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# APPENDIX 'A' GEOTECHNICAL REPORT



## REVISED REPORT GEOTECHNICAL INVESTIGATION DUGALD ROAD RECONSTRUCTION WINNIPEG, MANITOBA

Submitted to:

#### **City of Winnipeg**

Engineering Technology Services 106 – 1155 Pacific Avenue Winnipeg, Manitoba R3E 3P1

Attention: Mr. Garry Campbell

Submitted by:

AMEC Earth & Environmental
A Division of AMEC Americas Limited

440 Dovercourt Drive Winnipeg, Manitoba R3Y 1N4

12 January 2009

AMEC File No. WX10364



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#### 1.0 INTRODUCTION

AMEC Earth and Environmental, a division of AMEC Americas Limited (AMEC), were retained by the City of Winnipeg (COW) to conduct a geotechnical investigation for a proposed street upgrading project in Winnipeg, Manitoba. The location of the project is the eastbound lanes of Dugald Road, approximately between Terracon Place and Plessis Road.

The purpose of the investigation was to determine the pavement condition and soil profile at predetermined locations along Dugald Road.

#### 2.0 SITE CONDITIONS

The roadway consisted of an asphalt overlay on concrete pavement. At the time of the investigation, the roadway was snow and ice covered and therefore a detailed review of the pavement condition was not possible. Typical of roads in the Winnipeg area, the road was generally flat lying and level, with local slopes between catch basins to facilitate drainage.

#### 3.0 FIELD INVESTIGATION

The COW initially requested a total of 9 test holes, however at their request, four additional test holes were required at pavement joint locations. The initial 9 test hole locations were determined by the COW prior to the investigation. The additional four joint test hole locations (test holes TH02, TH05, TH09 and TH13) were determined by AMEC.

Prior to coring and drilling, AMEC had public underground utilities located. Due to the presence of utilities at one of the proposed test holes (TH14), the test hole could not be drilled. On December 5, 2008, following utility clearances, all remaining test hole locations, were cored prior to auger drilling in order to determine the asphalt and concrete thicknesses. All coring was completed with a 150 mm diameter diamond coring rig. Each of the cores was photographed and photos are presented in Appendix B.

On 8 and 9 December 2008, a total of thirteen (13) test holes (TH01 to TH13) were drilled on the roadway, using a truck mounted Acker MP5T drill rig operated by Paddock Drilling Ltd. of Winnipeg, and equipped with 125 mm continuous flight solid stem augers. The test hole locations are shown on Figures 1 to 4 in Appendix A.

Test hole logging and subcontractor supervision was provided on a full time basis by Mr. Blair Power of AMEC. Traffic control during the drilling operation was provided by Guardian Traffic Services.

All soils observed during test hole drilling were visually classified on site according to the Modified Unified Soil Classification System (MUSCS) and in accordance with the City of Winnipeg geotechnical guidelines. Groundwater and drilling conditions, as well as any pertinent subsurface observations, were also recorded at the time of the investigation.



Disturbed soil samples were taken at regular intervals from the auger flights in each test hole. All soil samples obtained during the field investigation were labelled, sealed in plastic bags to limit moisture loss and transported to AMEC's Soils Laboratory in Winnipeg for further examination and testing. The test hole logs are presented in Appendix A, Figures 5 to 17 and show the soil profile, results of the field and laboratory testing, and comments relative to groundwater and sloughing conditions encountered.

Each test hole was backfilled with the auger cuttings and topped with asphalt cold patch at the completion of drilling, with excess cuttings moved to the side of the road.

#### 4.0 LABORATORY TESTING

Soil samples were returned to AMEC's Soils Laboratory in Winnipeg for geotechnical laboratory testing. All soil samples were visually classified and tested for in-situ moisture contents, and selected samples were tested for Atterberg limits and Hydrometer analysis to confirm the field classification of soils.

#### 5.0 SUBSURFACE CONDITIONS

Although variable across the 13 test holes completed, the generalized stratigraphy of the roadway can be described as follows, as noted in descending order from the ground surface:

- Asphalt
- Concrete
- Gravel, Silt and/or Clay Fill
- Medium to High plastic clay
- Silt
- High plastic clay

Asphalt overlying concrete pavement were present at each test hole location. The asphalt thickness generally varied from 40 to 65 mm with the exception of test hole TH01, where the asphalt was 132 mm. The concrete below the asphalt overlay also varied in thickness from 76 to 206 mm. The concrete and asphalt were underlain by variable fill materials that generally consisted of gravel fill (12 test holes) underlain by clay fill (8 test holes). At the time of drilling, the gravel fill was frozen throughout its entire depth. The gravel fill again varied from about 15 mm (TH03) to 865 mm (TH04) thick (averaging about 310 mm thick) and generally consisted of 20mm granular material that was sandy, frozen and brown and ranged in moisture content from damp to wet. The clay fill was about 120 mm (TH01) to 970 mm (TH03) thick (averaged 530 mm) and was typically silty, medium to high plastic, moist, stiff (where not frozen) and grey to brown, and occasionally contained sand or gravel.

Native high plastic clay was present directly below the fill layers at 10 of the 13 locations, and was about 610 to 1800 mm thick, with an average thickness of 1150 mm. At these test holes the clay extended to the depths explored. At the other three locations (TH06, TH08 and TH11), the fill was underlain by low plastic silt which was soft to firm, moist, and occasionally sandy.



The thickness of the silt layer varied from 230 mm (TH06) to 760 mm (TH08), with an average thickness of 130 mm. Where silt was present, high plastic clay extended from below the silty clay layer to the depths explored, and was generally stiff to very stiff and moist.

Table 1, below, summarizes the thickness and types of pavements and soils encountered at each of the test hole locations. Selected test results are presented in Table 2. Detailed soil stratigraphy is illustrated in the attached test hole logs in Appendix A. Photographs of the core samples are attached in Appendix B.

Table 1: Pavement and Soil Thickness (mm)

Test Hole No.	TH01	TH02	TH03	TH04	TH05	TH06	TH07	TH08	TH09	TH10	TH11	TH12	TH13
Asphalt	132	47	48	42	56	58	45	54	42	51	43	60	65
Concrete	200	180	190	160	200	210	200	185	185	190	200	206	185
Fill – Gravel	95	160	15	865	510	495	510	65	380	520	50	N/A	120
Fill – Clay	122	330	966	N/A	N/A	N/A	460	460	610	N/A	315	954	N/A
Clay	1585	1524	915	1065	1065	N/A	914	N/A	610	N/A	1065	915	1800
Silt	N/A	N/A	N/A	N/A	N/A	230	N/A	760	N/A	N/A	305	N/A	N/A
Clay	N/A	N/A	N/A	N/A	N/A	1145	N/A	610	N/A	1220	150	N/A	N/A

**Table 2: Lab Test Results** 

Test Hole No.	Sample Depth (m)	Moisture Content (%)		Hydrometer Analysis						
			Liquid Limit	Plastic Limit	Plasticity Index	MUSCS Classification	Gravel (%)	Sand (%)	Silt (%)	Clay (%)
TH06	0.9	12	35	28	8	ML (Silt)	2	44	41	13
TH07	0.9	33	46	22	24	CI (Clay)	2	22	44	32

#### 6.0 CLOSURE

The findings of this report were based on the results of field and laboratory investigations at test hole locations as selected by the City of Winnipeg.

The site investigation was conducted for the sole purpose of profiling the pavement and subsurface conditions. Although no environmental issues were identified during the fieldwork, this does not indicate that no such issues exist. If the owner or other parties have any concern



regarding the presence of environmental issues, then an appropriate level environmental assessment should be conducted.

Soil conditions, by their nature, can be highly variable across a site. The placement of fill and prior construction activities on a site can contribute to the variability especially near surface soil conditions. A contingency should always be included in any construction budget to allow for the possibility of variation in soil conditions, which may result in modification of any potential design and construction procedures which may arise from this factual investigative report.

This report was prepared exclusively for the City of Winnipeg, and their agents for the proposed development as described in the report. The data provided herein are presented in a factual manner only with no engineering interpretation provided, and should not be used for any other purpose, or by any other parties, without review and advice from a qualified geotechnical engineer. No other warranty, expressed or implied, is given.

Yours truly,

**AMEC Earth & Environmental** 

Robert Brown, P. Eng. Geotechnical Engineer

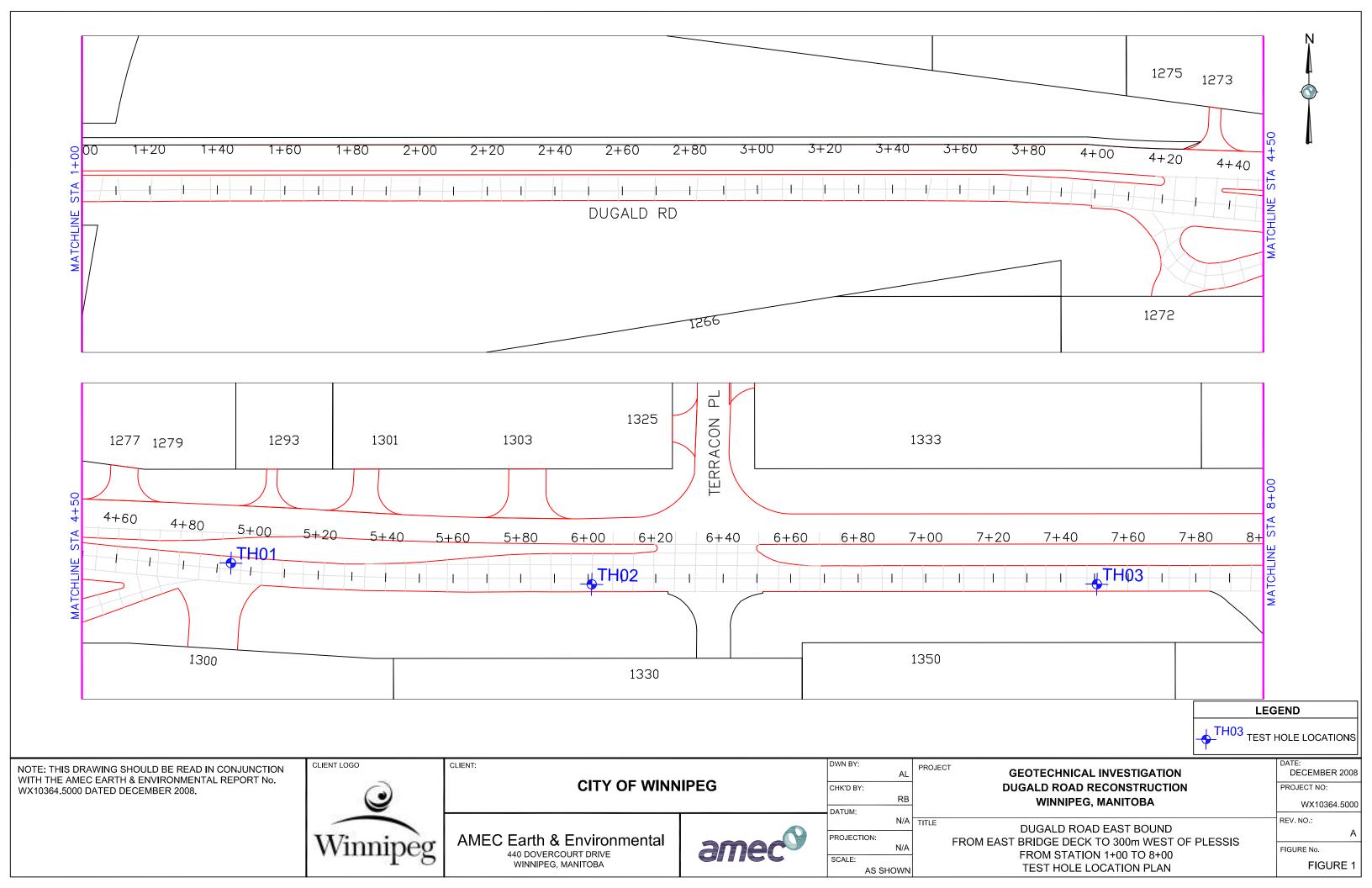
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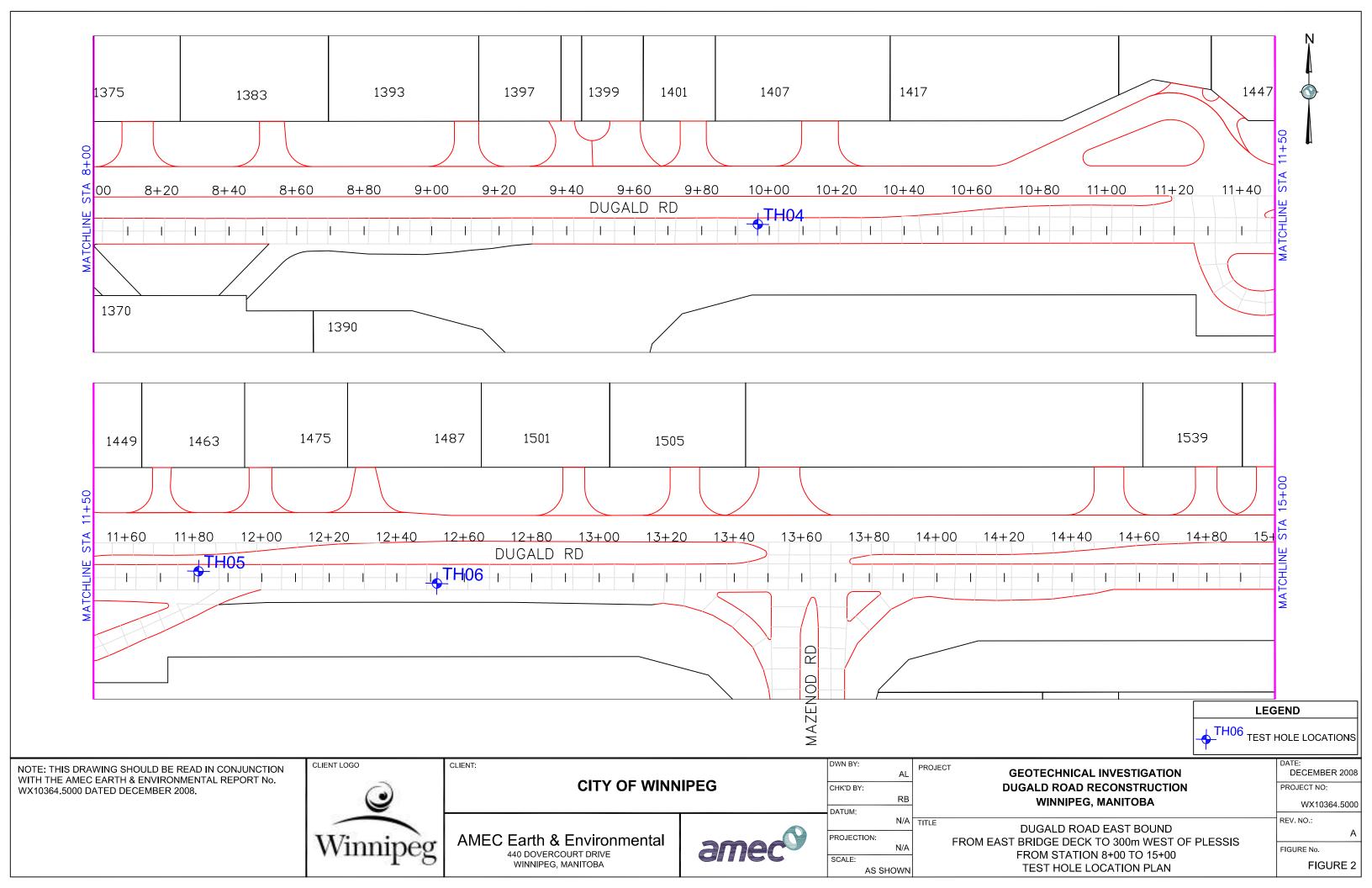
Trevor Gluck, P. Eng. Staff Geotechnical Engineer

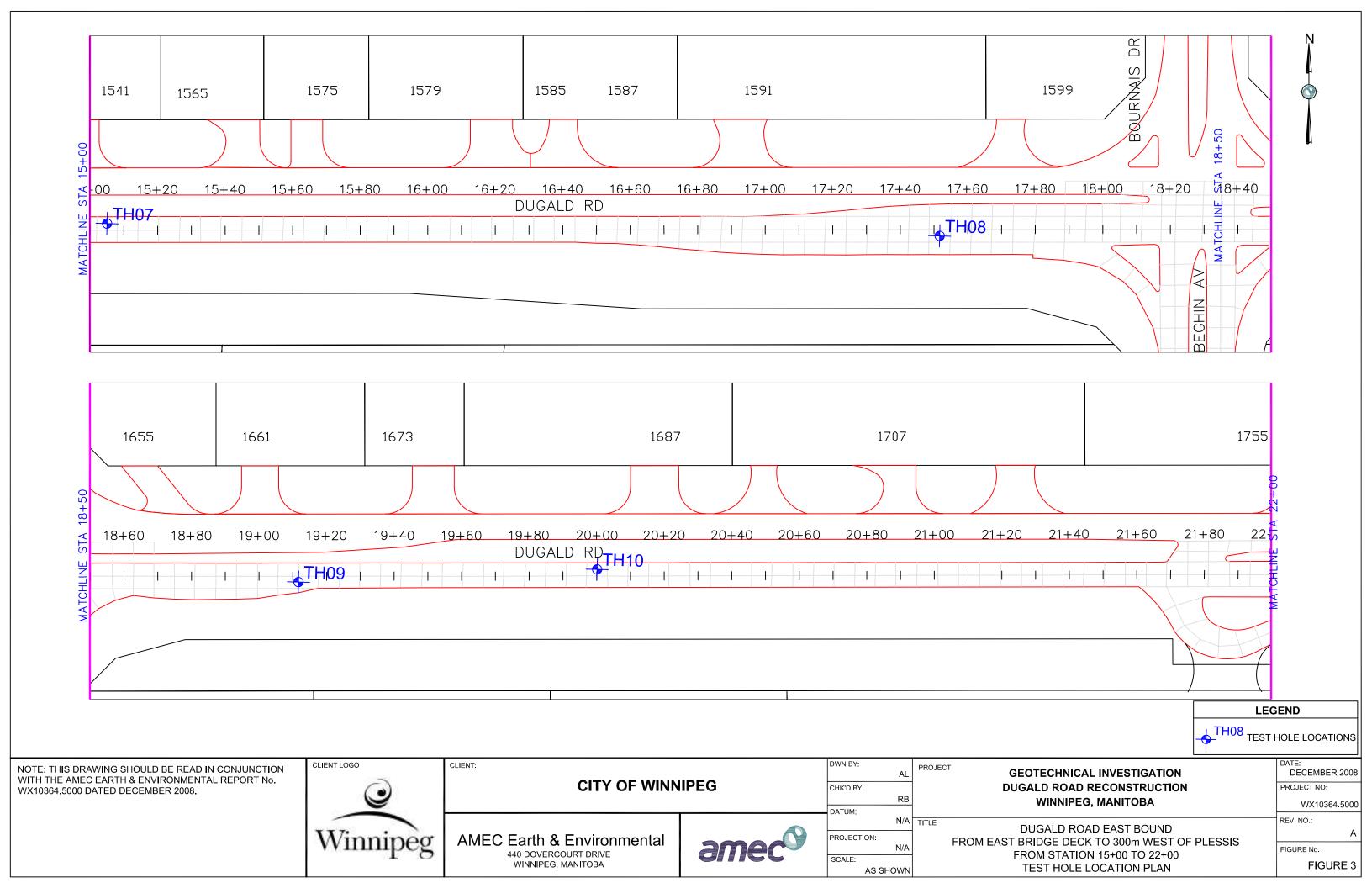


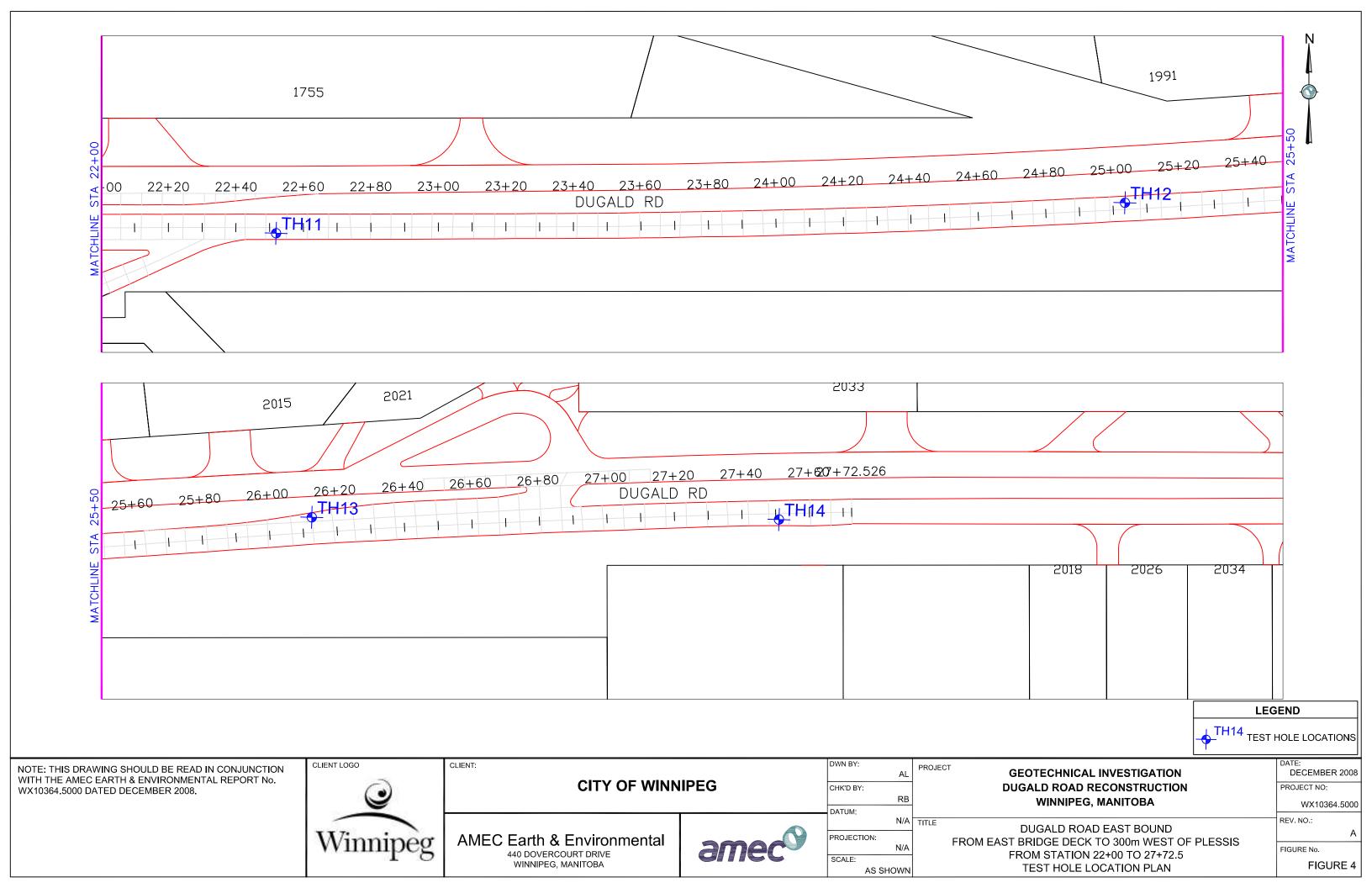
#### **APPENDIX A**

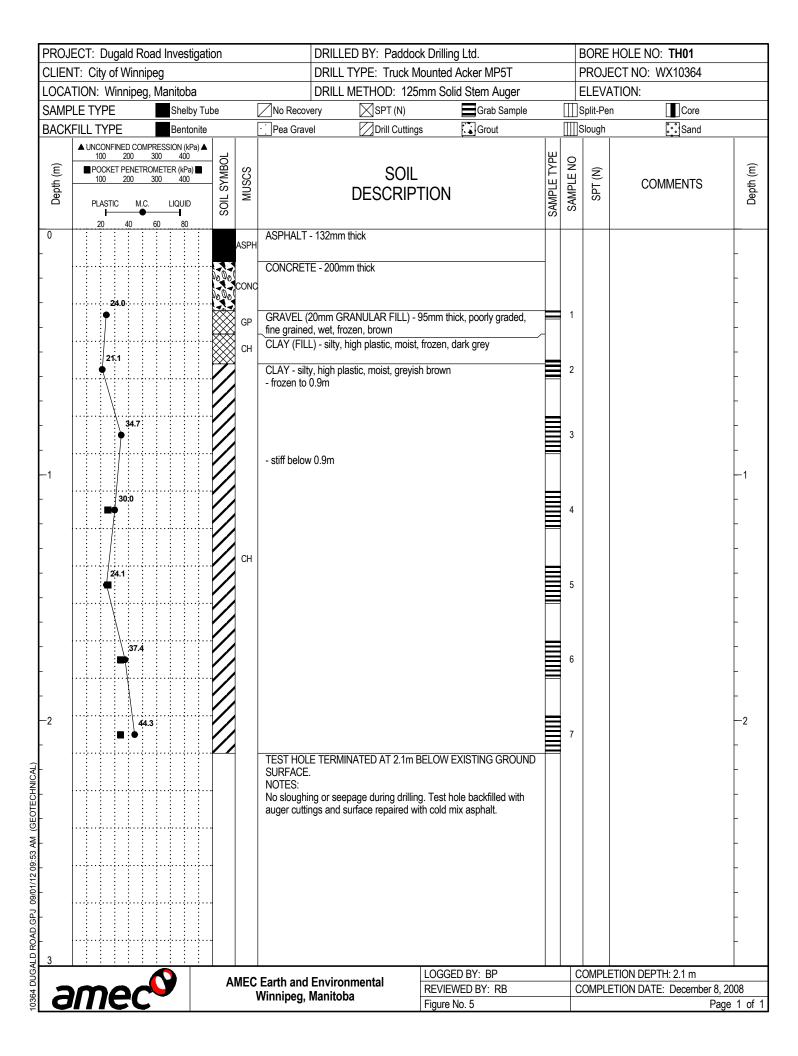
#### **TEST HOLE LOCATION PLANS AND TEST HOLE LOGS**

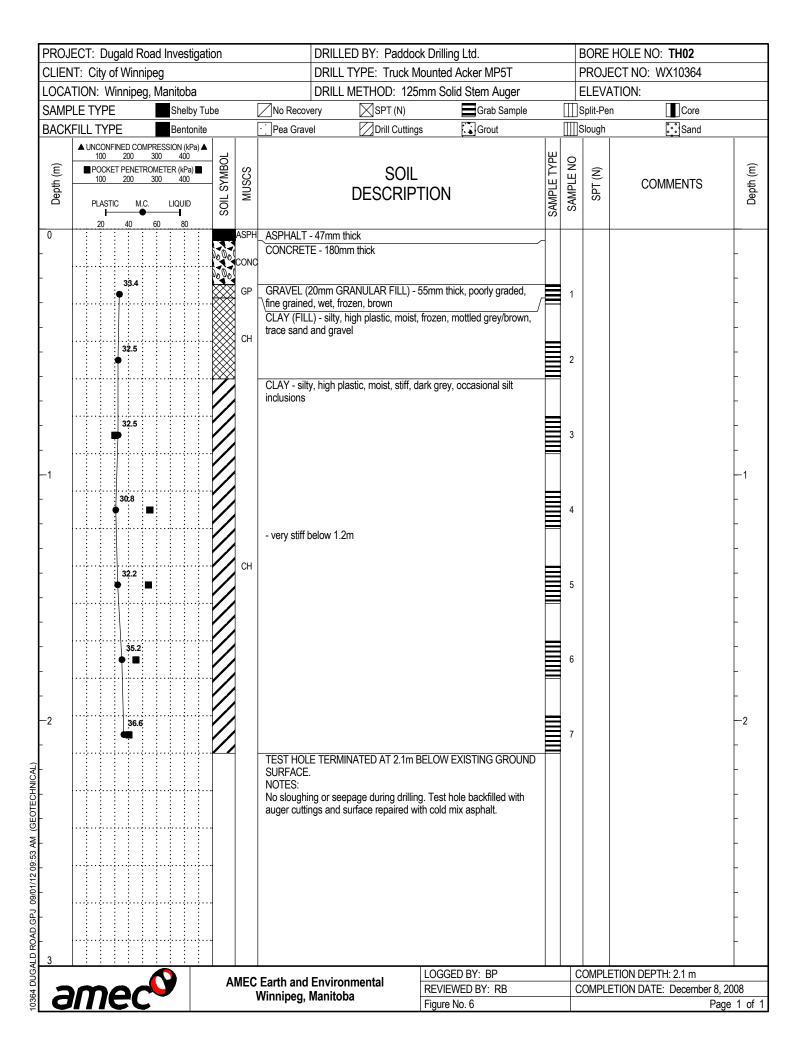


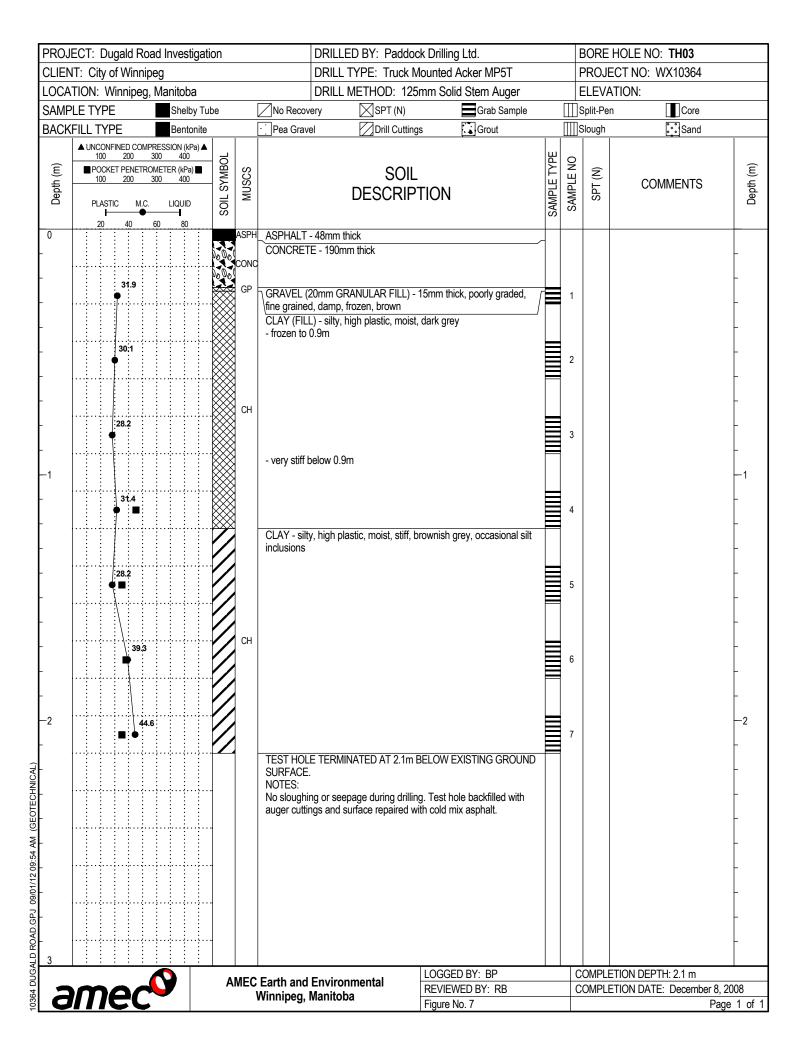


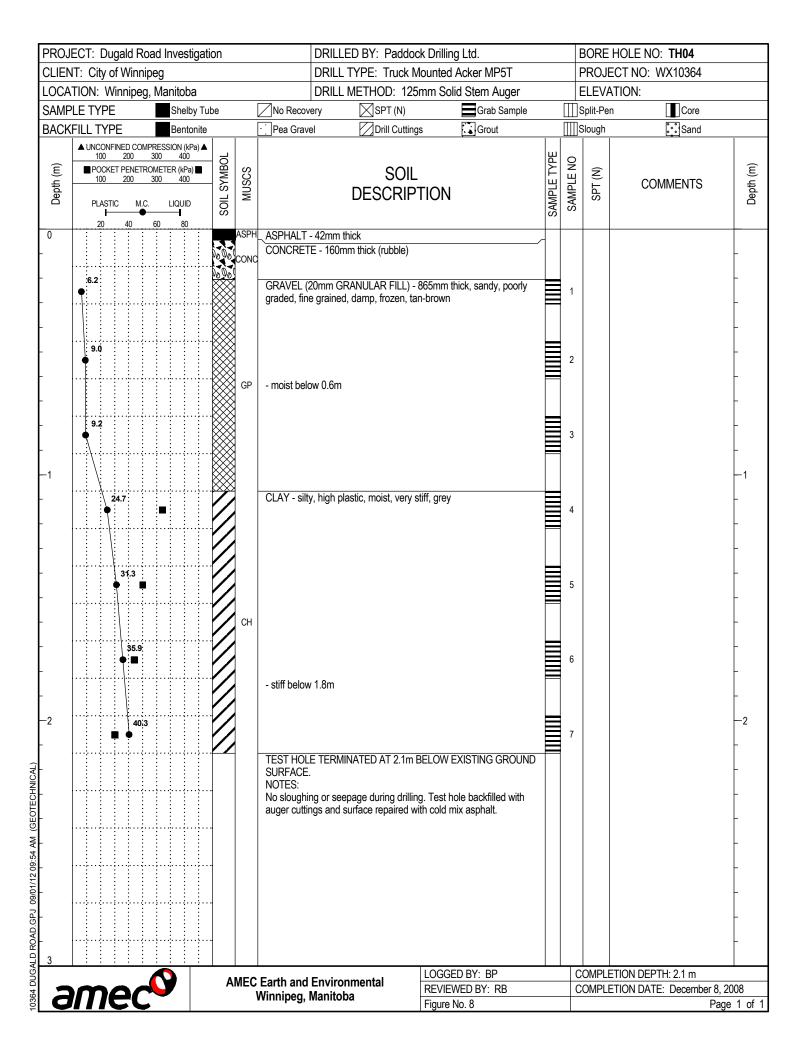


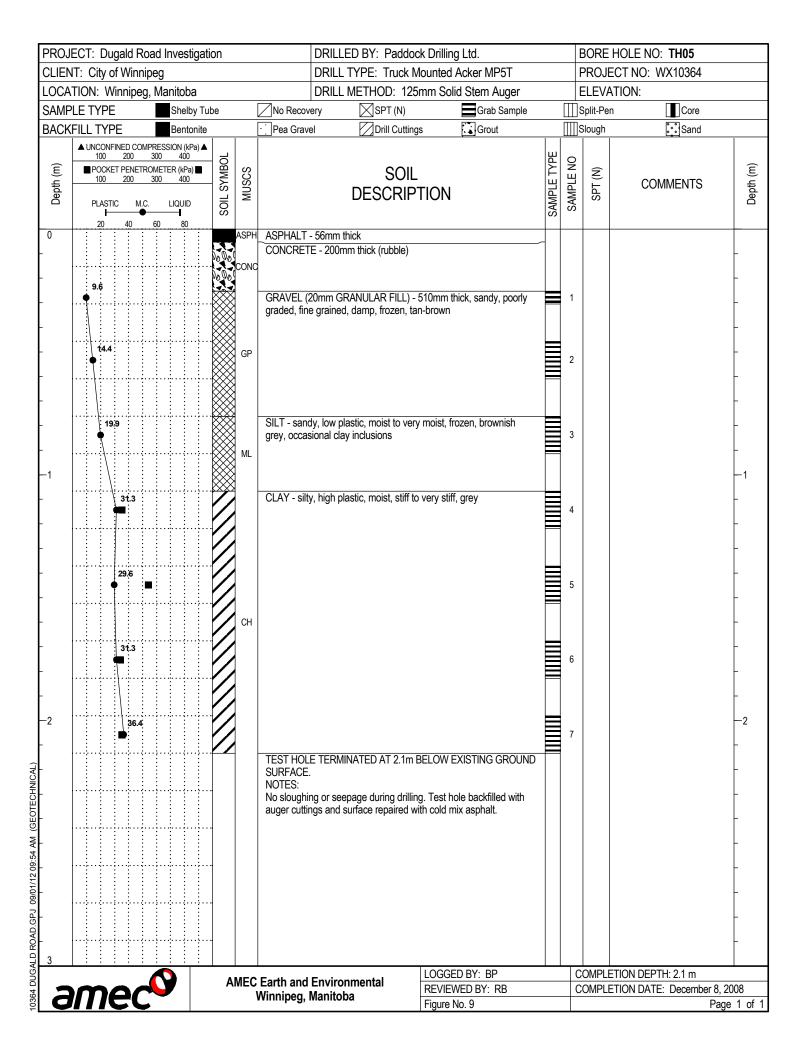


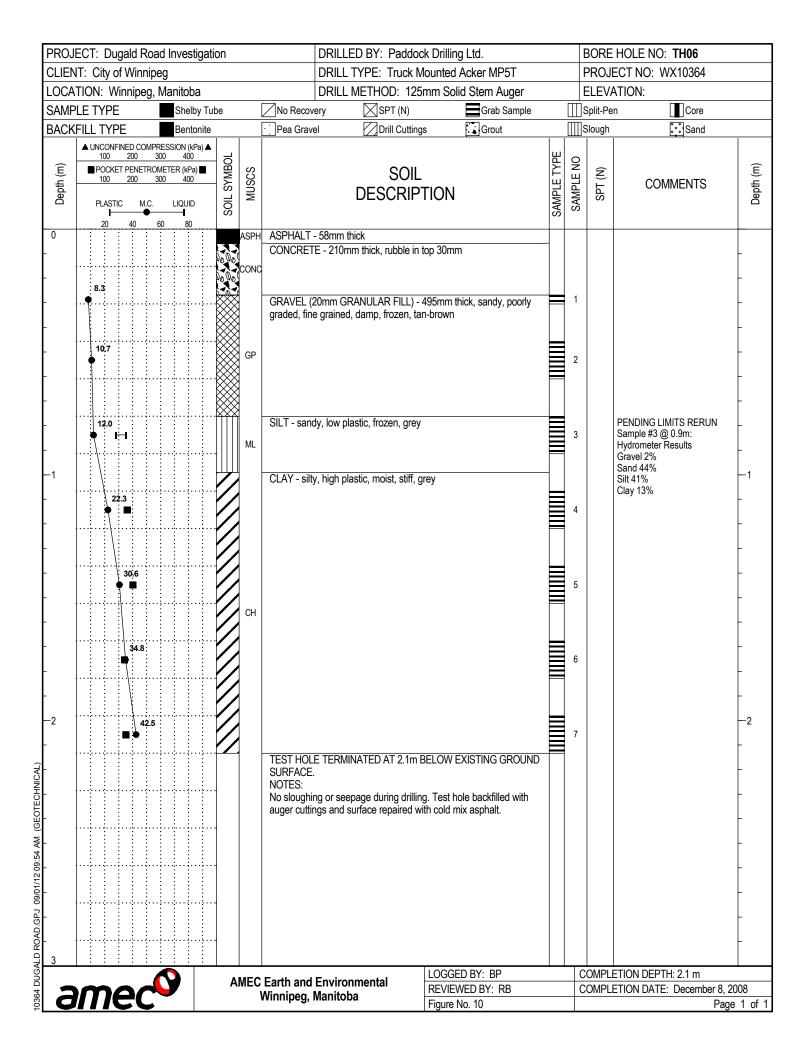


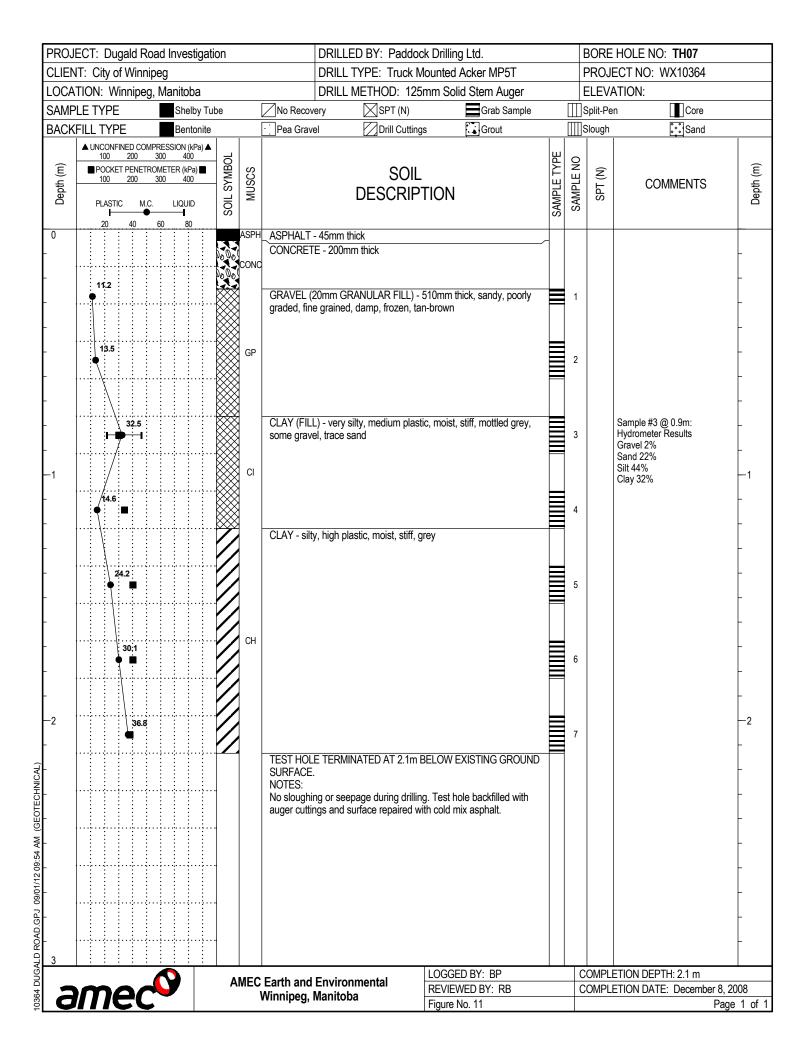


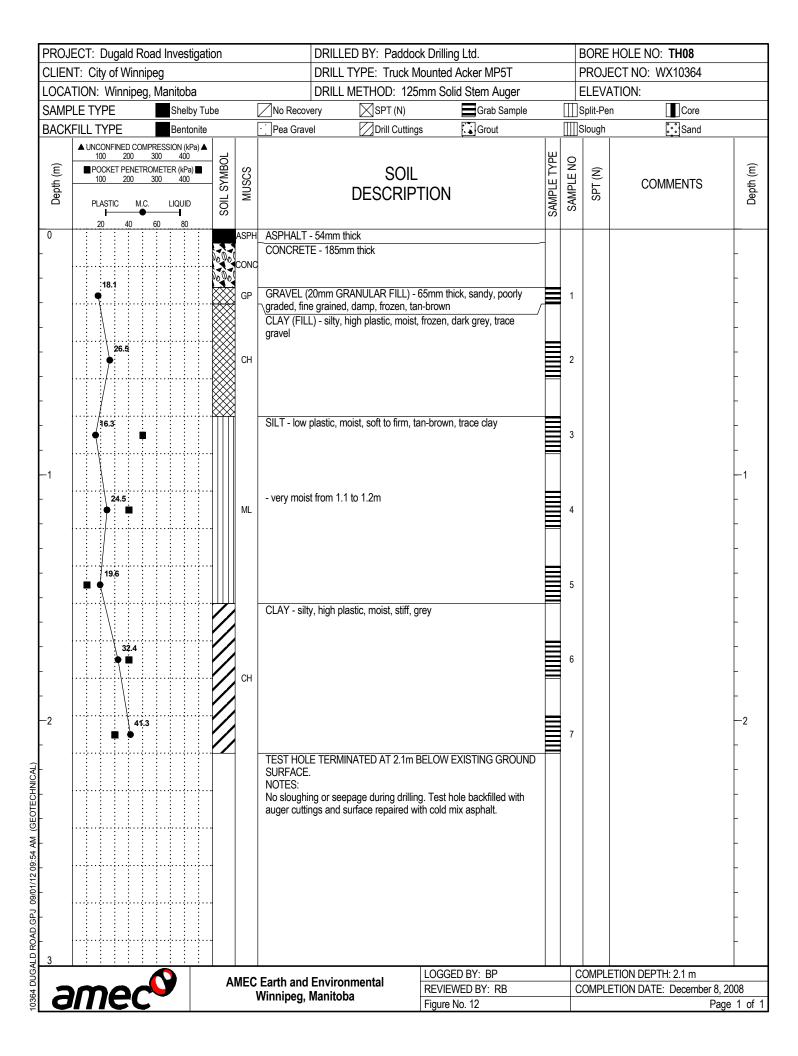


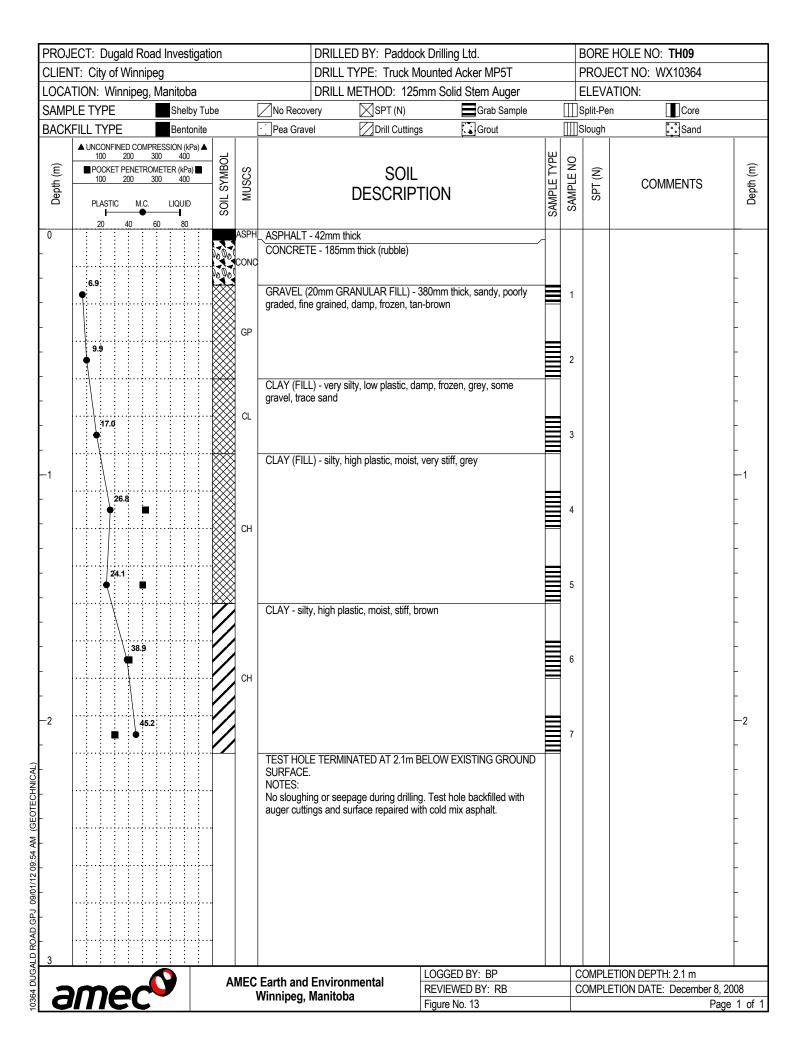


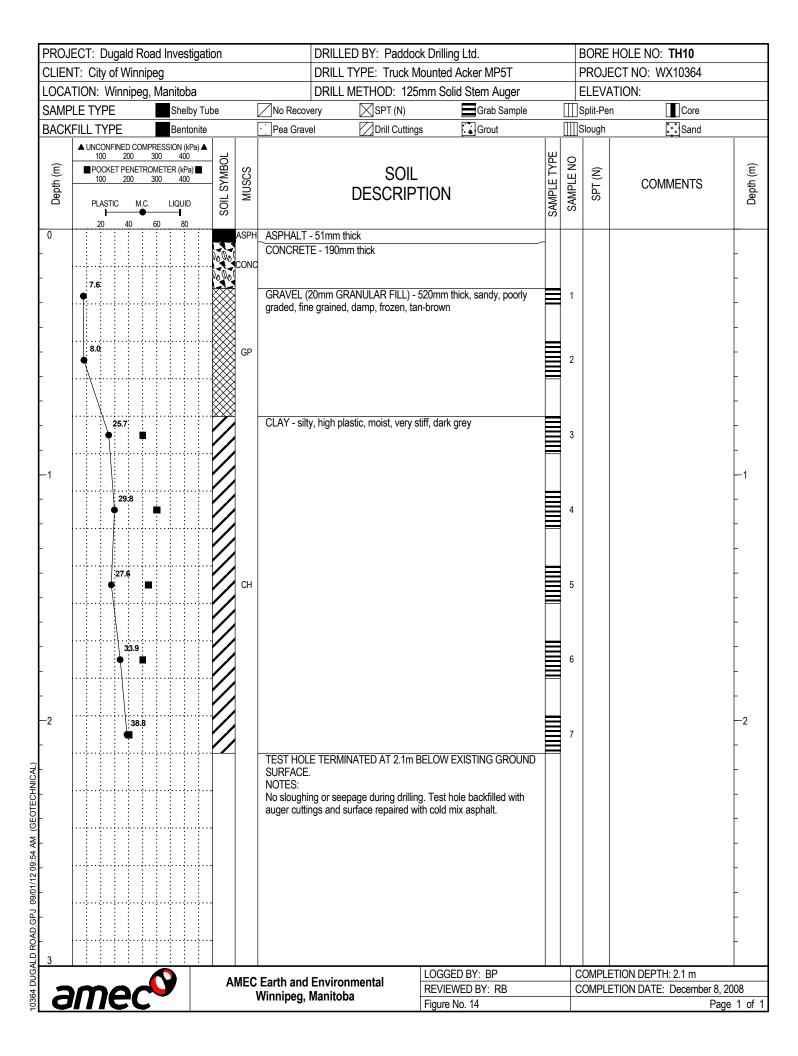


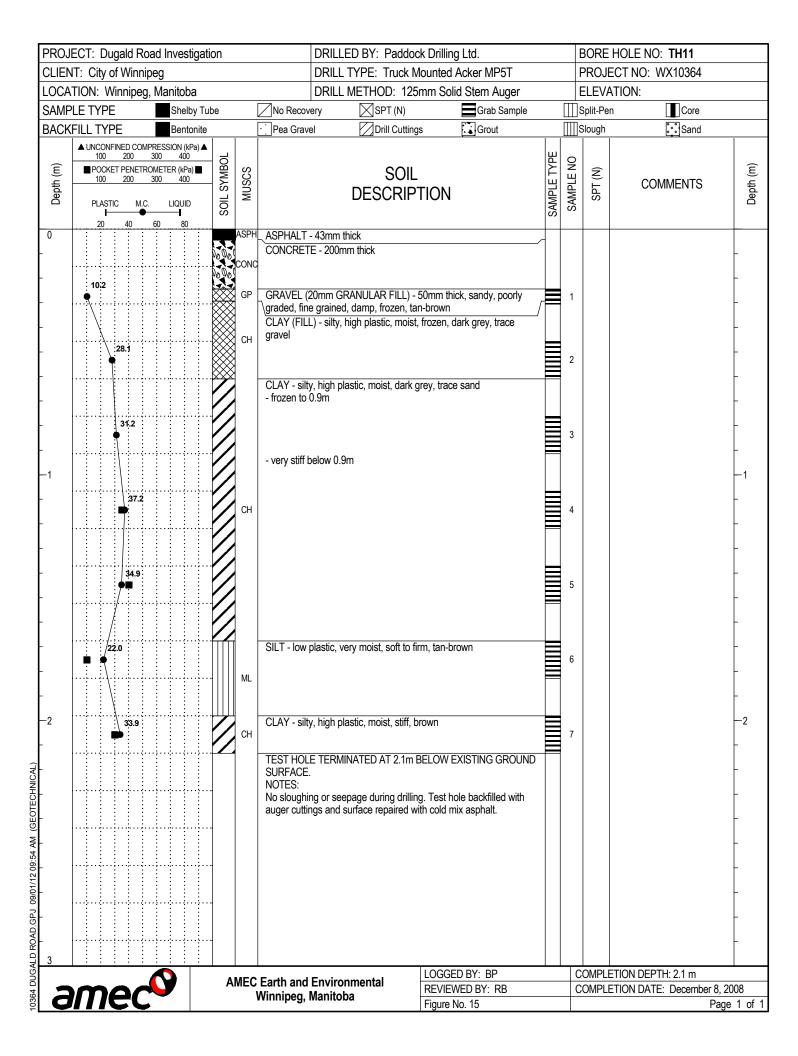




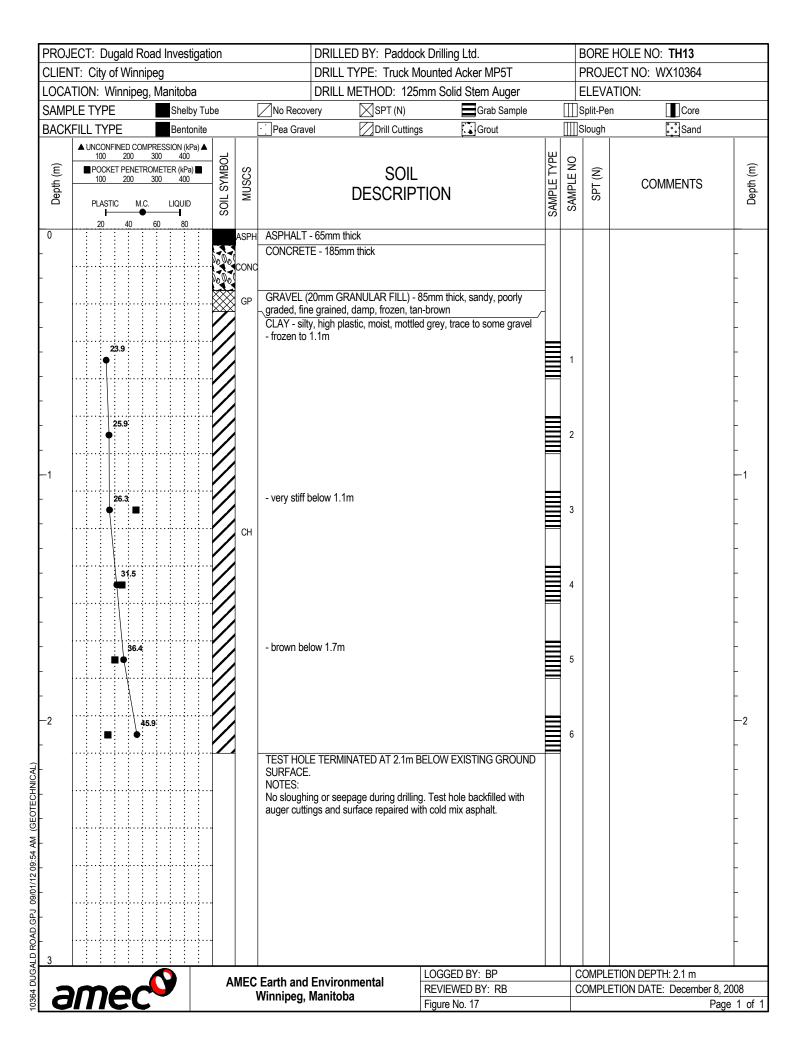








PROJ	IECT: Dugald Road Inves	tigation	DRILLED BY: Paddoo	k Drilling Ltd.	BOR	BORE HOLE NO: TH12			
CLIEN	NT: City of Winnipeg		DRILL TYPE: Truck N	lounted Acker MP5T	PRC	PROJECT NO: WX10364			
LOCA	ATION: Winnipeg, Manitob	ра	DRILL METHOD: 125	mm Solid Stem Auger	ELE'	ELEVATION:			
SAME	PLE TYPE Shell	by Tube	lo Recovery SPT (N)	Grab Sample	Split-l	Pen Core			
BACK	KFILL TYPE Bent	onite : F	ea Gravel Drill Cutting	s Grout	Sloug	اh د المعاملة المعام			
(m)	▲ UNCONFINED COMPRESSION (k 100 200 300 400 ■ POCKET PENETROMETER (kP: 100 200 300 400	Pa) A NBOL	SOIL	TYPE	ON EI	(E)			
Depth (m)	PLASTIC M.C. LIQUIE	SOIL S'	DESCRIP		SAMPLE NO SPT (N)	COMMENTS (E) #ded			
0	20 40 60 80	ASPH AS	PHALT - 60mm thick						
-	31.8	CONC	NCRETE - 206mm thick			-			
-	31.0	: XX   wo	AY (FILL) - silty, high plastic, moist od pieces ozen to 0.9m	grey, trace sand, occasional	1	-			
-	30.6				2	-			
-		СН				_			
-	25.1	- vi	ery stiff, trace gravel below 0.9m		3	-			
-1 -	30,5		ory cam, accession com			-1			
-	<b>1</b>	CL	AY - silty, high plastic, moist, very s	tiff, grey	4	-			
	34.8 •••				5	_			
-	36.8	СН			6	-			
-2 -	42.0		ery silty, very moist, soft to firm belo		7	-2			
ECHNICAL)		SU NO No	ST HOLE TERMINATED AT 2.1m I RFACE. ITES: sloughing or seepage during drillin	g. Test hole backfilled with		-			
H AM (GEOT		au	ger cuttings and surface repaired w	ith cold mix asphalt.					
101/12 09:54						-			
10364 DUGALD ROAD.GPJ 09/01/12 09:54 AM (GEOTECHNICAL)						-			
3	<u>                                     </u>	:			$\perp$				
DO _			th and Environmental	LOGGED BY: BP REVIEWED BY: RB		COMPLETION DEPTH: 2.1 m			
0364	<u>mec<sup>o</sup></u>	Win	nipeg, Manitoba	Figure No. 16	COIVIE	COMPLETION DATE: December 8, 2008  Page 1 of 1			





#### **APPENDIX B**

**CORE PHOTOS** 



**Photo 1:** Testhole 1: Eastbound Median Lane at 1300 Dugald



**Photo 2:** Testhole 2: Eastbound Median Lane at 1325 Dugald





**Photo 3:** Testhole 3: Eastbound Curb Lane at 1357 Dugald



**Photo 4:** Testhole 4: Eastbound Median Lane at 1407 Dugald





**Photo 5:** Testhole 5: Eastbound Median Lane, 1447/1467 Dugald



**Photo 6:** Testhole 6: Eastbound Curb Lane at 1487 Dugald





**Photo 7:** Testhole 7: Eastbound Median Lane at 1541 Dugald



**Photo 8:** Testhole 8: Eastbound Curb Lane at 1599 Dugald





**Photo 9:** Testhole 9: Dugald

Eastbound Curb Lane at 1665



**Photo 10:** Testhole 10: Eastbound Median Lane at 1687 Dugald





Photo 11: Testhole 11: 1755/1783 Dugald

Eastbound Curb Lane at



Photo 12: Testhole 12: Eastbound Median Lane at 1991 Dugald



**CITY OF WINNIPEG** 

Drawn: N/A Scale: N/A **GEOTECHNICAL INVESTIGATION DUGALD ROAD WINNIPEG, MANITOBA** 

Project No.: WX10364 December 2008 Figure: B6



**Photo 13:** Testhole 13: Eastbound Median Lane at 2015 Dugald

