	0.09	0.14	0.14	0.14	0.14	0.14	0.00	0
75	2.96	0.25	0.67	0.23	0.13	0.13	0.13	0.13	0.13	0.00	0
80	7.68	0.66	1.32	0.29	0.13	0.13	0.13	0.13	0.13	0.00	0
85	3.93	0.33	1.65	0.14	0.13	0.13	0.13	0.13	0.13	0.00	0
90	2.29	0.19	1.84	0.13	0.12	0.12	0.12	0.12	0.12	0.00	0
95	1.54	0.13	1.97	0.12	0.12	0.12	0.12	0.12	0.12	0.00	0
100	1.17	0.10	2.07	0.12	0.11	0.11	0.11	0.11	0.11	0.00	0
105	0.92	0.08	2.14	0.12	0.10	0.10	0.10	0.10	0.10	0.00	0
110	0.75	0.06	2.20	0.12	0.09	0.09	0.09	0.09	0.09	0.00	0
115	0.65	0.05	2.26	0.12	0.08	0.08	0.08	0.08	0.08	0.00	0
120	0.58	0.04	2.31	0.12	0.07	0.07	0.07	0.07	0.07	0.00	0
125	0.51	0.04	2.35	0.11	0.07	0.07	0.07	0.07	0.07	0.00	0
130	0.45	0.04	2.39	0.11	0.06	0.06	0.06	0.06	0.06	0.00	0
135	0.40	0.03	2.42	0.12	0.05	0.05	0.05	0.05	0.05	0.00	0
140	0.35	0.03	2.45	0.12	0.04	0.04	0.04	0.04	0.04	0.00	0
145	0.30	0.03	2.47	0.12	0.03	0.03	0.03	0.03	0.03	0.00	0
150	0.26	0.02	2.50	0.11	0.02	0.02	0.02	0.02	0.02	0.00	0
155	0.22	0.02	2.51	0.11	0.02	0.02	0.02	0.02	0.02	0.00	0
160	0.19	0.02	2.53	0.11	0.01	0.01	0.01	0.01	0.01	0.00	0
165	0.17	0.01	2.54	0.14	0.01	0.01	0.01	0.01	0.01	0.00	0
170	0.16	0.01	2.56	0.13	0.01	0.01	0.01	0.01	0.01	0.00	0
175	0.15	0.01	2.57	0.12	0.01	0.01	0.01	0.01	0.01	0.00	0
180	0.14	0.01	2.59	0.11	0.01	0.01	0.01	0.01	0.01	0.00	0
185	0.13	0.01	2.59	0.10	0.01	0.01	0.01	0.01	0.01	0.00	0
190	0.12	0.01	2.60	0.10	0.00	0.00	0.00	0.00	0.00	0.00	0
195	0.12	0.01	2.61	0.10	0.00	0.00	0.00	0.00	0.00	0.00	0
											1154

Notes:
 1 - Isochrone Calculation Method defined in Sec 3.4.4 of MacLaren (1974)
 2 - rainfall intensity from Table 3.2 MacLaren (1974), note errors in table after 135 min as demonstrated by A. Barylak (Acres, Mar 2006) - see corrections in GM copy of MacLaren report
 3 - Summary of sheet updates: Source data based passed on to by David Shaw who got the sheet from an engineer from KGS, sheet reformatted and calculation columns adjusted by G. Mohr (May 2006). On Jan 31, 2007 corrected formula error in Column G. Note that I am suspect of the infiltration rates of 0.52 in/hr which seems high for our clay soils. On Feb 1, 2007 I reviewed the Horton equation coefficient and have modified to reflect a SIC3 Class 2 soil and allowed these coefficients to be input. May 23, 2007 - corrected cell reference H10 to reference cell K8. On Jul 30, 2007, based on data in the Handbook of Steel Drainage and Highway Construction which discusses the Horton method, I modify the coefficients to clay loam (see table 3.8, page 113). The impact of changing Fc from 0.05 in/hr to 0.31 in/hr increases the overall infiltration rate and decreases runoff.
 Sept 2, 2008 - reviewed Horton coeff used by other consultants in their SWMM modelling as well as reviewed William James SWMM textbook, I increased from 2.55 to 3.00 in/hr, I decreased from 0.39 to 0.118 in/hr, k decreased from -0.08 to -0.069 1/min. Added timestep Q to analysis.

Q_{ALLOWABLE} = 3.72 cfs = 0.105 cu.m.
 Q_{RESTRICTED} = 0.11 cu.m. (USING 225mm DIA. FLOW RESTRICTOR PLACED AT EACH CB OUTLET)

REQUIRED TOTAL ON-SITE STORAGE = 33 CU.M.
 AVAILABLE TOTAL ON-SITE STORAGE = 24 CU.M.
 REMAINING 8 CU.M. VOLUME TO SPILL OUT TOWARDS PARKING LOT APPROACHES ONCE 1:25 YEAR LIMIT OF PONDING REACHED.

REFER DRAWING C01 FOR GENERAL CONSTRUCTION NOTES

METRIC
 WHOLE NUMBERS INDICATE MILLIMETRES
 DECIMALIZED NUMBERS INDICATE METRES



150 mm WM	WATERMAIN	150 mm WM	---	GAS			
○	HYDRANT	●	---	HYDRO			
⊗	VALVE	⊗	---	M.T.S.			
300 mm LDS	LAND DRAINAGE SEWER	300 mm LDS	---	PEDESTAL			
250 mm CS	WASTE WATER SEWER	250 mm CS	---	GROUND ELEVATION	230.00		
○	MANHOLE	●	---	PAVEMENT ELEVATION	(230.000)		
□	CATCH BASIN	■	---	PROPERTY LINE			
△	CURB INLET	▲	---	SURVEY BAR			
+	SIGN	+	---	CURB STOP			
○	LAMP STANDARD	●	---	TESTHOLE			
•	UTILITY POLE	•	---				
EXISTING	LEGEND - PLAN	PROPOSED	EXISTING	LEGEND - PLAN	PROPOSED	EXISTING	LEGEND - PROFILE

LOCATION APPROVED UNDERGROUND STRUCTURES
 SUPV. U/G STRUCTURES COMMITTEE DATE
 NOTE:
 LOCATION OF UNDERGROUND STRUCTURES AS SHOWN ARE BASED ON THE BEST INFORMATION AVAILABLE BUT NO GUARANTEE IS GIVEN THAT ALL EXISTING UTILITIES ARE SHOWN OR THAT THE GIVEN LOCATIONS ARE EXACT. CONFIRMATION OF EXISTENCE AND EXACT LOCATION OF ALL SERVICES MUST BE OBTAINED FROM THE INDIVIDUAL UTILITIES BEFORE PROCEEDING WITH CONSTRUCTION.

C.W.B.M. 11-004..... 232.035m
 N.E. CORNER OF CONCORDIA AVE. & LONDON ST., TBLT. IN S. CONC. FOUNDATION, 1m E AND 0.2m BELOW TYNDALL STONE FROM THE S.W. CORNER OF THE MOST SLY. WALL OF KILDONAN EAST REGIONAL SCHOOL, NO. 845 CONCORDIA AVE.

GENIVAR
 10 PRAIRIE WAY WINNIPEG, MANITOBA R2J 3J8
 PH: (204)-477-8650 FAX: (204)-474-2864

DESIGNED BY R.C. CHECKED BY D.T.B.
 DRAWN BY R.C. APPROVED BY B.D.E.
 HOR. SCALE: 1:250 (A1)
 VERT. SCALE:
 ISSUED FOR REVIEW 11/03/28 R.C.
 NO. REVISIONS DATE BY
 DATE: MARCH 2011

ENGINEER'S SEAL

THE CITY OF WINNIPEG
 PLANNING, PROPERTY & DEVELOPMENT

ELMWOOD/KILDONAN POOL
 PARKING LOT RECONSTRUCTION PROJECT
 GRADING PLAN

CITY DRAWING NO. 151-2011-C03
 SHEET 3 OF 5
 REV. 3 0