APPENDIX 'A'

GEOTECHNICAL REPORT

APPENDIX 'A' - GEOTECHNICAL REPORT

GEOTECHNICAL REPORT FOR:

- i) Day Street from Larche Avenue to Gunn Road
- ii) Harbison Avenue West from Brazier Street to Roch Street
- iii) Leslie Avenue from Glenwood Crescent to Silvia Street
- iv) Martin Avenue West from Glenwood Crescent to Beatrice Street
- v) Stanier Street from Dearborn Avenue to Gordon Avenue

The geotechnical report is provided to aid in the Contractor's evaluation of the existing pavement structure and/or soil conditions. The information presented is considered accurate at the locations shown on the Drawings and at the time of drilling. However, variations in pavement structure and/or soil conditions may exist between test holes and fluctuations in groundwater levels can be expected seasonally and may occur as a result of construction activities. The nature and extent of variations may not become evident until construction commences.



GEOTECHNICAL INVESTIGATION RESIDENTIAL STREETS RECONSTRUCTION WINNIPEG, MANITOBA

Submitted to:

City of Winnipeg Engineering Technology Services 106 – 1155 Pacific Avenue Winnipeg, Manitoba R3E 3P1

Attention: Mr. Brent Kellet

Submitted by:

AMEC Earth & Environmental A Division of AMEC Americas Limited

> 440 Dovercourt Drive Winnipeg, Manitoba R3Y 1N4

> > 12 January 2009

AMEC File No. WX10364



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1.0 INTRODUCTION

AMEC Earth and Environmental, a division of AMEC Americas Limited (AMEC), were retained by the City of Winnipeg (COW) to conduct a geotechnical investigation for a proposed upgrading project for several streets in Winnipeg, Manitoba. The investigation was conducted on sections of five residential Winnipeg streets, as follows:

• Day Street:

•

•

- Larche Ave. W. to Gunn Rd.,
- Harbison Avenue West: Leslie Avenue:
- Brazier St. to Roch St., Glenwood Cres. to Silva St.,
- Glenwood Cres. to Silva St., Glenwood Cres. to Beatrice St., and
- Martin Avenue West Glenwood C
- Stanier Street:
- Dearborn Ave. to Gordon Ave.
- The purpose of the investigation was to determine the pavement condition and soil profile at selected locations along the subject section of each street. The test hole locations were determined by the City of Winnipeg prior to the investigation.

2.0 SITE CONDITIONS

At the streets investigated, the roadway surface consisted of either asphalt or concrete. All asphalt was underlain by concrete pavement. At the time of the investigation, the roadways were snow and ice covered and therefore a detailed review of the pavement condition was not possible. Typical of roads in the Winnipeg area, the roads were generally flat lying and level, with local slopes between catch basins to facilitate drainage.

3.0 FIELD INVESTIGATON

Prior to coring and drilling, AMEC had public underground utilities located. On November 26 to 28, 2008, following utility clearances, all test hole locations were cored prior to auger drilling in order to determine the asphalt and concrete thicknesses. All coring was completed with a 150 mm diameter diamond coring rig. Each of the cores was photographed and photos are presented in Appendix B.

On 1 and 2 December 2008, a total of twenty one (21) test holes were drilled on the streets noted, using a truck mounted Acker MP5T drill rig operated by Paddock Drilling Ltd. of Winnipeg, and equipped with 125 mm continuous flight solid stem augers. The test hole locations are shown on Figures 1 to 5.

Test hole logging and subcontractor supervision was provided on a full time basis by Mr. Blair Power, CET of AMEC. Traffic control during the coring and drilling operation was provided by Guardian Traffic Services.

All soils observed during test hole drilling were visually classified on site according to the Modified Unified Soil Classification System and in accordance with the City of Winnipeg geotechnical guidelines. Groundwater and drilling conditions, as well as any pertinent



subsurface observations, were also recorded at the time of the investigation.

Disturbed soil samples were taken at regular intervals from the auger flights in each test hole. All soil samples obtained during the field investigation were labelled, sealed in plastic bags to limit moisture loss and transported to AMEC's Soils Laboratory in Winnipeg for further examination and testing. The test hole logs are presented in Appendix A, Figures 6 to 26, and show the soil profile, results of the field and laboratory testing, and comments relative to groundwater and sloughing conditions encountered.

Each test hole was backfilled with the auger cuttings and topped with asphalt cold patch at the completion of drilling, with excess cuttings removed from site.

4.0 LABORATORY TESTING

Soil samples were returned to AMEC's Soils Laboratory in Winnipeg for geotechnical laboratory testing. The soil samples were visually classified and tested for in-situ moisture contents, and selected samples were tested for Atterberg limits and Hydrometer analysis to confirm the field classification of soils.

5.0 SUBSURFACE CONDITIONS

The generalized stratigraphy, as noted in the test holes for each investigated street is summarized in the following sections. It should be noted that the gravel sizes noted are inferred, since drilling in freezing conditions tended to break the gravel into finer particles.

5.1 Day Street – Test Holes D1 to D4

Asphalt pavement was present at the surface at test hole D3 only and was 67 mm thick. Below the asphalt, and at the surface at the remaining test holes, concrete pavement was observed and varied from 137 to 178 mm thick.

Granular fill was present below the concrete pavement at every test hole (D1 to D4) and was about 130 to 280 mm thick. The gravel consisted of 20 mm granular fill and was frozen, sandy, poorly graded and moist.

A 305 mm thick layer of clay fill was present below the gravel fill in test holes D1 and D2 and was generally silty, medium to high plastic, moist, stiff to very stiff and contained trace amounts of sand.

Native medium to high plastic clay was encountered below the clay fill or gravel fill at each test hole and extended to the depths explored at test holes D1 and D2. The clay was silty, moist and stiff to very stiff. At test holes D3 and D4 a silt layer was present within the high plastic clay layer, and varied from 213 to 457 mm thick, with high plastic clay present below the silt and extending to the depth explored (2.1 m). The silt was clayey, low to medium plastic, moist and firm to stiff.



5.2 Harbison Avenue W. – Test Holes H1 to H7

Asphalt pavement was present at the ground surface at every test hole (H1 to H7) and was 19 to 48 mm thick. Concrete pavement was encountered below the asphalt and varied from 140 to 206 mm thick.

A clay fill layer, 122 to 719 mm thick (averaging about 400 mm thick), was encountered below the concrete in each test hole and was generally silty, medium to high plastic, moist and stiff to very stiff, and contained trace to some sand

At every test hole, high plastic clay was present underlying the clay fill and was silty, moist and stiff to very stiff,. The clay extended to the depths explored in test holes H1, H6 and H7.

Silt was noted below the clay to the depths explored (1.8 m) at test holes H2 to H5 and was low plastic, moist to very moist and firm to stiff, with trace to some clay.

5.3 Leslie Avenue – Test Holes L1 to L4

Concrete pavement was present at the ground surface at each test hole (L1 to L4) and was 137 to 159 mm thick. At test hole L3, the concrete was underlain by 320 mm of clay fill that was silty, medium plastic, moist, stiff and contained trace sand. At the remaining test holes and below the clay fill, medium to high plastic clay was encountered below the concrete or clay fill, extended to the depths explored and was silty, moist and stiff to very stiff.

5.4 Martin Avenue W. – Test Holes M1 to M3

Asphalt pavement, 13 to 25 mm thick, underlain by concrete pavement that was 130 to 159 mm thick, was present at each test hole (M1 to M3). Below the concrete, medium to high plastic clay was present throughout the remaining depths and was silty, moist and stiff to very stiff.

5.5 Stanier Avenue – Test Holes S1 to S3

Asphalt pavement was present at each test hole (S1 to S3) and was about 41 to 52 mm thick. The asphalt was underlain by concrete that was about 170 to 210 mm thick.

The concrete pavement was underlain by high plastic clay that was generally silty, moist and stiff to very stiff. The clay extended throughout the test hole depth at S2 At test holes S1 and S3 the clay was underlain by silt that extended to the depths explored (1.8 to 2.1 m). The silt was low to non-plastic, moist to very moist and soft to stiff, with trace to some clay.

The odd-numbered tables below, summarize the thickness and types of pavements and soils encountered at each of the test hole locations. Even numbered tables summarize selected lab test data. Detailed soil stratigraphy is illustrated in the attached test hole logs in Appendix A.



Test Hole No.	D1	D2	D3	D4
Asphalt	N/A	N/A	67	N/A
Concrete	178	177	137	171
Fill – Gravel	280	280	131	165
Fill – Clay	305	305	N/A	N/A
High Plastic Clay	1372	N/A	1189	823
Medium Plastic Clay	N/A	762	N/A	N/A
Silt	N/A	N/A	457	213
Clay	N/A	610	152	762

Table 1: Day Street Pavement and Soil Thickness (mm)

Table 2: Day Street Lab Test Results

Test	Sample	Moisture		Atte	Atterberg Limits			Hydrometer Analysis			
Test Hole No.	Depth (m)	Content (%)	Liquid Limit	Plastic Limit	Plasticity Index	MUSCS Classification	Gravel (%)	Sand (%)	Silt (%)	Clay (%)	
D1	0.9	36	76	21	55	CH (Clay)	0	5	29	66	
D3	1.8	27	37	15	22	CI (Clayey Silt)	0	2	69	29	

Table 3: Harbison Avenue W. Pavement and Soil Thickness (mm)

Test Hole No.	H1	H2	H3	H4	H5	H6	H7
Asphalt	36	19	29	24	29	30	48
Concrete	150	165	165	140	206	156	165
Fill – Clay	271	122	719	597	527	271	244
High Plastic Clay	1372	914	457	457	914	1372	1372
Silt	N/A	610	457	610	152	N/A	N/A

Table 4: Harbison Avenue Lab Test Results

Teet	Sample	Moisture	Atterberg Limits				Hydrometer Analysis			
Test Hole No.	Depth (m)	Content (%)	Liquid Limit	Plastic Limit	Plasticity Index	MUSCS Classification	Gravel (%)	Sand (%)	Silt (%)	Clay (%)
H2	1.5	23	28	25	3	ML (Silt)	0	4	72	24
H7	0.9	41	90	23	67	CH (Clay)	0	2	22	76



Table 5: Leslie Avenue Pavement and Soil Thickness (mm)

Test Hole No.	L1	L2	L3	L4
Concrete	159	137	137	140
Fill – Clay	N/A	N/A	320	N/A
Medium Plastic Clay	1670	1692	1372	1689

Table 6: Leslie Avenue Lab Test Results

Teet	Sample	Moisture	Atterberg Limits				Hydrometer Analysis			
Test Hole No.	Depth (m)	Content (%)	Liquid Limit	Plastic Limit	Plasticity Index	MUSCS Classification	Gravel (%)	Sand (%)	Silt (%)	Clay (%)
L1	0.6	26	48	22	26	CI (Clay)	0	18	47	35

Table 7: Martin Avenue W. Pavement and Soil Thickness (mm)

Test Hole No.	M1	M2	M3
Asphalt	13	25	25
Concrete	159	130	140
High Plastic Clay	1658	1676	1676

Table 8: Martin Avenue Lab Test Results

Test	Sample	Moisture	Atterberg Limits				Hydrometer Analysis			
Hole No.	Depth (m)	Content (%)	Liquid Limit	Plastic Limit	Plasticity Index	MUSCS Classification	Gravel (%)	Sand (%)	Silt (%)	Clay (%)
M1	0.9	34	72	22	50	CH (Clay)	0	4	45	51



Table 9: Stanier Street Pavement and Soil Thickness (mm)

Test Hole No.	S1	S2	S3
Asphalt	41	58	52
Concrete	187	210	168
High Plastic Clay	1143	1865	1000
Silt	762	N/A	610

Table 10: Stanier Street Lab Test Results

Test Hole No.	Sample	Moisture	Atterberg Limits				Hydrometer Analysis				
	Depth (m)	Content (%)	Liquid Limit	Plastic Limit	Plasticity Index	MUSCS Classification	Gravel (%)	Sand (%)	Silt (%)	Clay (%)	
S1	0.6	38	75	25	50	CH (Clay)	0	4	35	61	
S3	1.5	22			Non- Plastic	ML	0	3	84	13	

6.0 CLOSURE

The findings of this report were based on the results of field and laboratory investigations at test hole locations as selected by the City of Winnipeg.

The site investigation was conducted for the sole purpose of profiling the pavement and subsurface conditions. Although no environmental issues were identified during the fieldwork, this does not indicate that no such issues exist. If the owner or other parties have any concern regarding the presence of environmental issues, then an appropriate level environmental assessment should be conducted.

Soil conditions, by their nature, can be highly variable across a site. The placement of fill and prior construction activities on a site can contribute to the variability especially near surface soil conditions. A contingency should always be included in any construction budget to allow for the possibility of variation in soil conditions, which may result in modification of any potential design and construction procedures which may arise from this factual investigative report.



This report was prepared exclusively for the City of Winnipeg, and their agents for the proposed development as described in the report. The data provided herein are presented in a factual manner only with no engineering interpretation provided, and should not be used for any other purpose, or by any other parties, without review and advice from a qualified geotechnical engineer. No other warranty, expressed or implied, is given.

Yours truly,

AMEC Earth & Environmental

Robert Brown , P. Eng. Geotechnical Engineer

Reviewed By:

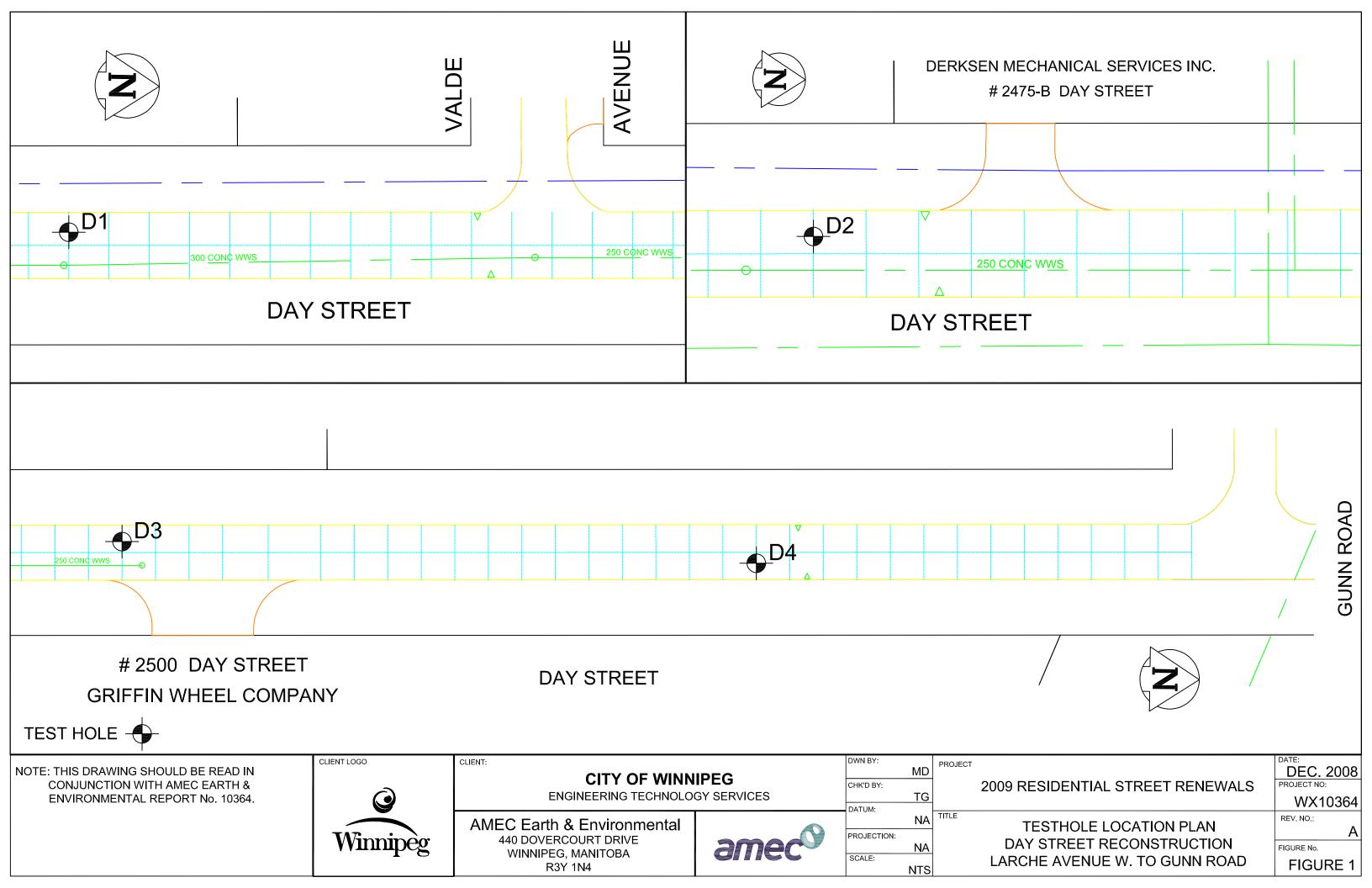
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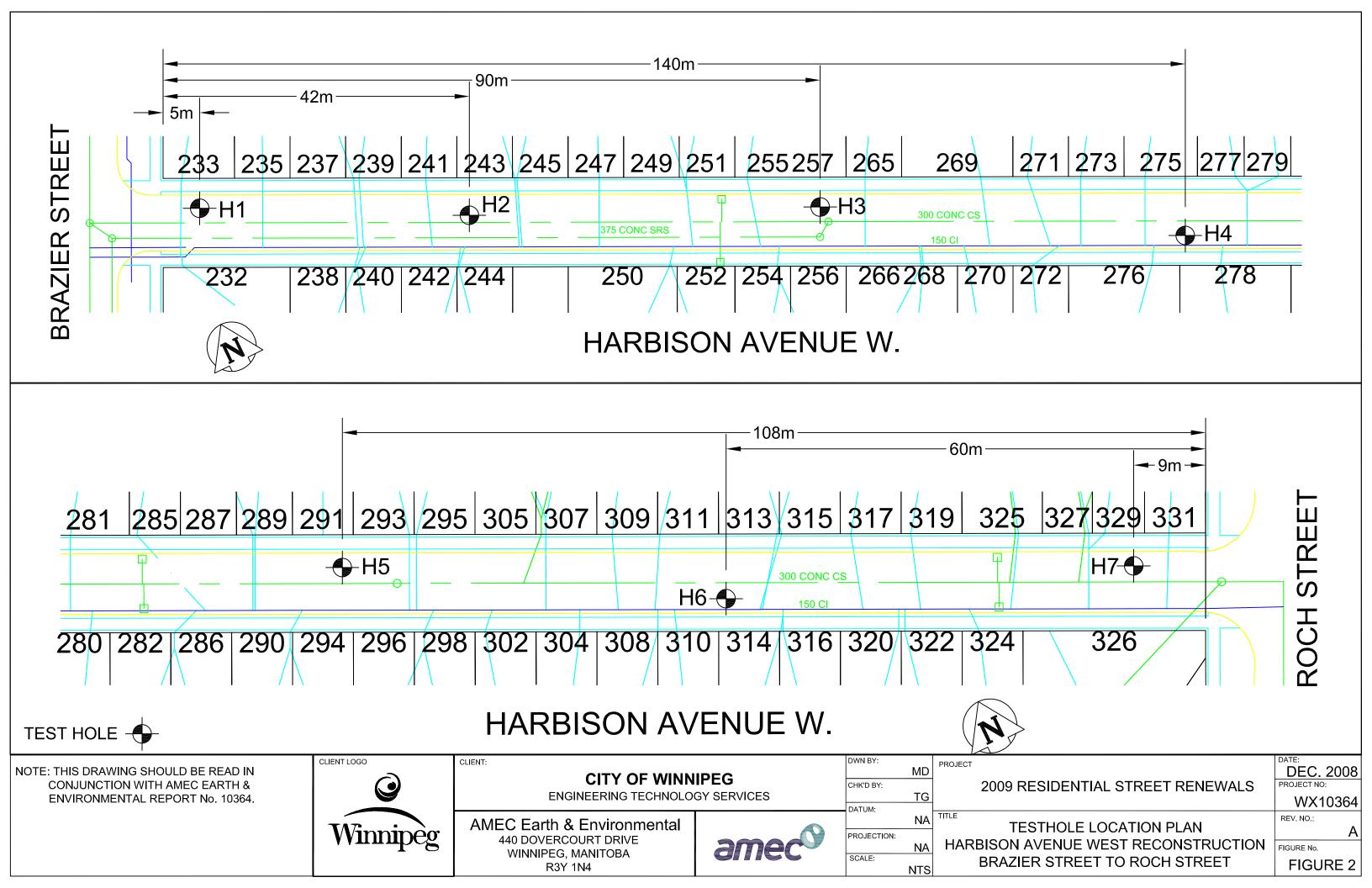
Trevor Gluck, P. Eng. Staff Geotechnical Engineer

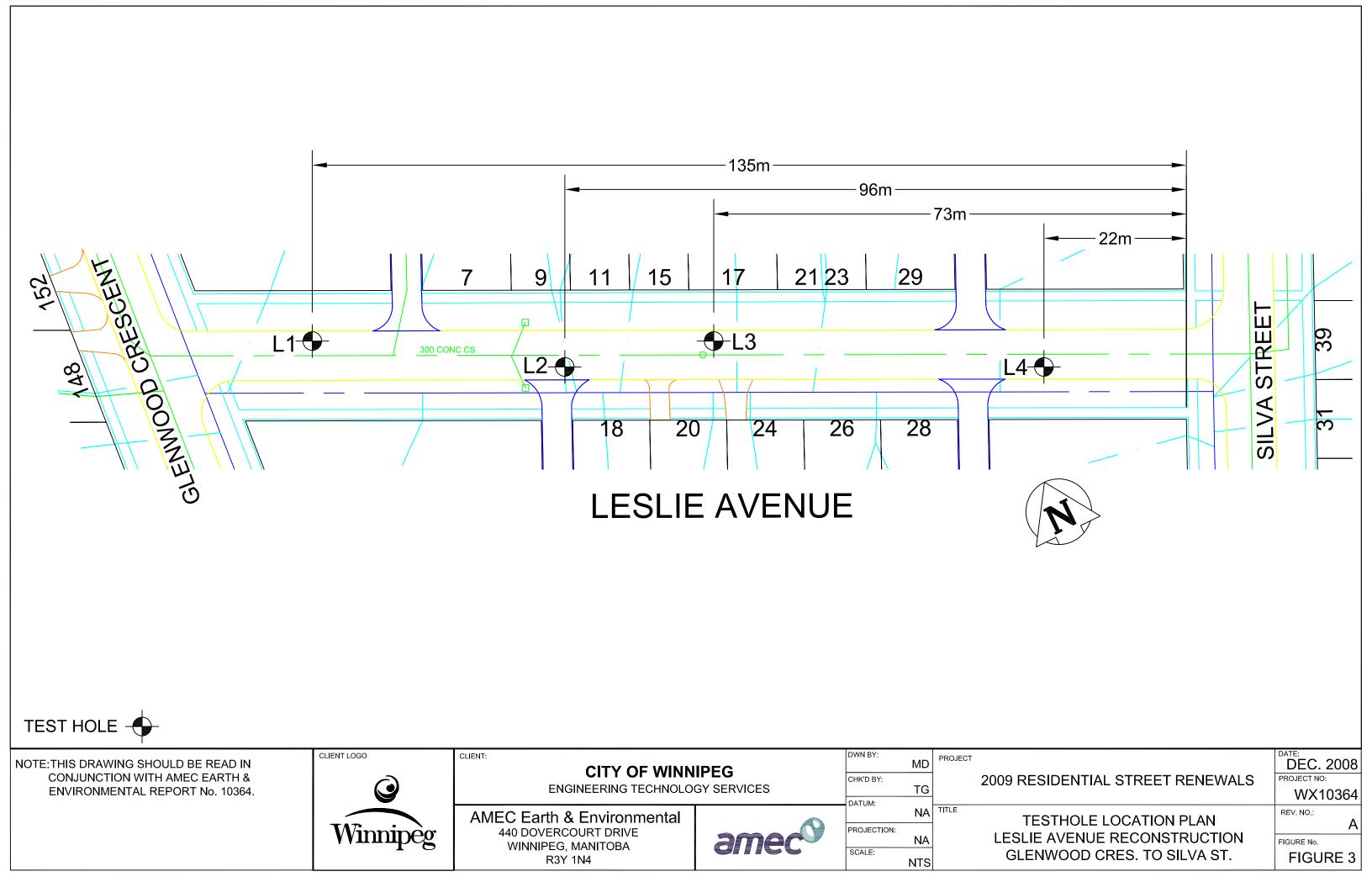


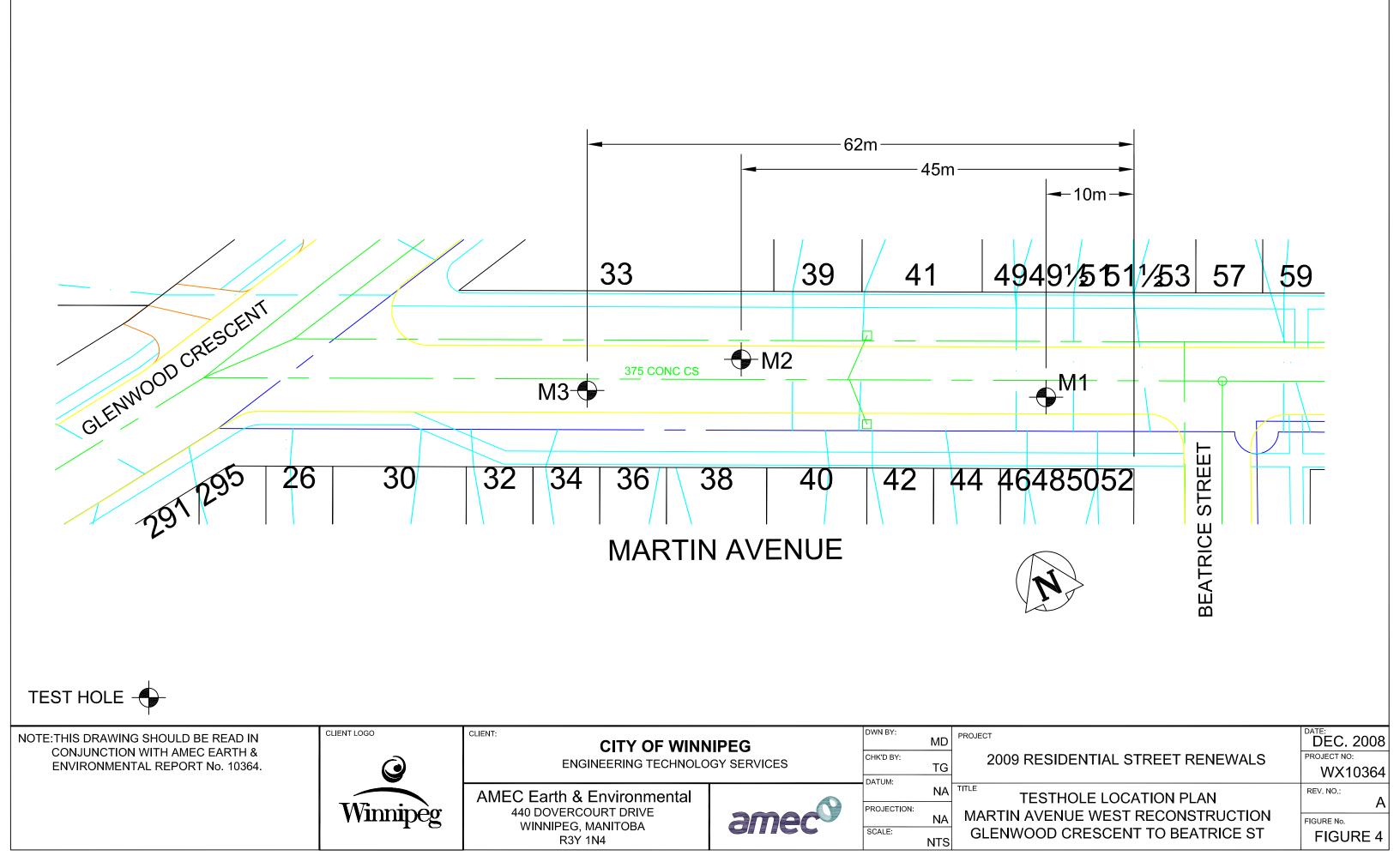
FIGURES 1 to 5

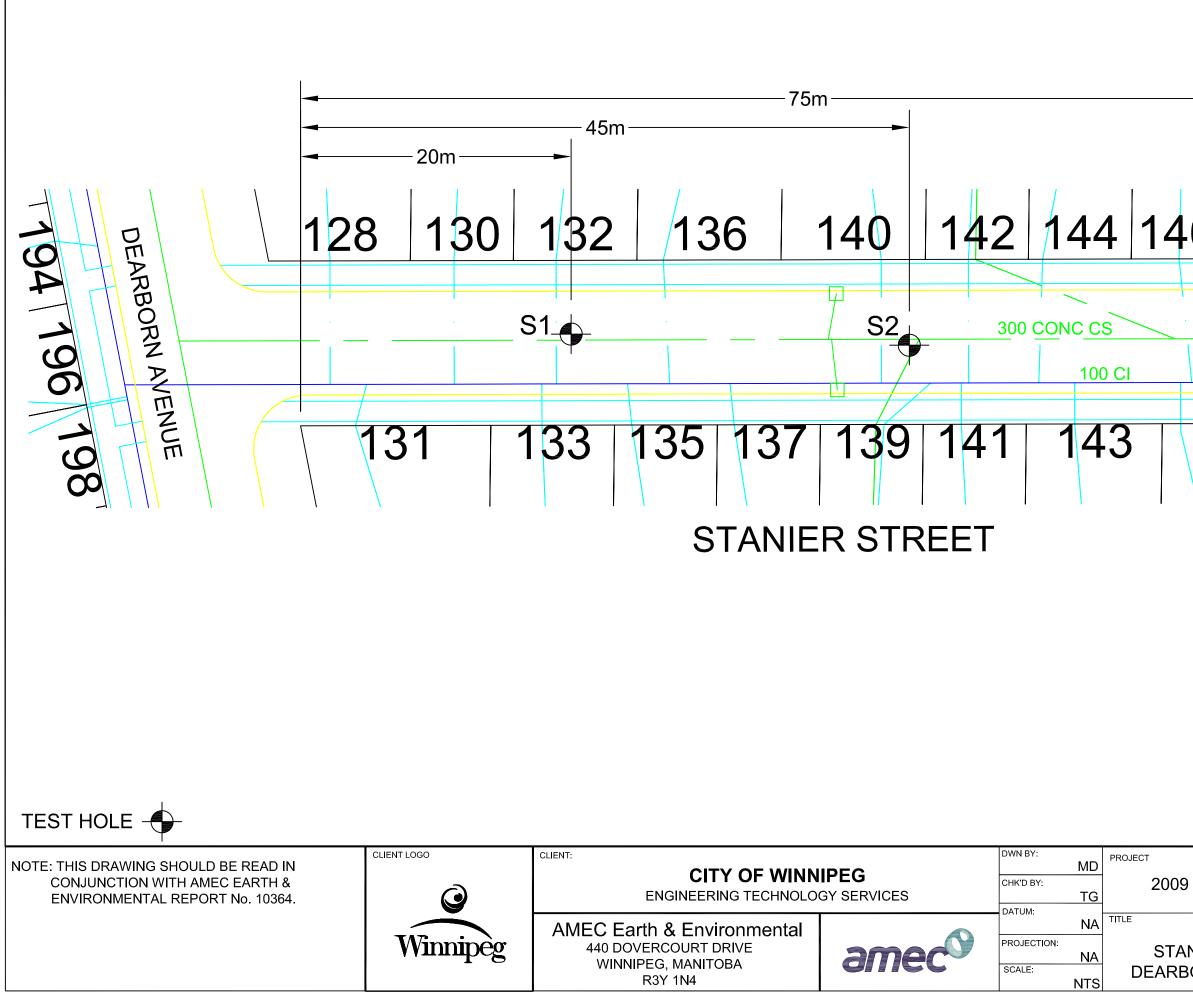
TEST HOLE LOCATION PLANS









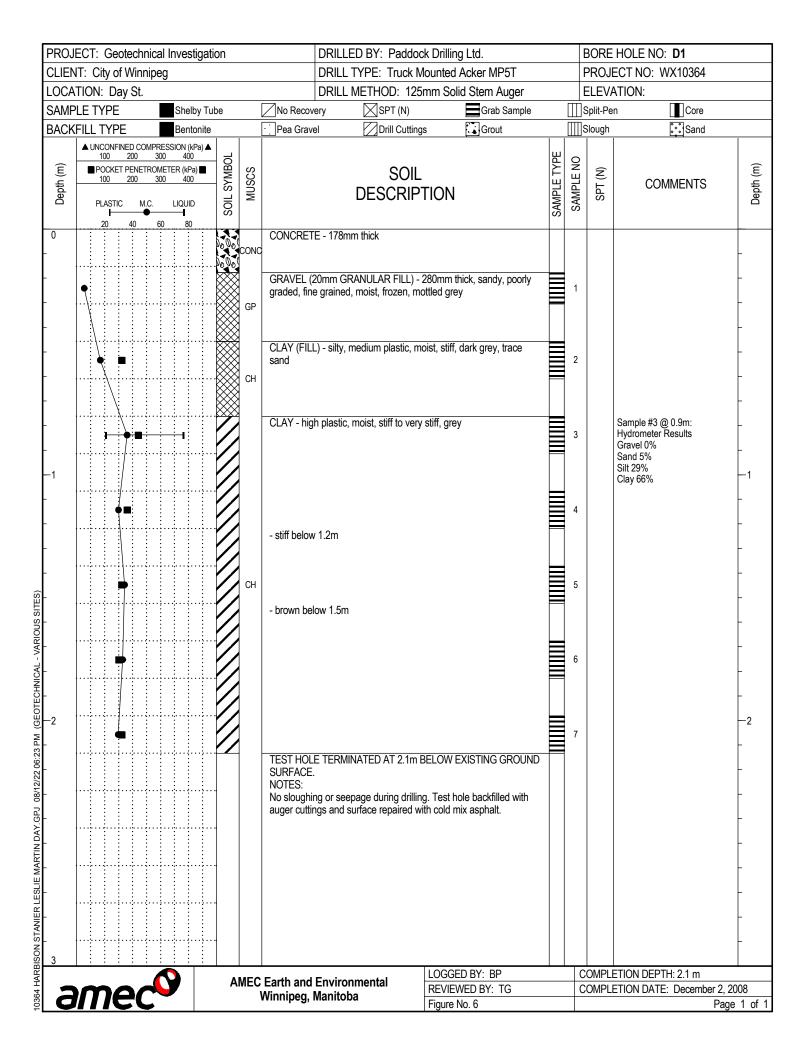


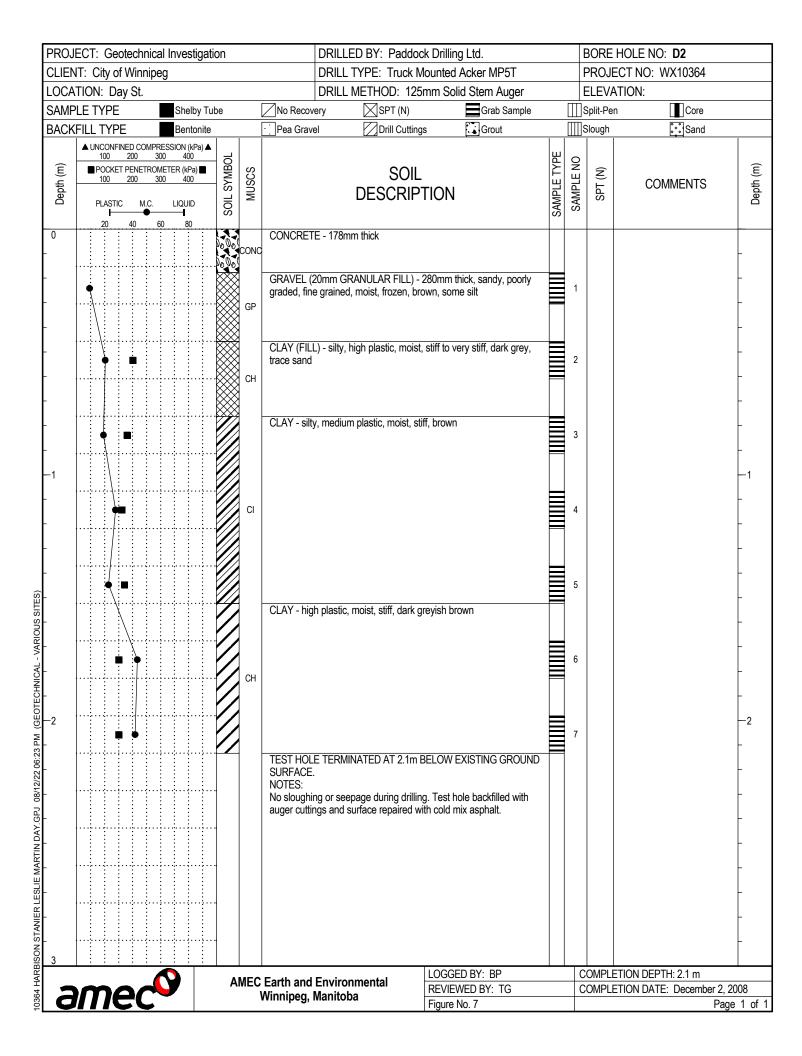
6 148 150 6 148 150 - - - 53 - - 145 147 - - - - - - - - - - - - - - - - - - - - - - - - -	GORDON AVENUE
RESIDENTIAL STREET RENEWALS	DATE: DEC. 2008 PROJECT NO: WX10364
TESTHOLE LOCATION PLAN NER STREET RECONSTRUCTION ORN AVENUE TO GORDON AVENUE	REV. NO.: FIGURE No. FIGURE 5

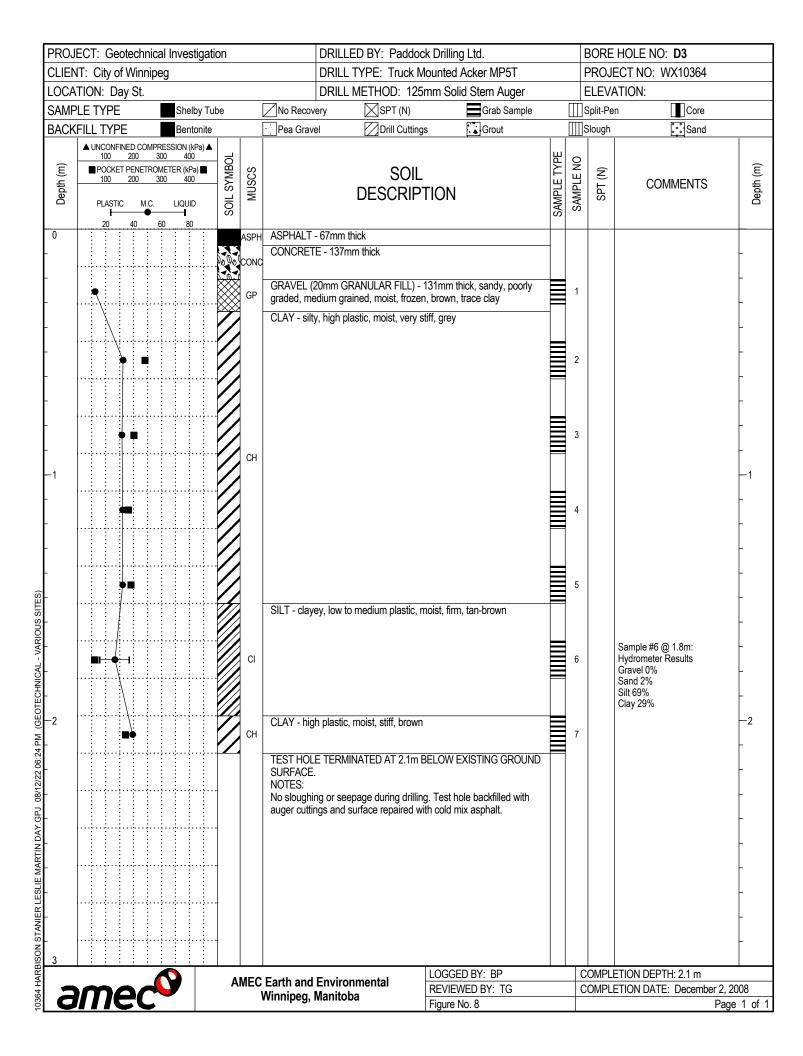


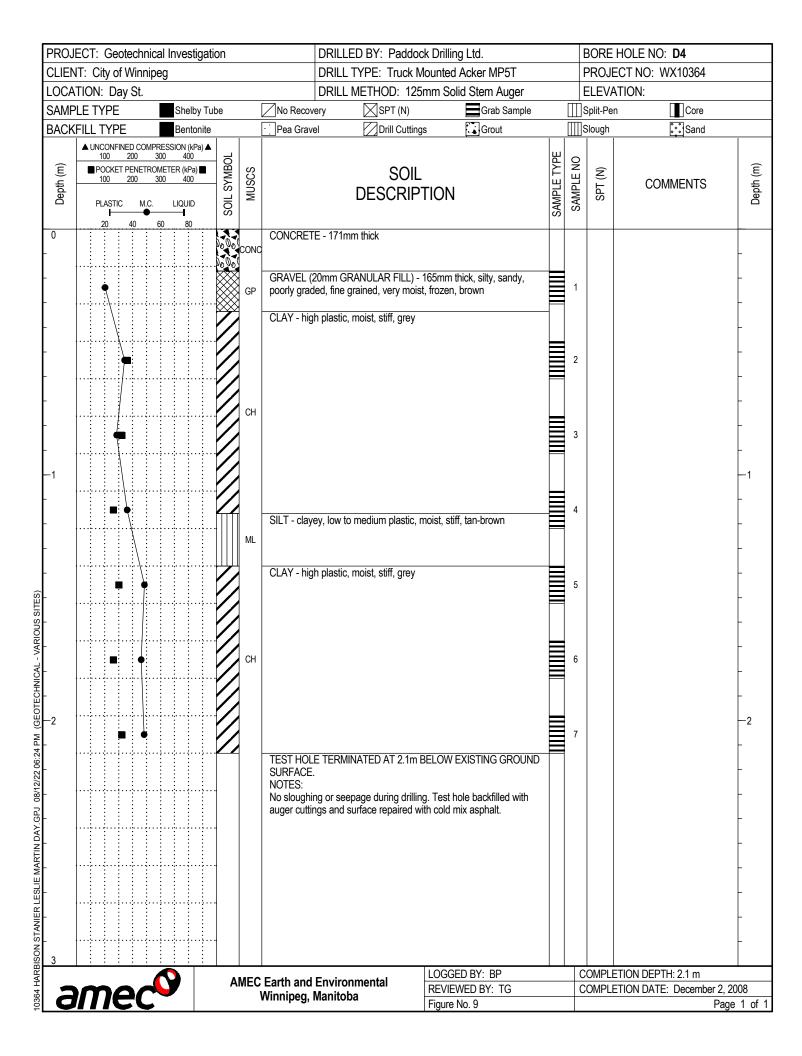
APPENDIX A

TEST HOLE LOGS

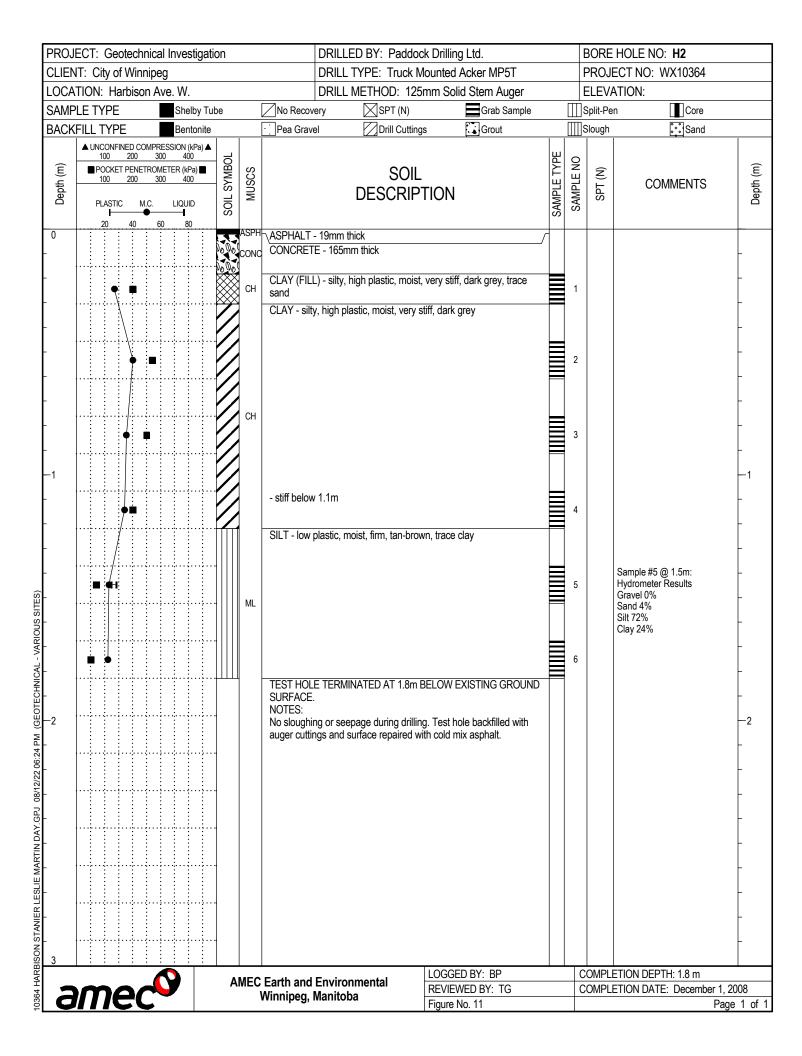




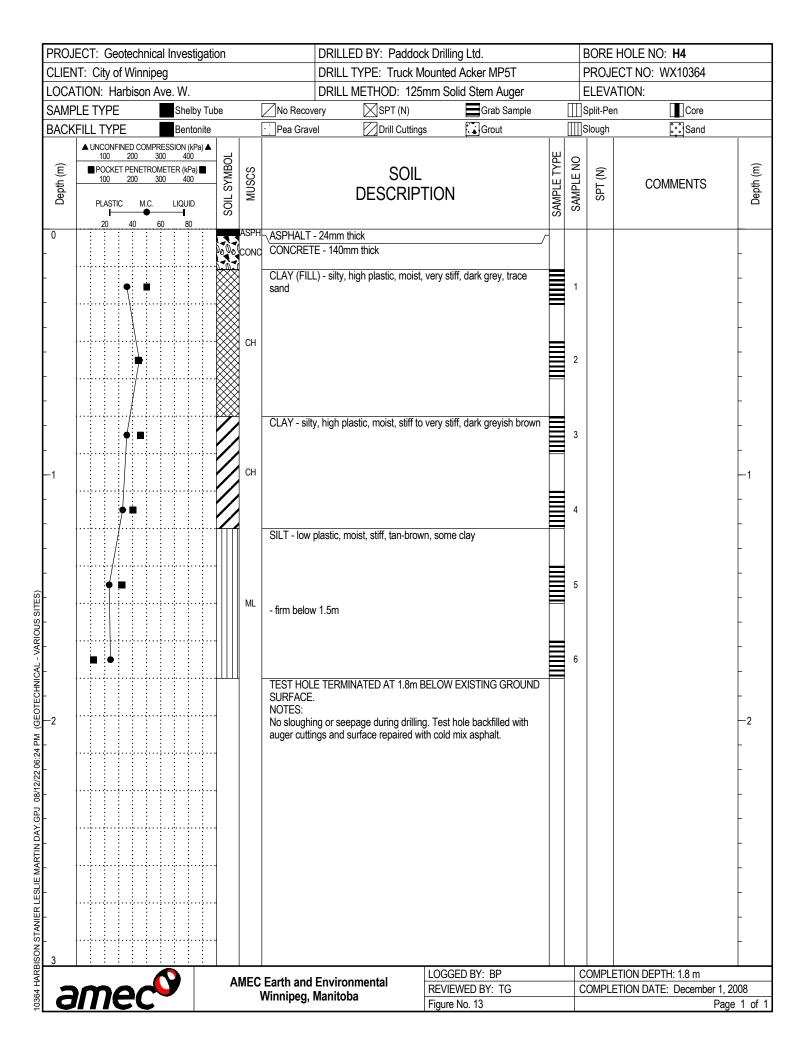


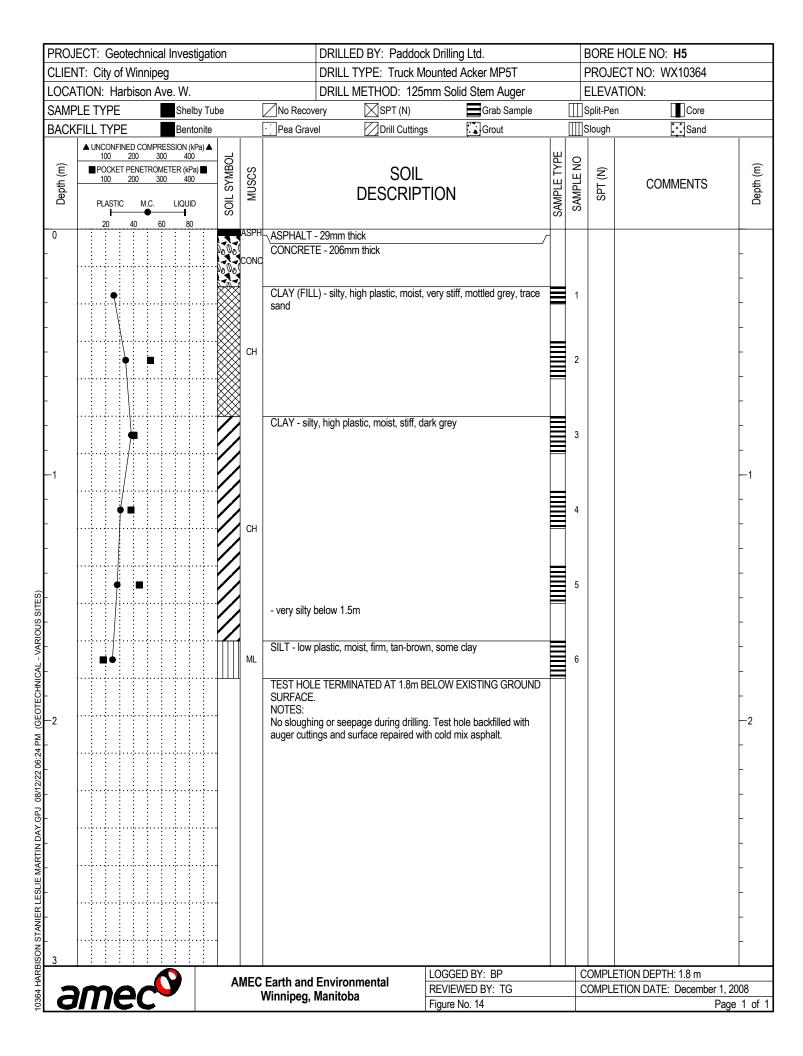


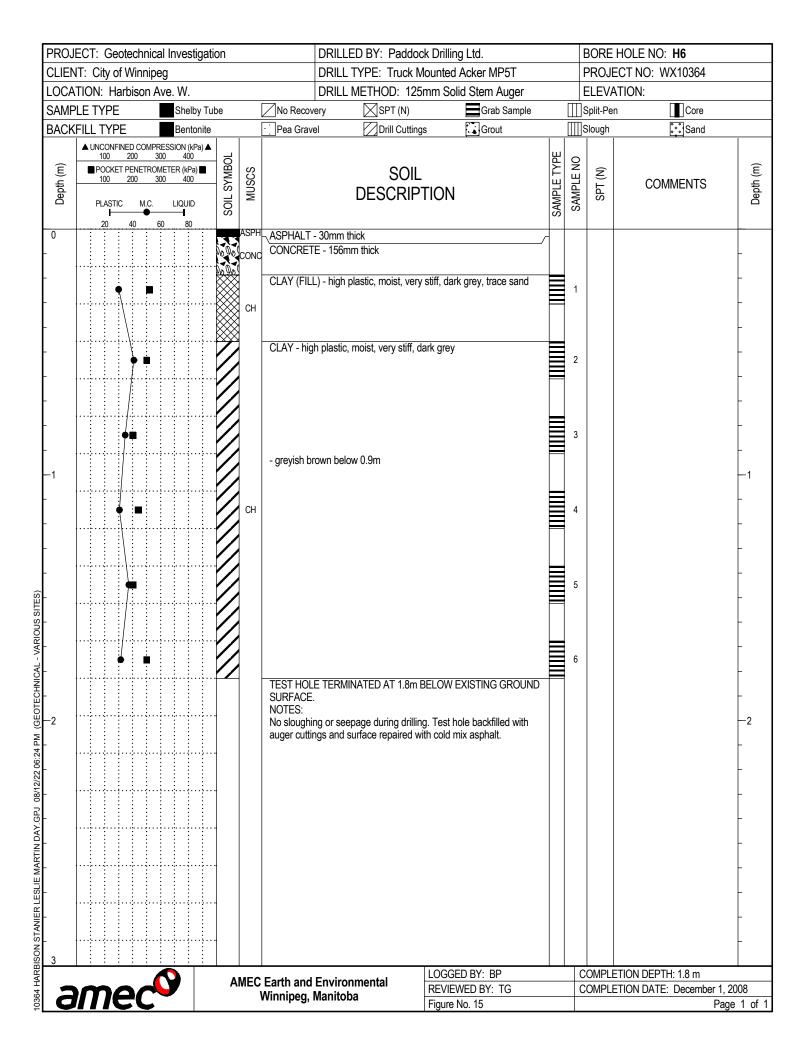
PROJ	ECT: Geotechnical Inves	tigation	DRILLI	ED BY: Paddock Drill	ing Ltd.		BORE	Hole No: H1			
CLIEN	IT: City of Winnipeg		DRILL	TYPE: Truck Mounte	d Acker MP5T		PROJ	PROJECT NO: WX10364			
LOCA	TION: Harbison Ave. W.		DRILL	METHOD: 125mm S	olid Stem Auger		ELEV	ATION:			
SAMP	LE TYPE Shel	by Tube	No Recovery	SPT (N)	Grab Sample		Split-Pe				
BACK	FILL TYPE Bent	onite	Pea Gravel	Drill Cuttings	Grout		Slough	ै ् ै Sand			
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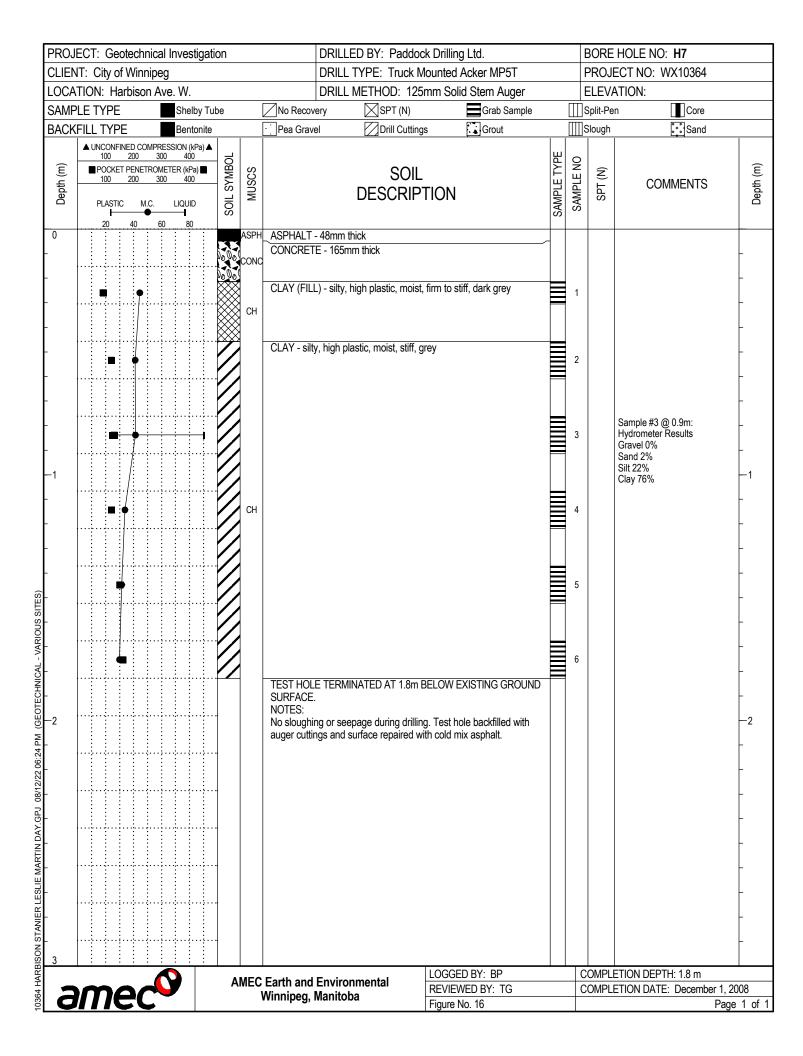


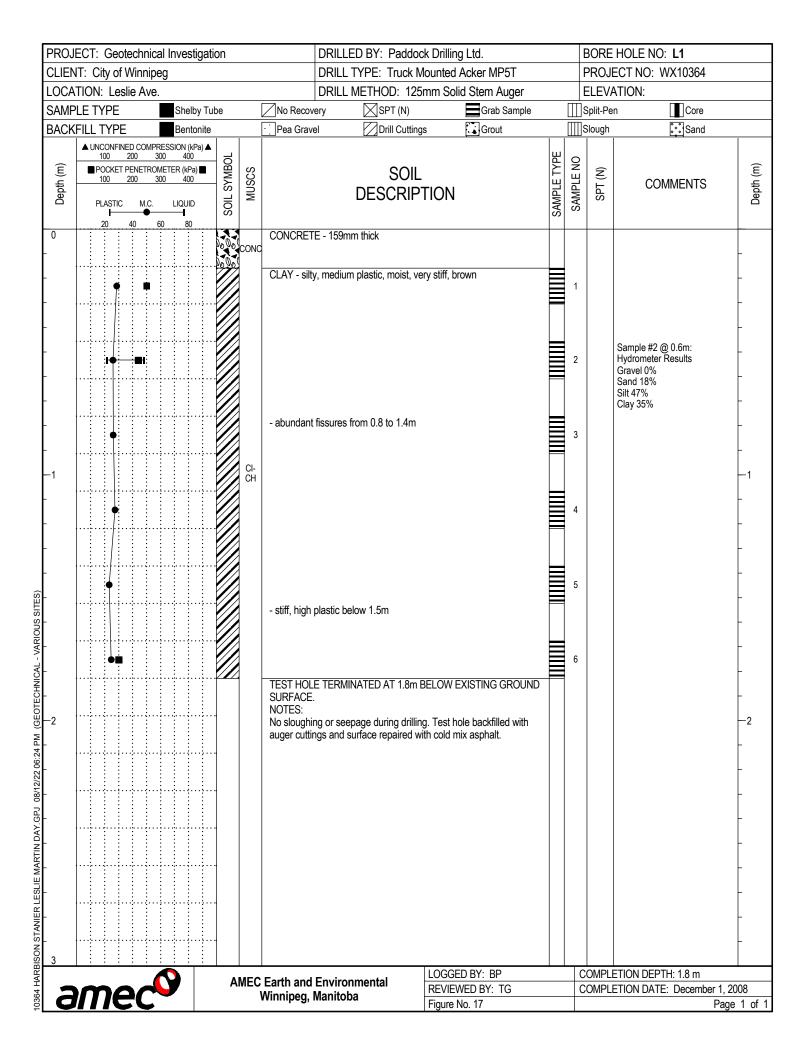
PROJ	ECT: Geotechnical Inves	tigation	DRILLED BY: Pa	addock Drilling Ltd.		BORE	HOLE NO: H3	
CLIEN	NT: City of Winnipeg		DRILL TYPE: Tru	uck Mounted Acker MP5T		PROJ	ECT NO: WX10364	
LOCA	TION: Harbison Ave. W.		DRILL METHOD:	125mm Solid Stem Auger		ELEVA	ATION:	
SAMF	PLE TYPE Shell	lby Tube	No Recovery SPT (Split-Pe		
BACK	FILL TYPE Bent	tonite	Pea Gravel Drill C	Cuttings 🔄 Grout		Slough	Sand [
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AKB				LOGGED BY: BP		COMPL	ETION DEPTH: 1.8 m	
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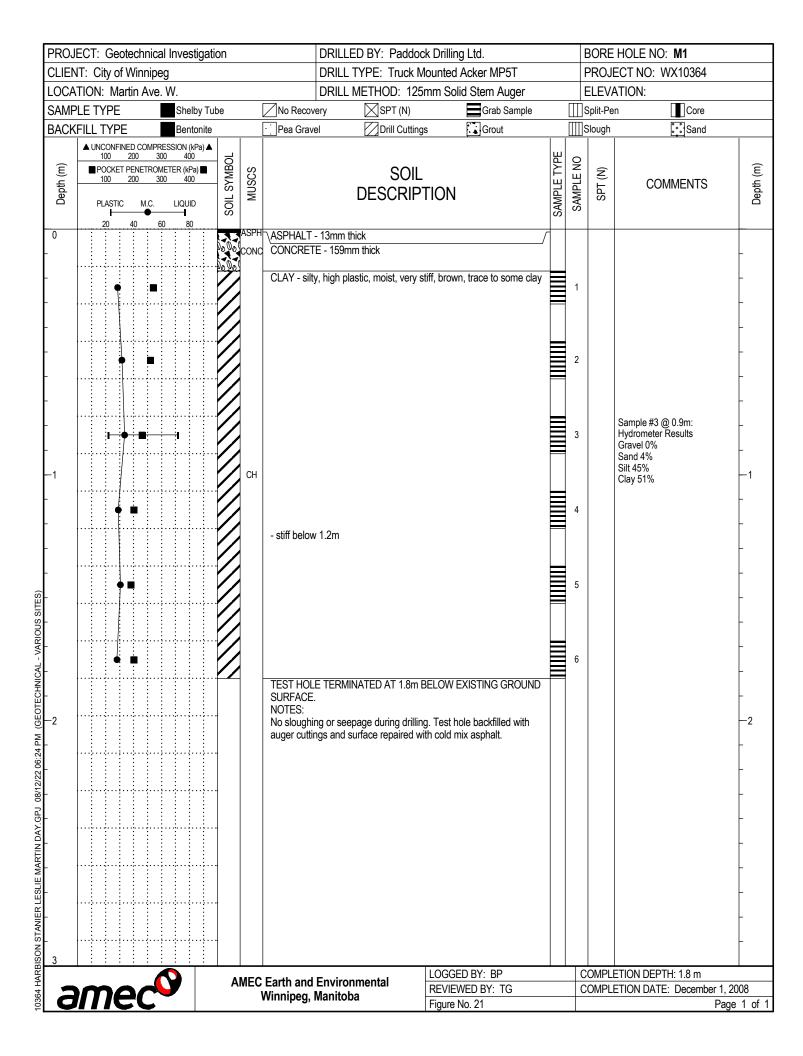




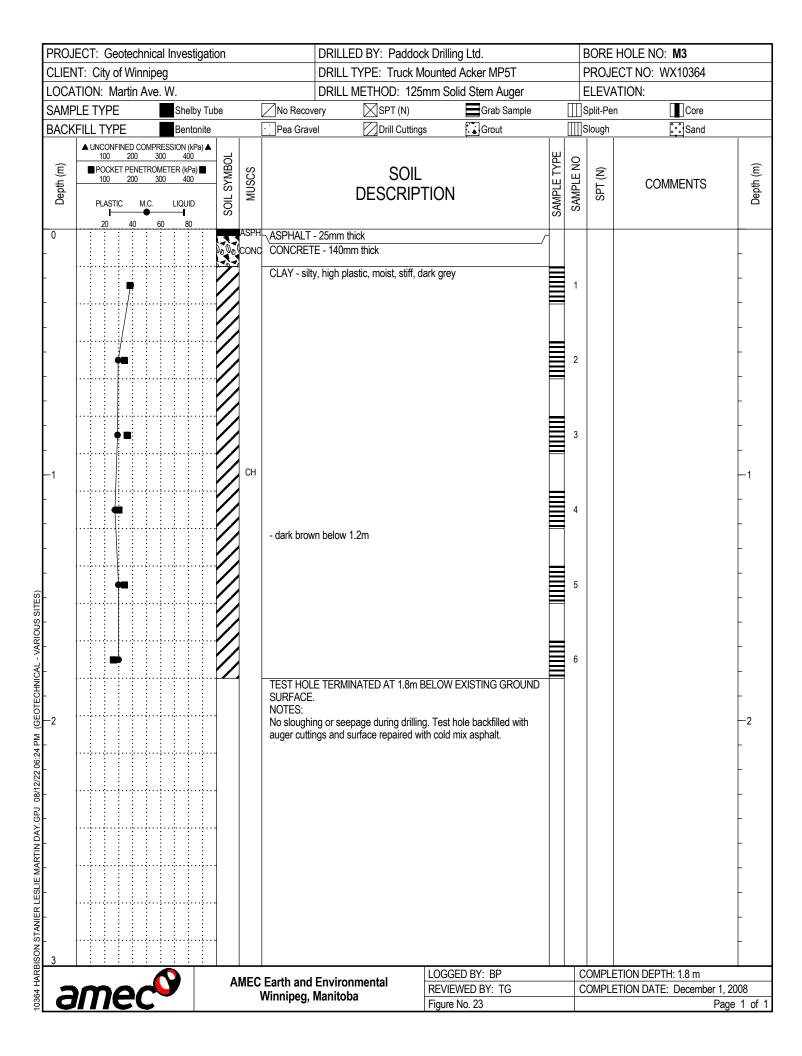
PROJ	ECT: Geotechnical Invest	stigation	DRILLE	D BY: Paddock D	rilling Ltd.		BOR	e hole no: L2	
CLIEN	IT: City of Winnipeg		DRILL ⁻	TYPE: Truck Moun	ted Acker MP5T		PRO	JECT NO: WX10364	
LOCA	TION: Leslie Ave.		DRILL	METHOD: 125mm	Solid Stem Auger		ELE	VATION:	
SAMF	PLE TYPE She	lby Tube	No Recovery	SPT (N)	Grab Sample		Split-F	Pen Core	
BACK	FILL TYPE Ben	tonite	Pea Gravel	Drill Cuttings	Grout		Sloug	h 👫 Sand	
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-							2		-
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SITES)	.		- brown below 1.2m				5		-
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.GPJ 08/12/22 06									-
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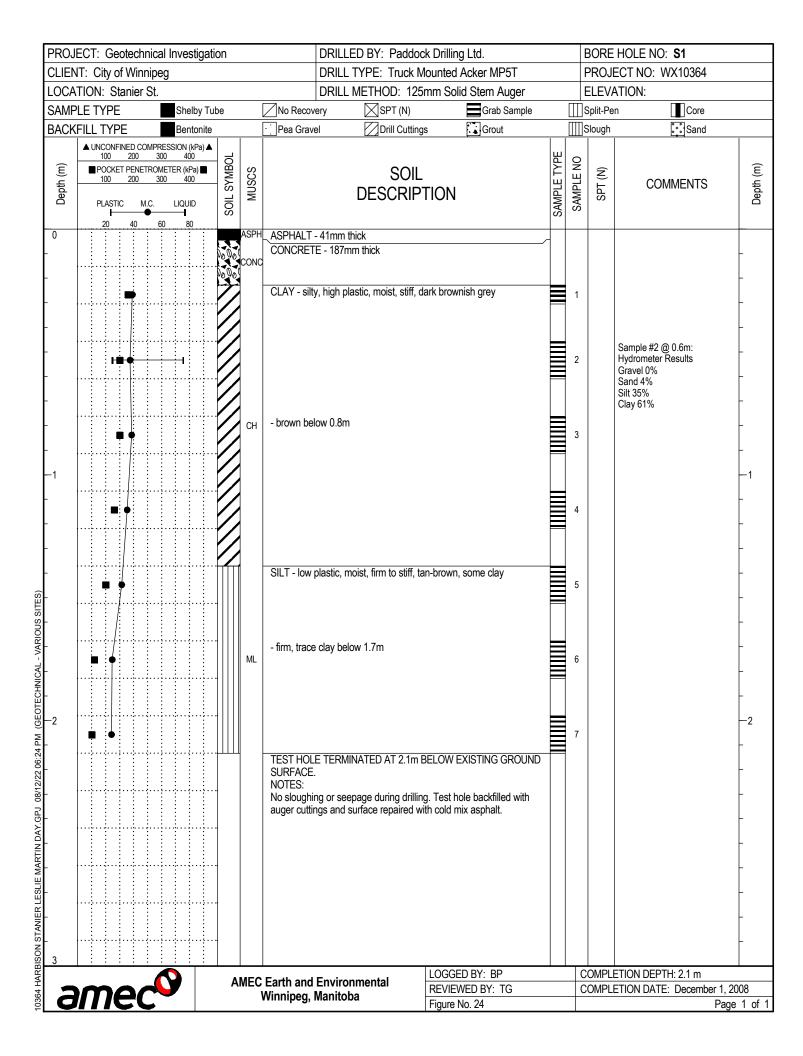
	PROJ	ECT: Geotechnical Invest	tigation						BORE HOLE NO: L3			
	CLIEN	T: City of Winnipeg			DRILL TYPE: Truck	Nounted Acker MP5T		PRO	JECT NO: WX10364			
	LOCA	TION: Leslie Ave.			DRILL METHOD: 12	5mm Solid Stem Auge	er	ELE/	/ATION:			
	SAMP	LE TYPE Shell	by Tube	No Recov	· – · · ·	Grab San		Split-P				
	BACK	FILL TYPE Bente	onite	Pea Grav	el 🛛 Drill Cuttin	gs Grout		Slough	n 🚉 Sand			
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	0			CONCRET	E - 137mm thick				-			
		•	cı	sand	L) - silty, medium plastic, r		, trace	1	-			
				CLAY - silt	y, high plastic, moist, stiff,	dark grey		2	-			
	- 1 -							3	1			
			ά č		vn below 1.2m			5	-			
- VARIOUS SITES)	-							6	-			
A (GEOTECHNICAL	- - 2			SURFACE NOTES: No sloughi	E TERMINATED AT 1.8m ng or seepage during drilli ngs and surface repaired v	ng. Test hole backfilled w			2			
J 08/12/22 06:24 PM	-					-						
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0364 HARBISON STANIER LESLIE MARTIN DAY.GPJ 08/12/22 06:24 PM (GEOTECHNICAL - VARIOUS SITES)												
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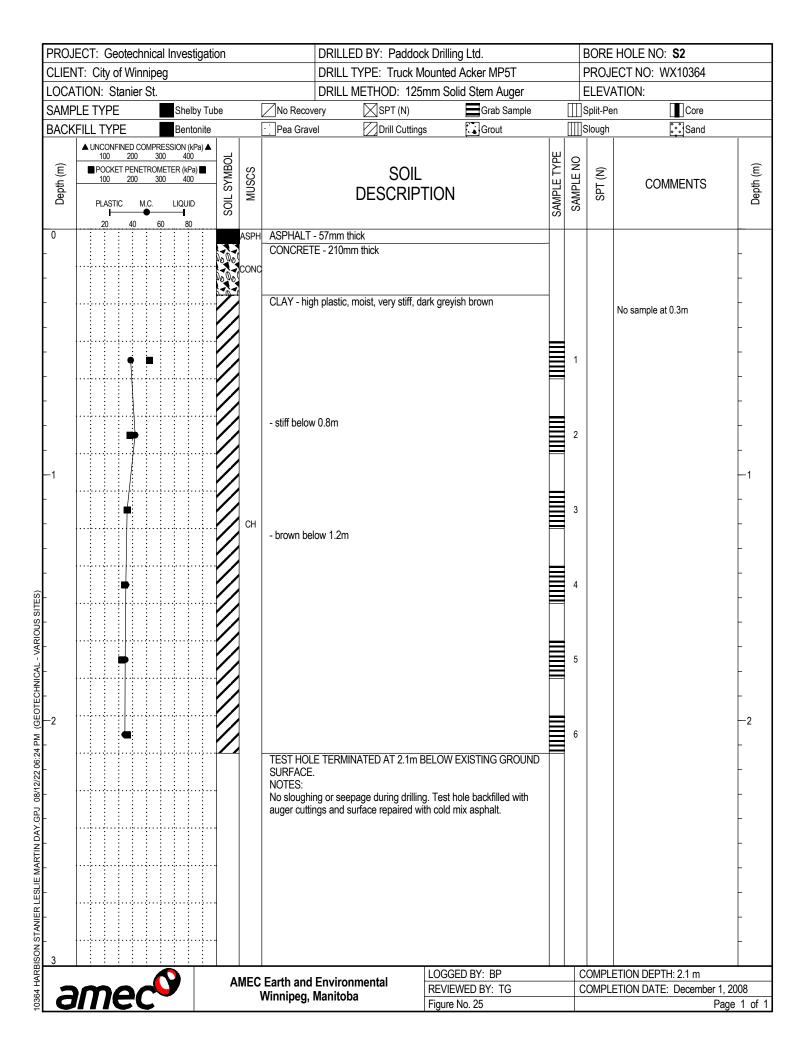
ſ	PROJI	ECT: Geotechnical Invest	tigation			DRILLE	D BY: Paddo	ock Drilling	g Ltd.		E	BORE	HOLE NO: L4	
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	LOCA	TION: Leslie Ave.				DRILL		5mm Soli	d Stem Auger		E	ELEV	ATION:	
	SAMP	LE TYPE Shell	by Tube		No Recov	ery	SPT (N)		Grab Sample		_	Split-Pe		
	BACK	FILL TYPE Bent	onite		Pea Grave	el	Drill Cuttin	gs	Grout		<u> </u>	Slough	Sand	
	Depth (m)	▲ UNCONFINED COMPRESSION (k 100 200 300 400 ■ POCKET PENETROMETER (kPp 100 200 300 400 PLASTIC M.C. LIQUIE 20 40 60 80		MUSCS			SOII DESCRIF			SAMPLE TYPE	SAMPLE NO	SPT (N)	COMMENTS	Depth (m)
	0		200	солс	CONCRET	E - 140m	m thick							-
10364 HARBISON STANIER LESLIE MARTIN DAY.GPJ 08/12/22 06:24 PM (GEOTECHNICAL - VARIOUS SITES)	-1			CI-CH	- trace sand TEST HOLI SURFACE. NOTES: No sloughir	t below 1 E TERMII	NATED AT 1.8m	BELOW E	XISTING GROUND le backfilled with ix asphalt.		1 2 3 4 5 6			
ON STANIE														- -
RBIS	3							IOGGE	D BY: BP			OMPL	ETION DEPTH: 1.8 m	
4 HA	-		A		Earth and				VED BY: TG				ETION DATE: December 1, 20	008
1036	AMEC Earth and Winnipeg,					vianitob	a	Figure N						1 of 1

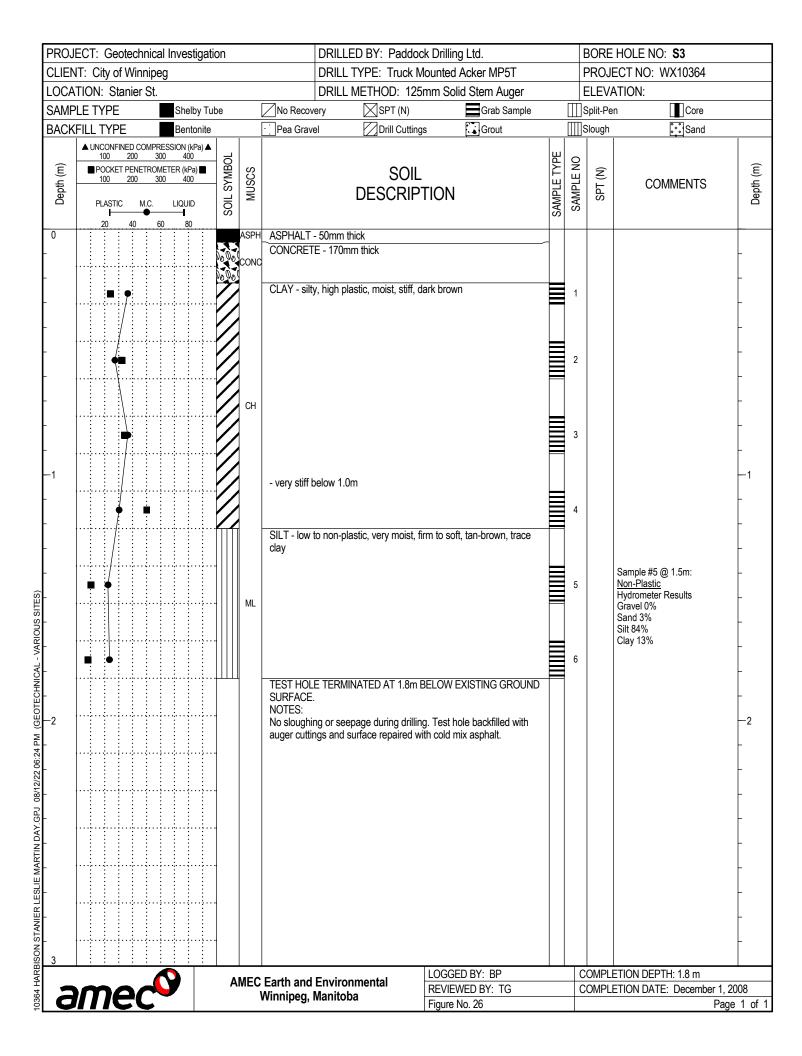


PROJ	ECT: Geotechnical Inves	stigation	DRILLED BY: Paddock	Drilling Ltd.		BORE	Hole no: m2	
CLIEN	NT: City of Winnipeg		DRILL TYPE: Truck Mo	unted Acker MP5T		PROJE	ECT NO: WX10364	
LOCA	TION: Martin Ave. W.		DRILL METHOD: 125m	m Solid Stem Auger		ELEVA	TION:	
SAMF	PLE TYPE She	lby Tube	overy SPT (N)	Grab Sample		Split-Per		
BACK	FILL TYPE Ben	tonite 📄 Pea Gra	vel Drill Cuttings	Grout		Slough	ैँ•ैं]Sand	
Depth (m)	▲ UNCONFINED COMPRESSION (100 200 300 40 ■ POCKET PENETROMETER (kF 100 200 300 40 PLASTIC M.C. LIQUI 20 40 60 80	Solt SYMB	SOIL DESCRIPT	ION	SAMPLE TYPE SAMPLE NO	SPT (N)	COMMENTS	Depth (m)
-			Г - 25mm thick TE - 130mm thick					_
			Ity, medium to high plastic, mo v to medium plastic, moist, stiff LE TERMINATED AT 1.8m BE E.	brown, some clay				- - - - - - - - - - - - - - - - - - -
				OGGED BY: BP		COMPLE	TION DEPTH: 1.8 m	
	mec ^o			REVIEWED BY: TG		COMPLE	TION DATE: December 1, 20	
	リノてし ご	vvnnnpeg		Figure No. 22			Page	e 1 of 1











APPENDIX B

CORE PHOTOS



Photo 1: Testhole D1: Southbound Lane between Valde & RR Tracks



Photo 2: Testhole D2: Southbound Lane at 2475B Day

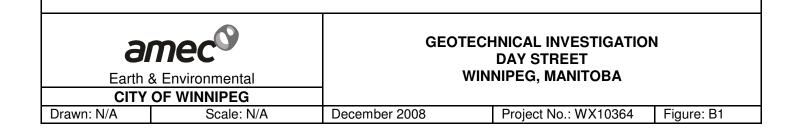
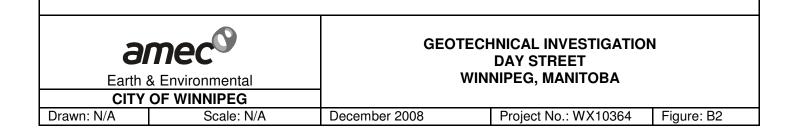






Photo 3: Testhole 3: Southbound Lane at 2500/2485 Day Photo 4: Testhol

Photo 4: Testhole D4: Northbound Lane at 2500 Day





<text>

Photo 5: Testhole H1: Westbound Lane at 233 Harbison

Photo 6: Testhole H2: Eastbound Lane at 244 Harbison

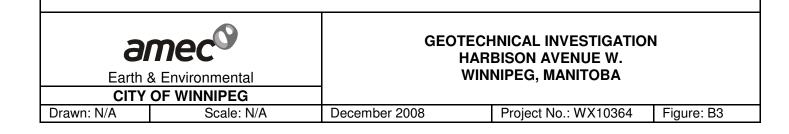


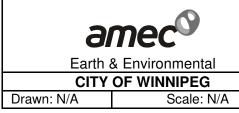


Photo 7: Testhole H3: Westbound Lane at 255 Harbison



Harbison

Photo 8: Testhole H4: Eastbound Lane at 273



GEOTECHNICAL INVESTIGATION HARBISON AVENUE W. WINNIPEG, MANITOBA

Drawn: N/A	Scale: N/A	December 2008	Project No.: WX10364	Figure: B4

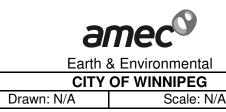




Photo 9: Testhole H5: Westbound Lane at 293 Harbison

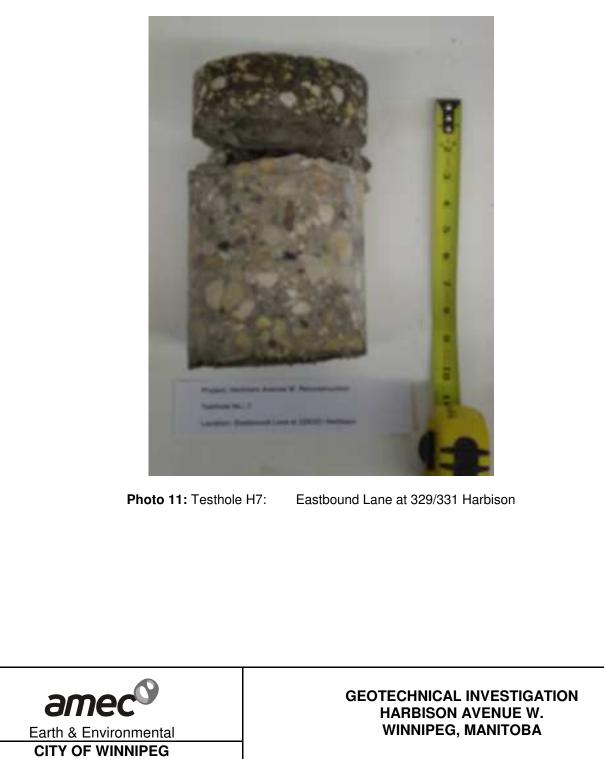
Harbison

Photo 10: Testhole H6: Eastbound Lane at 310/314



GEOTECHNICAL INVESTIGATION HARBISON AVENUE W. WINNIPEG, MANITOBA

Drawn: N/A	Scale: N/A	December 2008	Project No.: WX10364	Figure: B5



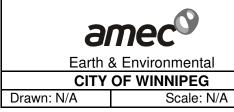
Drown: NI/A	Societ NI/A	December 2008	Project No.: WX10364	Figure: B6
Drawn: N/A	Scale: N/A	December 2008	Project No.: WX10364	Figure: B6
			, ,	0





Photo 13: Testhole L2: Leslie

Eastbound Lane at 9/11



Glenwood

GEOTECHNICAL INVESTIGATION LESLIE AVENUE WINNIPEG, MANITOBA

Drawn: N/A	Scale: N/A	December 2008	Project No.: WX10364	Figure: B7
Diawii. N/A		December 2000		i igule. Di



Westbound Lane at 20

Photo 15: Testhole L4:

Eastbound Lane at 32 Silvia



Photo 14: Testhole L3:

Leslie

GEOTECHNICAL INVESTIGATION LESLIE AVENUE WINNIPEG, MANITOBA

Scale: N/A

December 2008

Project No.: WX10364

Figure: B8

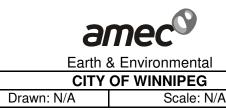




Photo 16: Testhole M1:

Southbound Lane at 49 Martin

Photo 17: Testhole M2: Martin Southbound Lane at 39



GEOTECHNICAL INVESTIGATION MARTIN AVENUE W. WINNIPEG, MANITOBA

Drawn: N/A	Scale: N/A	December 2008	Project No.: WX10364	Figure: B9



Photo 18: Testhole M3:

Northbound Lane at 34 Martin

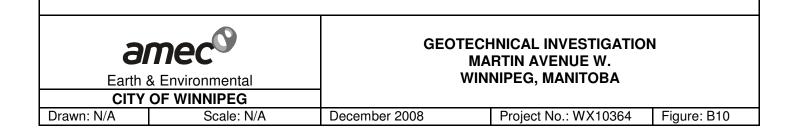




Photo 19: Testhole S1: Northbound Lane at 133 Stanier

Photo 20: Testhole S2: Southbound Lane at 140 Stanier

0 15

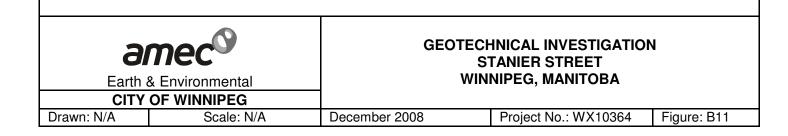




Photo 21: Testhole S3:

Northbound Lane at 145 Stanier

